# The Lake Lothing (Lowestoft) <br> Third Crossing Order 201[*] 



# Document 7.2: Transport Assessment 

Appendix E (Part 1) Highway LMVR and DMVR

## ID|)

## Suffolk County Council

## LOWESTOFT DCO MODELLING

Highway Model Local Validation and Forecasting Report


## Suffolk County Council

## LOWESTOFT DCO MODELLING

Highway Model Local Validation and Forecasting Report

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## 1 INTRODUCTION

### 1.1 BACKGROUND

1.1.1. WSP has been commissioned by Suffolk County Council (SCC) to upgrade the existing transport modelling tools available to SCC and develop an integrated county-wide multi-modal model known as the Suffolk County Transport Model (SCTM). WSP is working in partnership with Kier under the Suffolk County Council Professional Services Framework.
1.1.2. The SCTM has been developed to an extent that it is able to serve as a high-level strategic assessment tool for all such applications. However, no strategic model is capable of representing a whole county in fine detail, so the level of detail required for each application should be reviewed prior to testing. It may be necessary to enhance a particular local area for a specific testing purpose.
1.1.3. In particular the model was developed to enable the testing of The Upper Orwell Crossings (TUOC) in Ipswich and the Lake Lothing Third Crossing in Lowestoft. As a result the level of detail of coding in Ipswich and Lowestoft is in significantly greater detail and based upon the modelling used for the business case submissions in December of 2015.
1.1.4. The SCTM highway assignment model therefore incorporates the model networks used within the lpswich Transport Model (ITM) and Lowestoft Traffic Model (LTM) which were previously used to inform the Outline Business Cases (OBCs) for the Ipswich Wet Dock Crossing and Lake Lothing Third Crossing.
1.1.5. Details upon the full county model development, methodologies and performance are set out within the D3 SCTM LMVR (November 2017) and should be read in conjunction with this report.

### 1.2 MODEL PURPOSE

1.2.1. The SCTM is a substantial improvement to previous transport modelling tools within Suffolk and allows for a greater range of behavioural responses to be tested. The SCTM provides a robust evidence base for a range of possible applications.
1.2.2. In this application and local validation, the model is being used solely to test the impact of the Lowestoft third crossing and support the scheme through the DCO process and subsequent final business case application to the Department for Transport (DfT).

### 1.3 PURPOSE OF THIS REPORT

1.3.1. The SCTM model is currently validated at a county level, and to support the Lake Lothing Third Crossing through the DCO process a local model validation of Lowestoft is required. The local validation process ensures that more detailed local congestion is better represented in the model and that local validation issues are not lost in the scale of the countywide model.
1.3.2. The aim of this local validation report is therefore to demonstrate the SCTM highway model is fit for purpose particularly in Lowestoft and is developed following the principles set out within WebTAG guidance to ensure the model can provide an improved appraisal of the proposed scheme stand up to scrutiny as part of the DCO process.
1.3.3. In addition this report sets out the forecast model development methodologies to demonstrate the suitability of the model results for inclusion within the transport assessment and business case appraisal.

### 1.4 PREVIOUS MODELS

1.4.1. The Lowestoft Traffic Model (LTM) was a highway assignment model using SATURN validated to a 2015 base with Variable Demand Modelling (VDM) carried out using DIADEM. This was updated as part of the Lake Lothing Third Crossing Transport Business Case, with demand matrices developed based on ANPR data and traffic survey data collected in 2015.
1.4.2. The networks and zone system for LTM were incorporated into the SCTM and used to form the basis of the simulation network for Lowestoft.

### 1.5 PLANNED BASE YEAR MODELLING

1.5.1. The SCTM has a base year of 2016 based on an average Monday to Thursday for neutral months.
1.5.2. The main matrix demand for the 2016 base year uses Mobile Network Data (MND) from Telefonica, which is considered to be accurate at MSOA level. A detailed review of the mobile network data is set out in the WSP document entitled 'Mobile Network Data Verification Report' dated October 2017.
1.5.3. The following three time periods have been modelled:

- AM peak hour (0800-0900)
- Inter peak average hour (1000-1600)
- PM peak hour (1700-1800)


### 1.6 REPORT STRUCTURE

1.6.1. This Local Model Validation Report (LMVR) sets out information relating to the development, calibration and validation of the updated highway assignment model. It is structured as follows:

- Section 2 - Lake Lothing Third Crossing
- Section 3 - Local Model Detail
- Section 4 - Calibration and Validation Data
- Section 5 - Network Development
- Section 6 - Trip Matrix Development
- Section 7 - Assignment Calibration and Validation
- Section 8 - Forecasting Methodology
- Section 9 - Summary


## 2 LAKE LOTHING THIRD CROSSING

### 2.1 SCENARIOS TO BE FORECAST AND INTERVENTIONS TO BE TESTED

2.1.1. The base year validation update to the SCTM has been carried out in order to be able to test the latest central design for the Lake Lothing Third Crossing.
2.1.2. The town centre in Lowestoft currently has two river crossings shown in Figure 1 below.


Figure 1 - Bridge locations
2.1.3. The scheme which will be tested involves a third central bridge crossing between Peto Way / Denmark Road to the north and Waveney Drive to the south. The latest central crossing design, Option C 18 , is shown in Figure 2.


Figure 2 - Lake Lothing Third Crossing - Option C18

### 2.2 SCENARIOS TO BE TESTED

2.2.1. The following forecast years have been used to test the Lake Lothing Third Crossing:

- 2022; scheme opening year;
- 2037; scheme opening year + 15 years
2.2.2. The forecast year models have been developed on an uncertainty based approach for Waveney District which the scheme is located within. Forecasts have been developed for a core scenario only based on residential developments which are considered to be "Near Certain" or "More Than Likely".
2.2.3. Information relating to the development forecast model is reported in section 8 of this report.


## 3 LOCAL MODEL DETAIL

### 3.1 AREA OF INFLUENCE

3.1.1. The area of influence for the Lake Lothing Third Crossing scheme has been determined by comparing previous 2036 SCTM forecasts with and without the scheme to determine the extent of the significant flow changes which occur. Figure 3 shows the extent of the area of influence for the Lake Lothing Third Crossing.


Figure 3 - Modelled Area

### 3.2 ZONING SYSTEM

3.2.1. The zoning system within the SCTM was based initially on 2011 Census boundaries:

- Lower Super Output Area (LSOA) level across Suffolk
- Combinations of MSOAs within districts adjacent to Suffolk
- District level in other adjacent counties within the East of England (Norfolk, Cambridgeshire, Essex, Hertfordshire, Bedfordshire)
- County and regional level in remainder of UK outside of the East of England
3.2.2. Some zones when the further disaggregated where LSOA areas remained too large. The final model zone system contains 893 zones.
3.2.3. Figure 4 shows the detail of the model zones adjacent to the scheme and existing swing bridges in north and south Lowestoft.


Figure 4-Zoning in vicinity of Lake Lothing Third Crossing
3.2.4. Figure 5 shows the detail of the model zones within the area of influence for the scheme.


Figure 5-Zoning within Area of Influence
3.2.5. Figure 6 highlights the internal zones within Lowestoft and the surrounding area within Suffolk, as well as highlighting the external zones adjacent to Lowestoft covering Great Yarmouth and other locations in Norfolk.


Figure 6 - Zoning in external area
3.2.6. The level of detail shown for the zoning within Lowestoft and the surrounding area is considered to be sufficiently detailed to capture the key local land uses and provide a suitable basis for base year model validation and calibration.

### 3.3 NETWORK STRUCTURE

3.3.1. The key strategic roads within Lowestoft are included within the SCTM. The network structure in close proximity to the bridge crossings is shown in Figure 7.


Figure 7 - Local network structure
3.3.2. Figure 8 presents the network structure for Lowestoft and the surrounding area.


Figure 8 - Wider network structure
3.3.3. It is considered the SCTM network is sufficiently detailed in Lowestoft for the purposes of base year validation and calibration, and provides a sufficient basis from which to build forecasts to appraise the Lake Lothing Third Crossing scheme.

### 3.4 CENTROID CONNECTORS

3.4.1. Centroid connectors connect the zoning system to the model network, allowing trips to load onto the network for assignment. It is critical that centroid connectors represent realistic loading points, particularly in the fully modelled area. Centroid connectors have been designed to represent actual loading points to specific residential and commercial areas, generally via a spur link to represent the actual access point. In this way, turns into and out of zones can be clearly understood.
3.4.2. The number of centroid connectors has been minimised, with most zones having a single centroid connector except in cases where a zone has clear multiple points of access, and sub-dividing the zone would not be realistic.
3.4.3. Centroid connectors have been designed so that they do not cross the network, further ensuring that loading is realistic. Connectors for different zones are loaded at different points in the majority of cases, to ensure trips between adjacent zones are loaded on to the network. Centroid connectors are also loaded away from count locations, to avoid inconsistencies between the counted flow and loaded trips.
3.4.4. In the internal simulation network covered by Suffolk, zones are sufficiently small such that average costs to access the model are sufficiently represented by the spur access links, so centroids themselves do not have costs associated with them.
3.4.5. In the external area, centroid connectors are linked to the network with appropriate parameters for distance and average speed to represent the average cost of accessing the network.

### 3.5 USER CLASSES

3.5.1. The following user classes are modelled within the SCTM:

- UC1: Car - Home Based Work (Inbound)
- UC2: Car - Home Based Work (Outbound)
- UC3: Car - Home Based Employers Business (Inbound)
- UC4: Car - Home Based Employers Business (Outbound)
- UC5: Car - Non Home Based Employers Business
- UC6: Car - Home Based Other (Inbound)
- UC7: Car - Home Based Other (Outbound)
- UC8: Car - Non Home Based Other
- UC9:LGV
- UC10:HGV
3.5.2. These car user classes are consistent with those presented in the D3 SCTM LMVR (November 2017). This detail in terms of model user classes is used to aid the conversion of highway assignment matrices in OriginDestination format into Production-Attraction matrices in the SCTM Variable Demand Model (VDM). The SCTM VDM needs to be able to distinguish which part of a trip is home-based, inbound; meaning an individual is heading towards their place of residence, and outbound; an individual is leaving their home at the start of the trip. This directionality of trips is available in the Mobile Network Data (MND) which was used to build the matrices and therefore this information was utilised rather than the SCTM Demand Model having to infer directionality of home-based trips artificially from user classes which combine the inbound and outbound direction of home-based trips.


### 3.6 ASSIGNMENT METHODOLOGY

3.6.1. Model assignment of trips to the highway network was undertaken using a standard approach based on a 'Wardrop User Equilibrium', which seeks to minimise travel costs for all vehicles in the network. The Wardrop User Equilibrium is based on the following proposition:
3.6.2. "Traffic arranges itself on congested networks such that the cost of travel on all routes used between each origin-destination pair is equal to the minimum cost of travel and unused routes have equal or greater costs."
3.6.3. The Wardrop User Equilibrium as implemented in SATURN is based on the 'Frank-Wolfe Algorithm', which employs an iterative process. This process is based on successive 'All or Nothing' iterations, which are combined to minimise an ‘Objective Function’. The travel costs are recalculated after each iteration and compared to those from the previous iteration. The process is terminated once successive iteration costs have not changed significantly. This process enables multi-routeing between any origin-destination pair.

### 3.7 RELATIONSHIP WITH DEMAND MODELS AND PUBLIC TRANSPORT ASSIGNMENT MODELS

3.7.1. The SCTM Public Transport assignment model utilises the same MND provided by Telefonica as the basis for the matrices. As discussed in D3 SCTM LMVR (November 2017), movements designated as "Road" in the MND are separated into Cars / LGVs and Bus movements, with the latter matrix then used in the public transport model.
3.7.2. The SCTM VDM utilises time and distance skim matrices from the SCTM Highway Model, as well as skims from the public transport model in order to determine costs and the propensity for modal shift between different motorised modes. The SCTM Demand Model will be capable to taking into account trips which involve car usage at the start of a journey to then access the rail network and therefore create a composite cost for full park and ride trips, and therefore the potential for transport users to switch between modes taking into account congestion will occur on the highway network in the future.
3.7.3. The Suffolk VDM is described in greater detail in the D5 SCTM Demand Model Validation Report (DMVR; November 2017).

## 4 CALIBRATION AND VALIDATION DATA

### 4.1 INTRODUCTION

4.1.1. This section of the report details the sources of the traffic data in Lowestoft which was used for traffic flow and journey time calibration and validation. It also provides details of the screenlines which have been used to assess the ability of the SCTM highway model within Lowestoft to match to observed data across several sites representing key strategic movements within the county.

### 4.2 LOWESTOFT TRAFFIC SURVEY DATA

4.2.1. The main source of traffic data available in Lowestoft was collected in July 2015 and was used to support the Lake Lothing Third Crossing OBC.
4.2.2. WSP subsequently commissioned a range of surveys which are detailed in the D2 SCTM Data Collection Report (November 2017) these were used to calibrate and validate the SCTM at a county-wide level which include survey locations in Waveney District. The Data Collection Report also details the 2015 which has been utilised within the modelling outlined in this report.
4.2.3. Additional traffic survey data was collected in 2016 at the locations listed in Table 1 below, these counts were fully classified turning counts carried out on a single day.

Table 1-2016 MCC Locations

| Count Number | Location |
| :---: | :---: |
| 1 | London Road / Arbor Lane / A12 / Tower Road |
| 2 | Tom Crisp Way / Stradbroke Road / Elm Tree Road |
| 3 | Somerleyton Road / Oulton Street / Hall Lane / Gorleston Road |
| 4 | Yarmouth Road / Gorleston Road |
| 5 | Yarmouth Road / Leisure Way / Foxburrow Hill / Bentley Drive |
| 6 | Yarmouth Road / Corton Road |
| 7 | Millennium Way / Oulton Road / Peto Way |
| 8 | Horn Hill / Maconochie Way / A12 / Waveney Drive |
| 9 | A12 / Corton Long Lane / A12 / Unnamed Road |

4.2.4. The base year of the SCTM is 2016; therefore traffic surveys conducted in 2015 were adjusted using a combination of NTEM 7.2 factors for observed car values and the National Transport Model (NTM) for observed LGV and HGV values. These factors are summarised in Table 2.

Table 2 - Factors applied to 2015 counts

| Peak | Car | LGV | HGV |
| :---: | :---: | :---: | :---: |
| AM Peak | 0.986 | 1.028 | 1.028 |
| Inter Peak | 0.999 | 1.028 | 1.028 |
| PM Peak | 0.989 | 1.007 | 1.007 |

4.2.5. It is considered that the combination of traffic data sources provide a sufficient level of coverage to enable calibration and validation of the SCTM in the Lowestoft area. The sources of the various traffic data surveys is summarised in Table 3.

Table 3 - Lowestoft traffic survey data

| Data Source | Time Period | Number Of <br> Surveys |
| :---: | :---: | :---: |
| TRADS | April 2016 | 2 |
| LTM ATCs | July 2015 | 30 |
| SCC ATCs | April / May / June 2016 | 12 |
| LTM MCTCs | July 2015 | 9 |
| SCTM MCTCs | July 2016 | 6 |
| Total | Combined | 59 |

### 4.3 LOWESTOFT SCREENLINES

4.3.1. Figure 9 details the traffic counts and screenlines which have been used for validation and calibration within Lowestoft. In total there are 4 calibration screenlines and 3 validation screenlines which have been used specifically within Lowestoft.


Figure 9 - Lowestoft Screenlines and Count Data

### 4.4 JOURNEY TIME SURVEYS FOR CALIBRATION AND VALIDATION

4.4.1. The SCTM was validated and calibrated using a range of journey time routes covering key strategic routes across the county. The journey time routes are presented in the D3 SCTM LMVR (February 2017) and utilised 2015/2016 Trafficmaster GPS data. This included four journey time routes in Lowestoft, namely routes 14, 15, 60 and 64.
4.4.2. Additional journey time routes have been derived for Lowestoft to improve the local validation. All existing and new journey time routes were updated using 2015/2016 Trafficmaster GPS data.
4.4.3. Table 4 describes the journey time routes which were used for calibrating and validating the SCTM within Lowestoft. Following the guidance in WebTAG unit M1.2 it has been ensured the journey time routes were kept between 3 km and 15 km .

Table 4 - Lowestoft Journey time routes

| ID | Description | Length |
| :---: | :---: | :---: |
| 14 | A12 Wangford to Pakefield | 14.3 km |
| 15 | A134 Long Melford to Stanningfield | 4.5 km |
| 60 | A1145 through Lowestoft | 9 km |
| 64 | A145 Beccles to Lowestoft | 7.5 km |
| 101 | B1375 Gorleston Road | 4.5 km |
| 102 | A12 Yarmouth Road / Katwijk Way | 8.5 km |
| 103 | A1117 Normanston Drive / A1144 St Peter's Street | 3.3 km |
| 104 | A12 London Road / B1532 London Road South | 3.3 km |
| 105 | B1074 / A1117 Millennium Way / Oulton Road | 3.9 km |
| 106 | A146 Beccles Road / A146 Waveney Drive | 4.8 km |

4.4.4. The $2015 / 2016$ Trafficmaster GPS data was filtered to only include data from the following neutral months:

- September 2016
- October 2016
- November 2016
- March 2017
- April 2017
- May 2017
4.4.5. The data was processed to provide an average weekday (Monday to Thursday) travel time by direction for each peak hour modelled within the SCTM. Suffolk school holidays and bank holidays were excluded from the data used to derive the average travel times.


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4.4.6. The journey time routes used for model calibration and validation are shown in Figure 10.


Figure 10 - Journey Time Routes

## 5 NETWORK DEVELOPMENT

### 5.1 INTRODUCTION

5.1.1. While the SCTM provides a sound basis for countywide scheme appraisal, the suitability of the model within the vicinity of any scheme intervention needs to be reviewed. With regard to the Lake Lothing Third Crossing it was considered additional local coding was required to achieve better calibration and validation results in Lowestoft.
5.1.2. As part of the local validation the model was calibrated to better reflect the 2016 base year conditions, including updating the speed flow curves assigned to links, zone connectors, lane capacities and free flow speeds.

### 5.2 FLOW / DELAY RELATIONSHIPS

5.2.1. Table 5 shows the timings input into the model for the eastern Bascule Bridge. The delay was coded as a signalised node with a single stage, the red time representing when the swing bridge was lifted and with overall cycle time adding up to 3,600 seconds ( 1 hour). No swing bridge delay was coded for the western Mutford Bridge as observations showed the bridge rarely opens to maritime traffic.

Table 5 - Eastern Bascule Bridge timings input into model

| Peak Hour | Green Time <br> (Seconds) | Red Time (Seconds) |
| :---: | :---: | :---: |
| AM peak | 3,373 | 227 |
| Inter peak | 3,344 | 256 |
| PM peak | 3,300 | 300 |

5.2.2. There are two level crossings in the immediate vicinity of the western Mutford Bridge on Bridge Road and Victoria Road. These were represented in the model as a signalised node with a single stage using the timings detailed in Table 6 and Table 7.

Table 6 - Bridge Road level crossing timings input into the model

| Peak Hour | Green Time <br> (Seconds) | Red Time (Seconds) |
| :---: | :---: | :---: |
| AM peak | 950 | 250 |
| Inter peak | 1017 | 183 |
| PM peak | 980 | 220 |

Table 7 - Victoria Road level crossing timings input into the model

| Peak Hour | Green Time <br> (Seconds) | Red Time (Seconds) |
| :---: | :---: | :---: |
| AM peak | 1693 | 107 |
| Inter peak | 1693 | 107 |
| PM peak | 1593 | 207 |

5.2.3. The delays generated by the swing bridges and level crossings were compared to journey time route graphs combined with a comparison of modelled to observed flow to ensure an appropriate level of delay was applied.

### 5.3 TIDAL FLOW LANE

5.3.1. Between the A12 Belvedere Road / London Road S and A12 Station Square / Commercial Road the direction of travel for the central lane changes by time of day. In the AM peak this was coded with the central lane allowing northbound movements. In the inter peak and PM the central lane was coded to allow southbound movements.

### 5.4 DETAILED SATURATION FLOWS CALCULATIONS

5.4.1. The SCTM uses default measurements for junction saturation flows based upon typical junction layouts and these are set out within D3 SCTM LMVR (November 2017). In addition a number of junctions within the Lowestoft model area have individually calculated saturation flows from the coding of the original Lowestoft Transport model.
5.4.2. As part of this model update additional measurements were carried out using to derive saturation flows for the following local key junctions in Lowestoft:

- Normanston Drive / Peto Way
- Normanston Drive / Bridge Road / B1375
- Bridge Road / A146
- A12/A146
- A12 / Mill Road
- A12/A1145


### 5.5 SUMMARY

5.5.1. The inclusion of delays associated with swing bridges and level crossings, directionality of the tidal flow lane and specifically measured saturation flows at key junctions ensures the SCTM better replicates the local highway network within Lowestoft for the purposes of calibration and validation in the local area.
5.5.2. The Changes made as part of this local model validation have been incorporated into the wider county mode validation that are set out within D3 SCTM LMVR (November 2017).

## 6 TRIP MATRIX DEVELOPMENT

### 6.1 INTRODUCTION

6.1.1. The main source of the demand from which matrices have been derived within the SCTM is from Mobile Network Data (MND) provided by Telefonica. This data has been combined with a synthetic matrix derived from 2011 Census Journey to Work data and the National Travel Survey (NTS) which infills the MND for short distance trips $(0-2 \mathrm{~km})$ which are not present in the MND.
6.1.2. The D3 SCTM LMVR (November 2017) provides details of the verification which has been undertaken for the MND and the methodology which has been undertaken to derive the matrices.

### 6.2 TRIP MATRIX ESTIMATION

6.2.1. Matrix estimation was carried on the prior matrix during the calibration and validation process. Table 8 compares the prior matrix totals to the post matrix-estimation totals.

Table 8 - Prior and Post ME Matrix Totals

| User Class | AM Peak Hour <br> $(0800-0900)$ |  | Inter Peak Avg Hour <br> $(1000-1600)$ |  | PM Peak Hour <br> $(1700-1800)$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Prior | Post ME | Prior | Post ME | Prior | Post ME |
| UC1 - Car HBW IB | 3,046 | 3,719 | 7,557 | 7,899 | 61,232 | 53,969 |
| UC2 - Car HBW OB | 64,460 | 56,289 | 6,322 | 6,562 | 1,315 | 1,485 |
| UC3 - Car HEB IB | 302 | 316 | 956 | 1,036 | 4,041 | 4,031 |
| UC4 - Car HEB OB | 4,433 | 4,207 | 853 | 892 | 958 | 967 |
| UC5 - Car NHEB | 7,588 | 8,186 | 6,209 | 6,846 | 7,165 | 7,662 |
| UC6 - Car HBO IB | 4,928 | 5,510 | 26,251 | 28,444 | 40,921 | 37,366 |
| UC7 - Car HBO OB | 37,957 | 34,885 | 26,809 | 28,389 | 16,164 | 16,567 |
| UC8 - Car NHBO | 6,298 | 6,037 | 13,800 | 15,004 | 12,191 | 11,579 |
| UC9 - LGV | 14,806 | 12,629 | 11,563 | 10,611 | 11,689 | 10,292 |
| UC10 - HGV | 4,458 | 8,810 | 4,631 | 9,070 | 4,988 | 6,085 |
| Total | $\mathbf{1 4 8 , 2 7 5}$ | $\mathbf{1 4 0 , 5 8 7}$ | $\mathbf{1 0 4 , 9 5 1}$ | $\mathbf{1 1 4 , 7 5 2}$ | $\mathbf{1 6 0 , 6 6 5}$ | $\mathbf{1 5 0 , 0 0 4}$ |

## 7 ASSIGNMENT CALIBRATION AND VALIDATION

### 7.1 INTRODUCTION

7.1.1. This section preens the local model validation statistics for the Lowestoft study area. In general, the criteria detailed in this section of the report have been drawn from DfT TAG Unit M3.1, section 3.2 (January 2014). All the models reported on in this report are results from assignments in SATURN version 11.3.12w.

### 7.2 CONVERGENCE

7.2.1. An element of calibrating the model is ensuring that a satisfactory convergence is achieved. Model convergence is needed to ensure results remain stable between successive iterations of the model assignments. This is particularly important when model outputs are used to inform the economic benefits of scheme appraisal, as it is critical that calculated benefits arise from the impact of the scheme and not as a result of difference in convergence.
7.2.2. In accordance with criteria set out in TAG Unit M3.1 (January 2014), the parameters \%Flow, \%GAP and Delta ( $\delta$ ) have been monitored to determine the level of convergence. \%Flow measures the proportion of links in the network with flows changing by less than $1 \%$ from the previous iteration. $\delta$ is the difference between costs on chosen routes and costs on minimum cost paths. \%GAP is a generalisation of the $\delta$ function to include the interaction effects within the simulation.
7.2.3. The convergence criteria used to assess when a model is considered to have converged is shown in Table 9.

Table 9 - Convergence criteria

| Measure of Convergence | Acceptable Value |
| :---: | :---: |
| 'Delta' and \%GAP | Less than 0.1\% or at least stable with convergence fully <br> documented and all other criteria met |
| Percentage of links with flow change <br> $<1 \%$ | Four consecutive iterations greater than $98 \%$ |
| Percentage of links with cost change <br> $<1 \%$ | Four consecutive iterations greater than $98 \%$ |
| Percentage change in total user <br> costs | Four consecutive iterations less than $0.1 \%$ |

7.2.4. TAG Unit M3.1 (January 2014) indicates that delta ( $\delta$ ) and \%GAP values of less than $0.1 \%$ is the most fundamental indicator of model convergence and should be achieved as a minimum
7.2.5. Table 10, Table 11 and Table 12 show the convergence results against WebTAG criteria for each peak hour modelled.

Table 10-AM peak convergence results

| Iteration | Delta | \%Flow | \%Gap |
| :---: | :---: | :---: | :---: |
| 18 | 0.0179 | 98 | 0.014 |
| 19 | 0.0161 | 98.4 | 0.012 |
| 20 | 0.0147 | 98.7 | 0.011 |
| 21 | 0.0139 | 99 | 0.0093 |

Table 11 - Inter peak convergence results

| Iteration | Delta | \%Flow | \%Gap |
| :---: | :---: | :---: | :---: |
| 16 | 0.0062 | 98.4 | 0.023 |
| 17 | 0.0063 | 98.8 | 0.01 |
| 18 | 0.0064 | 98.7 | 0.0065 |
| 19 | 0.0044 | 98.4 | 0.0056 |

Table 12 - PM peak convergence results

| Iteration | Delta | \%Flow | \%Gap |
| :---: | :---: | :---: | :---: |
| 20 | 0.0182 | 98.1 | 0.034 |
| 21 | 0.0206 | 98.5 | 0.035 |
| 22 | 0.0263 | 98.2 | 0.034 |
| 23 | 0.0196 | 98.5 | 0.03 |

7.2.6. The model convergence results show the SCTM successfully converges to the WebTAG requirements in all three peaks.

### 7.3 SCREENLINE VALIDATION CRITERIA

7.3.1. Screenline validation is undertaken as a check on the trip matrix, and is assessed in terms of the percentage difference between observed and modelled flows as shown in Table 13.

Table 13-Screenline acceptability

| Criteria | Description of Criteria | Acceptability <br> Guideline |
| :---: | :---: | :---: |
| 1 | Differences between modelled flows and counts should be <br> less than $5 \%$ of counts | All or nearly all <br> screenlines |

7.3.2. Screenlines are presented for each time period, for all vehicle user classes, namely Cars, LGVs and HGVs. The main body of this report provides screenline analysis in terms of the total overall flow including all vehicle types. Breakdown of screenline performance by vehicle type is detailed in Appendix D for the final model assignment.

### 7.4 SCREENLINE VALIDATION PERFORMANCE

7.4.1. This section presents screenline calibration and validation by route and direction for each time period and an overall summary of model performance.

## AM PEAK SCREENLINE PERFORMANCE

7.4.2. Table 14 shows the performance in terms of percentage difference between modelled and observed flow for calibration and validation screenlines in the AM peak.

Table 14 - AM peak screenline performance

| ID | Description | Dir | Type | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | South Lowestoft | Inbound | Calibration | 1421 | 1452 | 2\% |
| 2 | South Lowestoft | Outbound | Calibration | 1330 | 1364 | 3\% |
| 3 | Lowestoft Screenline $1-N B$ | Northbound | Calibration | 2207 | 2207 | 0\% |
| 4 | Lowestoft Screenline $1-\mathrm{SB}$ | Southbound | Calibration | 1685 | 1693 | 0\% |
| 5 | Lowestoft Screenline $2-N B$ | Northbound | Calibration | 2866 | 2872 | 0\% |
| 6 | Lowestoft Screenline $2-S B$ | Southbound | Calibration | 2095 | 2114 | 1\% |
| 7 | North Lowestoft | Inbound | Calibration | 1194 | 1191 | 0\% |
| 8 | North Lowestoft | Outbound | Calibration | 1454 | 1452 | 0\% |
| 9 | Lowestoft Screenline $3 \text { - EB }$ | Eastbound | Validation | 1747 | 1626 | -7\% |
| 10 | Lowestoft Screenline $3-W B$ | Westbound | Validation | 1741 | 1541 | -11\% |
| 11 | Lowestoft Screenline $4-N B$ | Northbound | Validation | 2419 | 2308 | -5\% |
| 12 | Lowestoft Screenline $4-\text { SB }$ | Southbound | Validation | 1732 | 1883 | 9\% |
| 13 | Lowestoft Screenline $5-\text { NB }$ | Northbound | Validation | 1359 | 1344 | -1\% |
| 14 | Lowestoft Screenline $5-\text { SB }$ | Southbound | Validation | 1024 | 1052 | 3\% |

7.4.3. Table 14 shows all but 3 of the screenlines pass WebTAG guidance criteria. Of those that fail, validation screenline 3 eastbound shows a modelled flow difference of -7\%, Validation screenline 3 westbound shows a modelled flow difference of $-11 \%$ and Validation screenline 4 southbound shows a modelled flow difference of $9 \%$.

## INTER PEAK SCREENLINE PERFORMANCE

7.4.4. Table 15 shows the performance of the calibration and validation screenlines in the inter-peak.

Table 15 - Inter peak screenline performance

| ID | Description | Dir | Type | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | South Lowestoft | Inbound | Calibration | 1326 | 1339 | $1 \%$ |
| 2 | South Lowestoft | Outbound | Calibration | 1347 | 1361 | $1 \%$ |
| 3 | Lowestoft Screenline <br> $1-$ NB | Northbound | Calibration | 1826 | 1828 | $0 \%$ |
| 4 | Lowestoft Screenline <br> $1-$ SB | Southbound | Calibration | 1873 | 1876 | $0 \%$ |
| 5 | Lowestoft Screenline <br> $2-$ NB | Northbound | Calibration | 2473 | 2481 | $0 \%$ |
| 6 | Lowestoft Screenline <br> $2-$ SB | Southbound | Calibration | 2635 | 2628 | $0 \%$ |
| 7 | North Lowestoft | Calibration | 1052 | 1030 | $-2 \%$ |  |
| 8 | North Lowestoft | Calibration | 1027 | 1009 | $-2 \%$ |  |
| 9 | Lowestoft Screenline <br> $3-$ EB | Eastbound | Validation | 1665 | 1587 | $-5 \%$ |
| 10 | Lowestoft Screenline <br> $3-$ WB | Westbound | Validation | 1662 | 1595 | $-4 \%$ |
| 11 | Lowestoft Screenline <br> $4-$ NB | Northbound | Validation | 1857 | 2036 | $10 \%$ |
| 12 | Lowestoft Screenline <br> $4-$ SB | Southbound | Validation | 2013 | 2188 | $9 \%$ |
| 13 | Lowestoft Screenline <br> $5-$ NB | Lowestoft Screenline <br> $5-$ SB | Southbound | Validation | 940 | 936 |

7.4.5. Table 15 shows for nearly all screenlines, modelled flows are within $5 \%$ of observed flows. The exception to this is Validation Screenline 4 northbound with modelled flow 10\% higher compared to observed flow and Validation Screenline 4 southbound with a modelled flow $9 \%$ higher than observed.

## PM PEAK SCREENLINE PERFORMANCE

7.4.6. Table 16 details the performance of the screenlines in the PM peak.

Table 16-PM peak screenline performance

| ID | Description | Dir | Type | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | South Lowestoft | Inbound | Calibration | 1792 | 1801 | 1\% |
| 2 | South Lowestoft | Outbound | Calibration | 1517 | 1536 | 1\% |
| 3 | Lowestoft Screenline 1 - NB | Northbound | Calibration | 2145 | 2145 | 0\% |
| 4 | Lowestoft Screenline 1-SB | Southbound | Calibration | 2284 | 2292 | 0\% |
| 5 | Lowestoft <br> Screenline 2 - NB | Northbound | Calibration | 2471 | 2473 | 0\% |
| 6 | Lowestoft <br> Screenline 2 - SB | Southbound | Calibration | 3219 | 3228 | 0\% |
| 7 | North Lowestoft | Inbound | Calibration | 1851 | 1829 | -1\% |
| 8 | North Lowestoft | Outbound | Calibration | 1211 | 1207 | 0\% |
| 9 | Lowestoft <br> Screenline 3 - EB | Eastbound | Validation | 2021 | 1899 | -6\% |
| 10 | Lowestoft Screenline 4 - WB | Westbound | Validation | 1725 | 1784 | 3\% |
| 11 | Lowestoft Screenline 4 - NB | Northbound | Validation | 2041 | 2113 | 4\% |
| 12 | Lowestoft Screenline 4 - SB | Southbound | Validation | 2528 | 3001 | 19\% |
| 13 | Lowestoft Screenline 5 - NB | Northbound | Validation | 1129 | 1116 | -1\% |
| 14 | Lowestoft Screenline 5 - SB | Southbound | Validation | 1598 | 1699 | 6\% |

7.4.7. Table 16shows for the majority of screenlines there minimal differences between the observed and modelled flow. Validation Screenline 3 east bound has a modelled flow $6 \%$ less than observed while Validation Screenline 4 southbound is $19 \%$ higher compared to the modelled flow.

## SCREENLINE SUMMARY PERFORMANCE

7.4.8. Table 17 shows the overall screenline performance by modelled time period.

Table 17-Overall Screenline Performance

| Time Period | Total Screenlines | Modelled Flow <br> On Screenlines <br> Within 5\% of <br> Observed | Modelled (s) |
| :---: | :---: | :---: | :---: |
| AM Peak | 14 | 11 | $79 \%$ |
| Inter Peak | 14 | 12 | $86 \%$ |
| PM Peak | 14 | 11 | $79 \%$ |

7.4.9. Table 17 shows in the inter peak and PM peak, nearly all screenlines ( $86 \%$ ) have modelled flow within $5 \%$ of the observed flow. In the AM peak, there are three screenlines which show a difference of greater than $5 \%$, meaning 79\% of screenlines pass the threshold.
7.4.10. Appendix A provides further detail of the performance of the individual counts which comprise the Lowestoft screenlines.

### 7.5 ASSIGNMENT VALIDATION CRITERIA

7.5.1. This section presentation the link flow and turning movement validation statistics for the Lowestoft study area and is in line with TAG Unit M3.1 (January 2014).
7.5.2. Measures used for link validation are:

- Absolute and percentage differences between absolute and modelled flows
- GEH statistic
7.5.3. The GEH statistic is a modified Chi-squared statistic incorporating both relative and absolute errors, defined as follows:

$$
G E H=\sqrt{\frac{(M-C)^{2}}{(M+C) / 2}}
$$

7.5.4. The link flow and turning movement validation criteria are shown in Table 18.

Table 18 - Link acceptability

| Criteria | Description of Criteria | Acceptability <br> Guideline |
| :---: | :---: | :---: |
| 1 | Individual flows within 100 veh/hr of counts for flows less than <br> $700 \mathrm{veh} / \mathrm{hr}$ | $>85 \%$ of cases |
|  | Individual flows within $15 \%$ of counts for flows from 700 <br> veh/hr to 2,700 veh/hr | $>85 \%$ of cases |
|  | Individual flows within 400 veh/hr of counts for flows more <br> than 2,700 veh/hr | $>85 \%$ of cases |
| 2 | GEH < for individual flows | $>85 \%$ of cases |

7.5.5. Both link flows and turning movements are presented using the above criteria, although turning movements are not generally expected to fully meet the criteria.

### 7.6 LOWESTOFT ASSIGNMENT VALIDATION PERFORMANCE

7.6.1. The calibration and validation performance for Lowestoft is presented in this section. The performance of the full country model is provided in section 7.5.
7.6.2. The calibration and validation results for all user classes in the AM peak are shown in Table 19.

| Criteria and Measure |  | Acceptability Guideline | Calibration |  |  | Validation |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total | Meet | \% | Total | Meet | \% |
| $\begin{aligned} & <700 \\ & \text { vph } \end{aligned}$ | $\begin{gathered} \pm 100 \\ \text { vph } \end{gathered}$ |  | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 149 | 138 | 93\% | 11 | 10 | 91\% |
| $\begin{gathered} 700 \\ 2,700 \\ \text { vph } \end{gathered}$ | $\pm 15 \%$ | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 9 | 9 | 100\% | 7 | 7 | 100\% |
| $\begin{gathered} >2,700 \\ \text { vph } \end{gathered}$ | $\begin{gathered} \pm 400 \\ \text { vph } \end{gathered}$ | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 0 | 0 | 0\% | 0 | 0 | 0\% |

7.6.3. Table 19 shows that the model exceeds WebTAG guidance with a minimum of $91 \%$ of counts achieving criteria in all categories. Table 20 shows the breakdown of calibration and validation count performance by GEH band for the AM peak.

Table 20-AM Peak - All User Classes - GEH Test Calibration and Validation - Lowestoft

| GEH Range | Calibration |  | Validation |  | Combined |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| GEH < 2 | 120 | $76 \%$ | 11 | $61 \%$ | 131 | $74 \%$ |
| GEH < 4 | 136 | $86 \%$ | 15 | $83 \%$ | 151 | $86 \%$ |
| GEH < 6 | 147 | $93 \%$ | 17 | $94 \%$ | 164 | $93 \%$ |
| GEH < 8 | 150 | $95 \%$ | 17 | $94 \%$ | 167 | $95 \%$ |
| GEH < 10 | 156 | $99 \%$ | 17 | $94 \%$ | 173 | $98 \%$ |
| GEH <5 | 142 | $90 \%$ | 16 | $89 \%$ | 158 | $90 \%$ |

7.6.4. Table 20 shows that calibration, validation and combined count performance exceeds the WebTAG guidance with a minimum of $89 \%$ of counts achieving criteria in all categories This rises to $93 \%$ for a GEH below 6 , and $95 \%$ for a GEH below 8 , this implies there are a number of counts falling marginally outside the WebTAG requirement of a GEH below 5 .
7.6.5. The calibration and validation results for all user classes in the Inter peak are shown in Table 21.

Table 21 - Inter Peak - All User Classes - Flow Test Calibration and Validation - Lowestoft

| Criteria and <br> Measure | Acceptability <br> Guideline | Calibration |  |  | Total <br> Counts |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Meet <br> Criteria | $\%$ | Total <br> Counts | Meet <br> Criteria | $\%$ |  |  |

7.6.6. Table 21 shows that the model exceeds WebTAG guidance for calibration with a minimum of $95 \%$ of links meeting criteria but is marginally lower for validation with a minimum of $83 \%$ achieving criteria in all categories.
7.6.7. Table 22 details the performance of the calibration and validation counts in the inter peak by GEH band.

Table 22 - Inter Peak - All User Classes - GEH Test Calibration and Validation - Lowestoft

| GEH Range | Calibration |  | Validation |  | Combined |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| GEH < | 120 | $76 \%$ | 5 | $28 \%$ | 125 | $71 \%$ |
| GEH < 4 | 136 | $86 \%$ | 15 | $83 \%$ | 151 | $86 \%$ |
| GEH < 6 | 147 | $93 \%$ | 15 | $83 \%$ | 162 | $92 \%$ |
| GEH < 8 | 151 | $96 \%$ | 18 | $100 \%$ | 169 | $96 \%$ |
| GEH < 10 | 154 | $97 \%$ | 18 | $100 \%$ | 172 | $98 \%$ |
| GEH <5 | 142 | $90 \%$ | 15 | $83 \%$ | 157 | $89 \%$ |

7.6.8. Table 22 shows that calibration and combined count performance exceeds the WebTAG guidance with a minimum of $89 \%$ of counts achieving criteria but is marginally lower for validation with $83 \%$ achieving criteria. This rises to $100 \%$ for a GEH below 8 , which demonstrates there are a number of validation counts falling close to the WebTAG guidance for a GEH below 5 .
7.6.9. The calibration and validation results for all user classes in the PM peak are shown in Table 23.

7.6.10. Table 23 shows that the model is close to WebTAG guidance for calibration with a minimum of $83 \%$ of links meeting criteria but is lower for validation with a minimum of $70 \%$ achieving criteria in all categories. The validation would need 2 additional inks with flow under 700 and 1 link over 700 to pass criteria in order to fully meet the guidance.
7.6.11. Table 24 shows the GEH performance by band for the calibration and validation counts in the PM peak.

Table 24 - PM Peak - All User Classes - GEH Test Calibration and Validation - GEH

| GEH Range | Calibration |  | Validation |  | Combined |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| GEH < 2 | 113 | $72 \%$ | 8 | $44 \%$ | 121 | $69 \%$ |
| GEH < 4 | 138 | $87 \%$ | 11 | $61 \%$ | 149 | $85 \%$ |
| GEH < 6 | 145 | $92 \%$ | 16 | $89 \%$ | 161 | $91 \%$ |
| GEH < 8 | 151 | $96 \%$ | 17 | $94 \%$ | 168 | $95 \%$ |
| GEH < 10 | 155 | $98 \%$ | 18 | $100 \%$ | 173 | $98 \%$ |
| GEH <5 | 141 | $89 \%$ | 14 | $78 \%$ | 155 | $88 \%$ |

7.6.12. Table 24 shows that calibration and combined count performance exceeds the WebTAG guidance with a minimum of $88 \%$ of counts achieving criteria but is lower for validation with $78 \%$ achieving criteria. This rises to $89 \%$ for a GEH below 6 , which demonstrates there are a number of validation counts falling marginally outside the WebTAG guidance of a GEH below 5 .
7.6.13. Appendix $B$ contains details of the performance for each individual link count used in validation or calibration in terms of GEH and flow by peak hour modelled. Overall it is consider the mode provides a good calibration and validation across all time periods within the Lowestoft model area.

### 7.7 COUNTYWIDE ASSIGNMENT VALIDATION PERFORMANCE

7.7.1. The calibration and validation results for all user classes in the AM peak are shown in Table 25.

| Criteria and Measure |  | Acceptability Guideline | Calibration |  |  | Validation |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total | Meet | \% | Total | Meet | \% |
| < 700 vph | $\pm 100 \mathrm{vph}$ |  | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 1022 | 884 | 86\% | 200 | 140 | 70\% |
| $\begin{gathered} 700 \text { - } \\ \text { 2,700 vph } \end{gathered}$ | $\pm 15 \%$ | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 195 | 173 | 89\% | 54 | 38 | 70\% |
| $\begin{gathered} >2,700 \\ \text { vph } \end{gathered}$ | $\pm 400 \mathrm{vph}$ | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 7 | 7 | 100\% | 0 | 0 | 0\% |

7.7.2. Table 25 shows that the model meets WebTAG guidance for calibration with a minimum of $86 \%$ of links meeting criteria but is lower for validation with a minimum of $70 \%$ meeting criteria. Given the analysis provided in section 7.4, it has been demonstrated the majority of links which fail criteria are located outside of the area of interest and are not significant on the assessment of this scheme.
7.7.3. Table 26 shows the breakdown of calibration and validation count performance by GEH band for the AM peak.

Table 26 - AM Peak - All User Classes - GEH Test Calibration and Validation - Overall County Level

| GEH Range | Calibration |  | Validation |  | Combined |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| GEH < 2 | 780 | $64 \%$ | 11 | $4 \%$ | 791 | $54 \%$ |
| $\mathrm{GEH}<4$ | 943 | $77 \%$ | 138 | $54 \%$ | 1081 | $73 \%$ |
| $\mathrm{GEH}<6$ | 1059 | $87 \%$ | 186 | $73 \%$ | 1245 | $84 \%$ |
| $\mathrm{GEH}<8$ | 1113 | $91 \%$ | 209 | $82 \%$ | 1322 | $89 \%$ |
| $\mathrm{GEH}<10$ | 1157 | $95 \%$ | 223 | $88 \%$ | 1380 | $93 \%$ |
| $\mathrm{GEH}<5$ | 1011 | $83 \%$ | 166 | $65 \%$ | 1177 | $80 \%$ |

7.7.4. Table 26 shows that calibration and combined count performance are close to WebTAG guidance with a minimum of $80 \%$ of counts achieving criteria but is lower for validation with $65 \%$ achieving criteria. Combined $84 \%$ have a GEH less than 6 showing a number of counts marginally miss the criteria.
7.7.5. The calibration and validation results for all user classes in the Inter peak are shown in Table 27.

Table 27 - Inter Peak - All User Classes - Flow Test Calibration and Validation - Overall County Level

| Criteria and Measure |  | Acceptability Guideline | Calibration |  |  | Validation |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total | Meet |  | Total | Meet |  |
| < 700 vph | $\pm 100$ vph |  | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 1085 | 1007 | 93\% | 228 | 177 | 78\% |
| $\begin{gathered} 700 \text { - } \\ 2,700 \mathrm{vph} \end{gathered}$ | $\pm 15 \%$ | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 139 | 129 | 93\% | 26 | 21 | 81\% |
| $\begin{gathered} >2,700 \\ \text { vph } \end{gathered}$ | $\pm 400 \mathrm{vph}$ | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 0 | 0 | 0\% | 0 | 0 | 0\% |

7.7.6. Table 27 shows that the model meets WebTAG guidance for calibration with a minimum of $93 \%$ of links meeting criteria but is lower for validation with a minimum of $78 \%$ achieving criteria. Given the analysis provided in section 7.4, it has been demonstrated the majority of links which fail criteria are located outside of the area of interest and are not significant on the assessment of this scheme.
7.7.7. Table 28 details the performance of the calibration and validation counts in the inter peak by GEH band.

Table 28 - Inter Peak - All User Classes - GEH Test Calibration and Validation - Overall County Level

| GEH Range | Calibration |  | Validation |  | Combined |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathrm{GEH}<2$ | 846 | $69 \%$ | 5 | $2 \%$ | 851 | $58 \%$ |
| $\mathrm{GEH}<4$ | 999 | $82 \%$ | 164 | $65 \%$ | 1163 | $79 \%$ |
| $\mathrm{GEH}<6$ | 1099 | $90 \%$ | 196 | $77 \%$ | 1295 | $88 \%$ |
| $\mathrm{GEH}<8$ | 1159 | $95 \%$ | 220 | $87 \%$ | 1379 | $93 \%$ |
| $\mathrm{GEH}<10$ | 1195 | $98 \%$ | 234 | $92 \%$ | 1429 | $97 \%$ |
| $\mathrm{GEH}<5$ | 1053 | $86 \%$ | 181 | $71 \%$ | 1234 | $83 \%$ |

7.7.8. Table 28 shows that calibration meets WebTAG guidance with $86 \%$ of counts achieving criteria but is lower for combined at $83 \%$ and validation $71 \%$ achieving criteria. Combined $88 \%$ have a GEH less than 6 showing a number of counts marginally miss the criteria and validation has $87 \%$ with a GEH of less than 8 .
7.7.9. The calibration and validation results for all user classes in the PM peak are shown in Table 29.

| Criteria and Measure |  | Acceptability Guideline | Calibration |  |  | Validation |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total | Meet | \% | Total | Meet | \% |
| < 700 vph | $\pm 100 \mathrm{vph}$ |  | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 985 | 859 | 87\% | 190 | 125 | 66\% |
| $\begin{gathered} 700 \text { - } \\ \text { 2,700 vph } \end{gathered}$ | $\pm 15 \%$ | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 230 | 200 | 87\% | 64 | 45 | 70\% |
| $\begin{gathered} >2,700 \\ \text { vph } \end{gathered}$ | $\pm 400 \mathrm{vph}$ | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 9 | 9 | 100\% | 0 | 0 | 0\% |

7.7.10. Table 29 shows that the model meets WebTAG guidance for calibration with $87 \%$ of links meeting criteria. It is lower for validation with $66 \%$ achieving criteria. Given the analysis provided in section 7.4 , it has been demonstrated the majority of links which fail criteria are located outside of the area of interest and are not significant on the assessment of this scheme.
7.7.11. Table 30 details the performance of the calibration and validation counts in the PM peak by GEH band.

Table 30 - PM Peak - All User Classes - GEH Test Calibration and Validation - Overall County Level

| GEH Range | Calibration |  | Validation |  | Combined |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| GEH <2 | 773 | $63 \%$ | 8 | $3 \%$ | 781 | $53 \%$ |
| GEH < 4 | 949 | $78 \%$ | 142 | $56 \%$ | 1091 | $74 \%$ |
| GEH < 6 | 1051 | $86 \%$ | 187 | $74 \%$ | 1238 | $84 \%$ |
| GEH < 8 | 1109 | $91 \%$ | 216 | $85 \%$ | 1325 | $90 \%$ |
| GEH < 10 | 1152 | $94 \%$ | 225 | $89 \%$ | 1377 | $93 \%$ |
| GEH <5 | 1007 | $82 \%$ | 162 | $64 \%$ | 1169 | $79 \%$ |

7.7.12. Table 30 shows that calibration is close to WebTAG guidance with $82 \%$ of counts achieving criteria but is lower for combined at $79 \%$ and validation $64 \%$ achieving criteria. Combined $84 \%$ have a GEH less than 6 showing a number of counts marginally miss the criteria and validation has $85 \%$ with a GEH of less than 8 .

### 7.8 MANUAL CLASSIFIED TURNING COUNTS

7.8.1. Further validation of the Lowestoft modelled area was carried out by comparing the GEH for all turns at the junctions for which Manual Classified Turning Counts were commissioned for the 2015 LTM. WebTAG guidance indicates that GEH and flow should match minimum requirements at a link based level, rather than at the level of individual turns, therefore the analysis in this section is a general indicator only of how well the SCTM matches to turning movements at the surveyed junctions.
7.8.2 Table 31 provides a summary of the GEH performance of the turns at the manual classified turning count locations in the AM peak.

Table 31-: AM peak manual classified turning count performance

| ID | Description | GEH $<$ <br> 5 | GEH < <br> 7.5 | GEH $<$ <br> 10 |
| :---: | :---: | :---: | :---: | :---: |
| 1 | London Road/Arbor Lane/A12/Tower Road | $92 \%$ | $100 \%$ | $100 \%$ |
| 2 | Tom Crisp Way/Stadbroke Road/Elm Tree Road | $97 \%$ | $97 \%$ | $100 \%$ |
| 3 | Somerleyton Road/Oulton Street/Hall Lane/Gorleston Road | $88 \%$ | $94 \%$ | $100 \%$ |
| 4 | Yarmouth Road/Gorleston Road | $100 \%$ | $100 \%$ | $100 \%$ |
| 5 | Yarmouth Road/Leisure Way/Foxburrow Hill/Bentley Drive | $63 \%$ | $100 \%$ | $100 \%$ |
| 6 | Yarmouth Road/Corton Road | $89 \%$ | $100 \%$ | $100 \%$ |
| 7 | Millennium Way/Oulton Road/Peto Way | $75 \%$ | $88 \%$ | $94 \%$ |
| 8 | Horn Hill/Maconochie Way/A12/Waveney Drive | $100 \%$ | $100 \%$ | $100 \%$ |
| 9 | A12/Corton Long Lane/A12/Unnamed Road | $92 \%$ | $100 \%$ | $100 \%$ |

7.8.3. Table 31 shows the majority of junctions achieve $100 \%$ of turns with a GEH below 10 and $78 \%$ have a GEH below 5 .
7.8.4. Table 32 provides a summary of the GEH performance of the turns at the manual classified turning count locations in the inter peak.

Table 32 - Inter peak manual classified turning count performance

| ID | Description | GEH $<$ <br> 5 | GEH $<$ <br> 7.5 | GEH $<$ <br> 10 |
| :---: | :---: | :---: | :---: | :---: |
| 1 | London Road/Arbor Lane/A12/Tower Road | $96 \%$ | $100 \%$ | $100 \%$ |
| 2 | Tom Crisp Way/Stadbroke Road/Elm Tree Road | $97 \%$ | $100 \%$ | $100 \%$ |
| 3 | Somerleyton Road/Oulton Street/Hall Lane/Gorleston Road | $100 \%$ | $100 \%$ | $100 \%$ |
| 4 | Yarmouth Road/Gorleston Road | $100 \%$ | $100 \%$ | $100 \%$ |
| 5 | Yarmouth Road/Leisure Way/Foxburrow Hill/Bentley Drive | $81 \%$ | $100 \%$ | $100 \%$ |
| 6 | Yarmouth Road/Corton Road | $100 \%$ | $100 \%$ | $100 \%$ |
| 7 | Millennium Way/Oulton Road/Peto Way | $81 \%$ | $94 \%$ | $100 \%$ |
| 8 | Horn Hill/Maconochie Way/A12/Waveney Drive | $100 \%$ | $100 \%$ | $100 \%$ |
| 9 | A12/Corton Long Lane/A12/Unnamed Road | $100 \%$ | $100 \%$ | $100 \%$ |

7.8.5. Table 32 shows all junctions achieve $100 \%$ of turns with a GEH below 10 and $81 \%$ have a GEH below 5 .
7.8.6. Table 33 provides a summary of the GEH performance of the turns at the manual classified turning count locations in the PM peak.

Table 33 - PM peak manual classified turning count performance
$\left.\left.\begin{array}{|c|c|c|c|c|}\hline \text { ID } & \text { Description } & \text { GEH < } & \text { GEH < } \\ 5\end{array}\right) \begin{array}{c}\text { GEH < } \\ 10\end{array}\right]$
7.8.7. Table 33 shows the majority of junctions achieve $100 \%$ of turns with a GEH below 10 and $69 \%$ have a GEH below 5 .

### 7.9 JOURNEY TIME VALIDATION CRITERIA

7.9.1. The criteria for journey time validation is stipulated within WebTAG and presented in Table 34.

Table 34 - Journey time acceptability

| Criteria | Description of Criteria | Acceptability <br> Guideline |
| :---: | :---: | :---: |
| 1 | Modelled times along routes should be within $15 \%$ of <br> surveyed times (or one minute, if higher than $15 \%$ ) | $>85 \%$ of routes |

7.9.2. The model does not feature different speed/flow relationships or link speeds for different vehicle types with the comparisons being presented for all vehicles combined. Comparisons are presented separately for all modelled time periods.

### 7.10 JOURNEY TIME VALIDATION PERFORMANCE

7.10.1. This section presents journey time validation by route and direction for each time period and an overall summary of model performance.

## AM PEAK JOURNEY TIME PERFORMANCE

7.10.2. Table 35 provides a summary of the performance by each journey time route and direction in the AM peak.

Table 35-AM peak journey time route performance

| ID | Name | Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 14 - Northbound | 766 | 760 | -6 | -1\% | Yes |
| 2 | 14 - Southbound | 782 | 751 | -31 | -4\% | Yes |
| 3 | 15 - Northbound | 544 | 538 | -6 | -1\% | Yes |
| 4 | 15 - Southbound | 479 | 479 | 0 | 0\% | Yes |
| 5 | 60 - Northbound | 1025 | 894 | -131 | -13\% | Yes |
| 6 | 60 - Southbound | 956 | 889 | -67 | -7\% | Yes |
| 7 | 64 - Eastbound | 316 | 306 | -10 | -3\% | Yes |
| 8 | 64 - Westbound | 311 | 317 | 6 | 2\% | Yes |
| 9 | 101 - Northbound | 356 | 412 | 56 | 16\% | Yes |
| 10 | 101 - Southbound | 412 | 361 | -51 | -12\% | Yes |
| 11 | 102 - Northbound | 701 | 662 | -39 | -6\% | Yes |
| 12 | 102 - Southbound | 753 | 668 | -85 | -11\% | Yes |
| 13 | 103 - Eastbound | 383 | 375 | -8 | -2\% | Yes |
| 14 | 103 - Westbound | 372 | 428 | 56 | 15\% | Yes |
| 15 | 104 - Northbound | 543 | 518 | -25 | -5\% | Yes |
| 16 | 104 - Southbound | 390 | 401 | 11 | 3\% | Yes |
| 17 | 105 - Eastbound | 505 | 462 | -43 | -8\% | Yes |
| 18 | 105 - Westbound | 447 | 365 | -82 | -18\% | No |
| 19 | 106 - Eastbound | 620 | 480 | -140 | -23\% | No |
| 20 | 106 - Westbound | 515 | 502 | -13 | -3\% | Yes |

7.10.3. Table 35 shows all but 2 routes pass WebTAG guidance criteria. Of the two routes that fail, Route 105 Westbound falls marginally outside of the minimum criteria of $15 \%$ with a modelled journey time $18 \%$ slower than the observed and Route 106 Eastbound has a modelled journey time $23 \%$ faster than the observed.

## INTER PEAK JOURNEY TIME PERFORMANCE

7.10.4. Table 36 provides a summary of the performance by each journey time route and direction in the Inter peak.

Table 36 - Inter peak journey time route comparison

| ID | Name | Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 14 - Northbound | 814 | 753 | -61 | -7\% | Yes |
| 2 | 14 - Southbound | 787 | 746 | -41 | -5\% | Yes |
| 3 | 15 - Northbound | 560 | 622 | 62 | 11\% | Yes |
| 4 | 15 - Southbound | 514 | 438 | -76 | -15\% | Yes |
| 5 | 60 - Northbound | 873 | 958 | 85 | 10\% | Yes |
| 6 | 60 - Southbound | 887 | 954 | 67 | 8\% | Yes |
| 7 | 64 - Eastbound | 311 | 299 | -12 | -4\% | Yes |
| 8 | 64 - Westbound | 311 | 302 | -9 | -3\% | Yes |
| 9 | 101 - Northbound | 344 | 368 | 24 | 7\% | Yes |
| 10 | 101 - Southbound | 387 | 358 | -29 | -7\% | Yes |
| 11 | 102 - Northbound | 723 | 632 | -91 | -13\% | Yes |
| 12 | 102 - Southbound | 868 | 690 | -178 | -20\% | No |
| 13 | 103 - Eastbound | 359 | 388 | 29 | 8\% | Yes |
| 14 | 103 - Westbound | 401 | 474 | 73 | 18\% | No |
| 15 | 104 - Northbound | 641 | 516 | -125 | -20\% | No |
| 16 | 104 - Southbound | 401 | 395 | -6 | -1\% | Yes |
| 17 | 105 - Eastbound | 464 | 446 | -18 | -4\% | Yes |
| 18 | 105 - Westbound | 423 | 426 | 3 | 1\% | Yes |
| 19 | 106 - Eastbound | 532 | 481 | -51 | -10\% | Yes |
| 20 | 106 - Westbound | 483 | 515 | 32 | 7\% | Yes |

7.10.5. Table 36 shows all but 3 routes pass WebTAG guidance criteria. Of the 3 routes that fail, Route 102 Southbound has a modelled journey time 20\% faster than the observed, Route 103 Westbound has a modelled journey time $18 \%$ slower than the observed and Route 104 northbound is $20 \%$ faster than the observed journey time.

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## PM PEAK JOURNEY TIME PERFORMANCE

7.10.6. Table 37 provides a summary of the performance by each journey time route and direction in the PM peak

Table 37 - PM peak journey time route comparison

| ID | Name | Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 14 - Northbound | 882 | 775 | -107 | -12\% | Yes |
| 2 | 14 - Southbound | 743 | 746 | 3 | 0\% | Yes |
| 3 | 15 - Northbound | 584 | 521 | -63 | -11\% | Yes |
| 4 | 15 - Southbound | 499 | 541 | 42 | 8\% | Yes |
| 5 | 60 - Northbound | 958 | 958 | 0 | 0\% | Yes |
| 6 | 60 - Southbound | 941 | 1005 | 64 | 7\% | Yes |
| 7 | 64 - Eastbound | 323 | 344 | 21 | 7\% | Yes |
| 8 | 64 - Westbound | 312 | 314 | 2 | 1\% | Yes |
| 9 | 101 - Northbound | 341 | 376 | 35 | 10\% | Yes |
| 10 | 101 - Southbound | 467 | 432 | -35 | -8\% | Yes |
| 11 | 102 - Northbound | 683 | 630 | -53 | -8\% | Yes |
| 12 | 102 - Southbound | 760 | 708 | -52 | -7\% | Yes |
| 13 | 103 - Eastbound | 352 | 369 | 17 | 5\% | Yes |
| 14 | 103 - Westbound | 468 | 488 | 20 | 4\% | Yes |
| 15 | 104 - Northbound | 655 | 546 | -109 | -17\% | No |
| 16 | 104 - Southbound | 359 | 399 | 40 | 11\% | Yes |
| 17 | 105 - Eastbound | 462 | 444 | -18 | -4\% | Yes |
| 18 | 105 - Westbound | 412 | 440 | 28 | 7\% | Yes |
| 19 | 106 - Eastbound | 677 | 483 | -194 | -29\% | No |
| 20 | 106 - Westbound | 570 | 548 | -22 | -4\% | Yes |

7.10.7. Table 37 shows all but 2 routes pass WebTAG guidance criteria. Of the 3 routes that fail, Route 104 northbound is modelled 17\% faster than the observed journey time and Route 106 eastbound is modelled 29\% faster than the observed journey time.

### 7.11 JOURNEY TIME PERFORMANCE SUMMARY

7.11.1. The overall performance of journey time routes by peak hour modelled is presented in Table 38.

Table 38-Overall journey time route performance

| ID | Total Observed <br> Journey Time <br> Routes | Modelled <br> Journey Time <br> Routes Within <br> $15 \%$ of Observed | Modelled (s) |
| :---: | :---: | :---: | :---: |
| AM Peak | 20 | 18 | $90 \%$ |
| Inter Peak | 20 | 17 | $85 \%$ |
| PM Peak | 20 | 18 | $90 \%$ |

7.11.2. Table 38 demonstrates that across all three time periods the minimum criteria of $85 \%$ of modelled routes return travel times within $15 \%$ of the observed journey time has been achieved. The modelled is therefore considered to have excellent journey time validation.
7.11.3. Appendix C contains graphs for each journey time route by direction and for each time period modelled.

### 7.12 ROUTE VALIDATION

7.12.1. The routes chosen to validate the route choice were based on the criteria set out in TAG Unit M3.1 (January 2014)
7.12.2. Routes were plotted for all user classes. Guidance presented in section 7.3 of TAG Unit M3.1 (January 2014), with the number of OD pairs determines as follows:

Number of $O D$ pairs $=(\text { number of zones) })^{0.25} x$ number of user classes
7.12.3. Based on the 116 SCTM zones which are within the area of influence for the Lake Lothing Third Crossing, this equates to 36 routes. The origin / destination zone pairs and descriptions used for the route validation are described in Table 39.

Table 39 - Origin-destination route checks

| ID | Origin Zone | Origin Description | Destination Zone | Destination <br> Description |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 408 | Stirrups Lane | 465 | A12 Near the Hollies <br> Camping 7 Leisure <br> Resort |
| 2 | 465 | A12 Near the Hollies <br> Camping 7 Leisure <br> Resort | 408 | Stirrups Lane |

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| 11 | 408 | Stirrups Lane | 582 | 270 Normanston Drive |
| :---: | :---: | :---: | :---: | :---: |
| 12 | 582 | 270 Normanston Drive | 408 | Stirrups Lane |
| 13 | 427 | Harvey Street (Denmark Road) | 781 | Hadenham Road (Screwfix) |
| 14 | 781 | Hadenham Road (Screwfix) | 427 | Harvey Street (Denmark Road) |
| 15 | 639 | 1 Spinney Gardens | 463 | Coopers Lane |
| 16 | 463 | Coopers Lane | 639 | 1 Spinney Gardens |
| 17 | 409 | 5 Lowry Way | 584 | 51-55 Borrow Road |
| 18 | 584 | 51-55 Borrow Road | 409 | 5 Lowry Way |
| 19 | 586 | 2 The Leas | 762 | 10 Gordon Road |
| 20 | 762 | 10 Gordon Road | 586 | 2 The Leas |
| 21 | 432 | 5 Highland Way | 761 | 6 Spurgeon Score |
| 22 | 761 | 6 Spurgeon Score | 432 | 5 Highland Way |
| 23 | 416 | 39 Blyford Road | 588 | 46 Marine Parade |
| 24 | 588 | 46 Marine Parade | 416 | 39 Blyford Road |
| 25 | 593 | 43 Kirkley Cliff Road | 598 | 2 John Lang Court |
| 26 | 598 | 2 John Lang Court | 593 | 43 Kirkley Cliff Road |
| 27 | 773 | Riverside Road | 782 | 48-50 Rectory Road |
| 28 | 782 | 48-50 Rectory Road | 773 | Riverside Road |
| 29 | 409 | 5 Lowry Way | 589 | 138 Waveney Drive |
| 30 | 589 | 138 Waveney Drive | 409 | 5 Lowry Way |
| 31 | 587 | Woodland Path | 779 | 15 Briarwood Road |
| 32 | 779 | 15 Briarwood Road | 587 | Woodland Path |
| 33 | 766 | 15 Leonard Drive | 427 | Maidstone Road (North Sea Charters) |
| 34 | 427 | Maidstone Road (North Sea Charters) | 766 | 15 Leonard Drive |
| 35 | 819 | 20 North Quay Retail Park | 588 | 46 Marine Parade |
| 36 | 588 | 46 Marine Parade | 819 | 20 North Quay Retail |

7.12.4. Appendix D contains "forests" which plot the lowest cost routes in the SATURN for each of the origindestination pairs. These plots have been produced for the car user class with the highest proportion of traffic for the peak hour modelled and for HGVs. Checks of these routes show they are appropriate and provide further evidence the SATURN model provides sensible and predictable routes for vehicles on key routes within the area of influence of the Lake Lothing Third Crossing.

## 8 FORECASTING METHODOLOGY

8.1.1. In order to assess the scheme, it was necessary to build demand trip matrices in relation to the forecast years 2022 (scheme opening year) and 2037 (scheme opening year + 15 years).
8.1.2. The methodology in deriving the aforementioned trip matrices is set out in the following 8 distinct steps:

- Step 1 - Establishing site specific developments within Waveney, whereby the term development refers to either residential or commercial site use. Categorising these developments in relation to 'Uncertainty' definitions as defined from Table A in TAG Unit M4 - Forecasting \& Uncertainty;
- Step 2 - Of these development sites, determine a quantum to be applied in the trip generation process. For example, defining the number of dwellings for residential units;
- Step 3 - Allocate these site specific developments a corresponding SATURN zone;
- Step 4 - Calculation of trip rates to convert the number of dwellings in to peak hour trips in relation to the forecast years via TEMPRO;
- Step 5 - Proportioning out these development trips across the 8 car based user classes.
- Step 6 - Determining growth factors for the surrounding districts via the application of alternative planning assumptions in TEMPRO (dataset version 7.2) to remove the number of household units used in the site specific development trip generation process;
- Step 7 - Applying NRTF (National Road Traffic Forecast) factors to account for UC9 (LGV) and UC10 (HGV) growth in a respective forecast year model; and
- Step 8 - Applying a combination of the above components and matrix furnessing to produce a set of forecast year matrices for the respective model years and peak periods.


### 8.2 STEP 1 - SITE SPECIFIC DEVELOPMENTS (CORE SCENARIO)

8.2.1. Appendix E outlines each of the individual site specific developments that were utilised in the trip generation process and how these site specific developments align with TAG Unit M4 definitions of 'Uncertainty'. For the forecast matrix development, only developments defined as 'Near Certain' or 'More than likely' have been taken in to account and are combined to reflect what is henceforth known as the 'Core' scenario.

### 8.3 STEP 2 - PHASED DEVELOPMENT PROJECTIONS

8.3.1. Of the policies defined in the 'Core' scenario, there was an associated value outlining the sum total of dwellings related to the site. However, not all these dwellings are in operation in the year construction begins and in some cases can cross over between the forecast years being modelled. As such, a phased approach for the sum total of dwellings is applied using housing projections defined via these policies. For example, developments such as the Kirkley Waterfront and Woods Meadow developments adopt this phased approach and can be seen via the columns labelled as "Dwelling Projections" for 2022 and 2037 in Appendix E.

### 8.4 STEP 3 - HIGHWAY ASSIGNMENT MODEL ZONES

8.4.1. All of the developments specified in the 'Core' scenario were allocated a specific zone in relation to the highway assignment model in SATURN. Some developments have been split across multiple zones to account for the size and manner in which they connect with the network; Kirkley Waterfront and Woods Meadow are examples of such developments.

### 8.5 STEP 4 - TRIP RATES

8.5.1. Trip rates were derived using a combination of TEMPRO growth rates in Waveney and local count data. Changes in origin and destination growth rates were used as a proxy to derive the equivalent data covering the AM, IP and PM peak periods. Following this, local ATC data was used to represent peak hour equivalents for the AM and PM peak and average hour for the IP. A summary of these trip rates in relation to the Waveney site specific developments is presented in Table 40.

Table 40 - Forecast Model Trip Rates for Site Specific Developments (Waveney)

| Forecast <br> Year | AM Origin | AM <br> Destination | IP Origin | IP <br> Destination | PM Origin | PM <br> Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{2 0 2 2}$ | 0.182 | 0.255 | 0.242 | 0.240 | 0.258 | 0.202 |
| $\mathbf{2 0 3 7}$ | 0.147 | 0.213 | 0.210 | 0.209 | 0.221 | 0.172 |

### 8.6 STEP 5 - DISTRIBUTION OF TRIPS AMONGST USER CLASSES

8.6.1. As the trips generated are not assigned to a specific user class in line with the base year model, all the development trips are then distributed via user class proportions for the 3 peaks in relation to the 8 car specific user classes. These are based on the user class proportions in the equivalent time periods modelled in the 2016 base year.

### 8.7 STEP 6 - BACKGROUND GROWTH IN SURROUNDING DISTRICTS

8.7.1. Upon determining the quantum of development trips in the Waveney district, it was necessary to derive growth factors in the surrounding districts to account for background growth and thereby constraining the total trip matrix to TEMPRO origin and destination growth factors. An illustration of the different planning assumptions applied to the Waveney district is presented in Appendix F, which outlines the different growth factors with and without alternative planning assumptions applied.

### 8.8 STEP 7 - LGV \& HGV GROWTH FACTORS

8.8.1. NRTF factors were applied to UC9 (LGV) and UC10 (HGV) to account for background growth respective to these user classes.

### 8.9 STEP 8 - MATRIX FURNESSING

8.9.1. The above calculations and factors were then applied to generate forecast matrices for 2022 and 2037 by using the matrix furnessing capability within SATURN. The 2016 base year matrix was used as the pivot for the furnessing process, the final forecast model trip matrix totals are presented via Table 41 in PCU's (Passenger Car Units).

Table 41 - Forecast Trip Matrix Totals (PCU Units)

| User Class | 2016 | 2022 | 2037 |
| :---: | :---: | :---: | :---: |
| AM |  |  |  |
| UC1: Car - Home Based Work (Inbound) | 3,719 | 3,960 | 4,385 |
| UC2: Car - Home Based Work (Outbound) | 56,288 | 59,778 | 65,970 |
| UC3: Car - Home Based Employers Business (Inbound) | 316 | 335 | 369 |
| UC4: Car - Home Based Employers Business (Outbound) | 4,207 | 4,469 | 4,931 |
| UC5: Car - Non Home Based Employers Business | 8,186 | 8,708 | 9,628 |
| UC6: Car - Home Based Other (Inbound) | 5,510 | 5,858 | 6,472 |
| UC7: Car - Home Based Other (Outbound) | 34,885 | 37,043 | 40,864 |
| UC8: Car - Non Home Based Other | 6,037 | 6,416 | 7,084 |
| UC9: LGV | 12,629 | 14,866 | 20,084 |
| UC10: HGV | 8,810 | 9,466 | 11,171 |
| Total | 140,587 | 150,897 | 170,959 |
| \% Change relative to 2016 | - | 7\% | 22\% |
| IP |  |  |  |
| UC1: Car - Home Based Work (Inbound) | 7,899 | 8,553 | 9,847 |
| UC2: Car - Home Based Work (Outbound) | 6,562 | 7,103 | 8,174 |
| UC3: Car - Home Based Employers Business (Inbound) | 1,036 | 1,121 | 1,290 |
| UC4: Car - Home Based Employers Business (Outbound) | 892 | 966 | 1,111 |
| UC5: Car - Non Home Based Employers Business | 6,846 | 7,416 | 8,541 |
| UC6: Car - Home Based Other (Inbound) | 28,444 | 30,796 | 35,441 |
| UC7: Car - Home Based Other (Outbound) | 28,389 | 30,734 | 35,368 |
| UC8: Car - Non Home Based Other | 15,004 | 16,249 | 18,713 |
| UC9: LGV | 10,611 | 12,491 | 16,875 |
| UC10: HGV | 9,070 | 9,745 | 11,500 |
| Total | 114,752 | 125,173 | 146,859 |
| \% Change relative to 2016 | - | 9\% | 28\% |
| PM |  |  |  |
| UC1: Car - Home Based Work (Inbound) | 53,969 | 57,262 | 63,401 |
| UC2: Car - Home Based Work (Outbound) | 1,485 | 1,579 | 1,752 |
| UC3: Car - Home Based Employers Business (Inbound) | 4,031 | 4,280 | 4,741 |
| UC4: Car - Home Based Employers Business (Outbound) | 967 | 1,027 | 1,137 |
| UC5: Car - Non Home Based Employers Business | 7,662 | 8,141 | 9,030 |
| UC6: Car - Home Based Other (Inbound) | 37,366 | 39,635 | 43,859 |
| UC7: Car - Home Based Other (Outbound) | 16,567 | 17,591 | 19,496 |
| UC8: Car - Non Home Based Other | 11,579 | 12,294 | 13,624 |
| UC9: LGV | 10,292 | 12,116 | 16,369 |
| UC10: HGV | 6,085 | 6,538 | 7,715 |
| Total | 150,004 | 160,463 | 181,124 |
| \% Change relative to 2016 | - | 7\% | 21\% |

## 9 SUMMARY

9.1.1. This Local Model Validation Report (LMVR) details the fitness for purpose of the SCTM in Lowestoft. This highway model will be used in conjunction with a public transport and demand model which have been developed in VISUM.
9.1.2. The SCTM highway model represents a base year of 2016, and incorporated the networks previously developed in the Ipswich Transport Model (ITM) and Lowestoft Transport Model (LTM) which were used in 2015 for the Outline Business Cases for the Upper Orwell Crossing in Ipswich and Third Crossing in Lowestoft.
9.1.3. The SCTM highway model can be used as a stand-alone highway model from which to build forecast highway assessments, but also in conjunction with the public transport model and demand model to test the impacts of multi-modal changes and interventions. The SCTM will be used during the DCO process for the Upper Orwell Crossing in Ipswich and Lowestoft Third Crossing.
9.1.4. The validation and calibration has referenced the latest guidelines stipulated in WebTAG as the basis for determining the fitness for purpose of the SCTM highway model.
9.1.5. An extensive data collection exercise was carried out in Lowestoft in 2015 to inform the OBC. This has been supplemented by additional count data collected in 2016 and 2017. The range of traffic data available within Lowestoft is considered appropriate and sufficient to create a strategic highway assignment. Further data collection will have to be considered going forward for any local testing of schemes and developments which need to be carried out.
9.1.6. The trip matrices used for both the highway model and public transport model have been derived predominantly from Mobile Network Data (MND) supplied by Telefonica, supplemented with a synthetic matrix based upon a gravity model. Extensive verification of the MND has been carried out.
9.1.7. Screenlines have been presented in this report, covering key movements in Lowestoft. The final matrix assignment is shown to match well across the array of screenlines, achieving a flow difference within $5 \%$ in the majority of cases.
9.1.8. The model is shown to converge satisfactorily across all three peaks. In terms of combined calibration and validation counts, the model is shown to achieve $85 \%$ of counts with a GEH of below 5 across all three peaks.
9.1.9. A comprehensive coverage of journey time routes have been included in the SCTM highway model, taking into account directionality of routes there are 125 routes which have been used to analyse journey time route performance across the key strategic routes within the county. In terms of the WebTAG requirement of observed modelled journey times being within $15 \%$ of the modelled journey times, this has been achieved for $90 \%$ of routes in the AM peak, $85 \%$ in the inter peak and $90 \%$ in the PM peak. This is significantly above the $85 \%$ threshold stipulated in WebTAG. Routes which do not meet the requirement within WebTAG are detailed in Section 11 of this report and highlight issues where the SCTM highway model has not been able to emulate significant increases in the observed delay present in the AM and PM peak.
9.1.10. The SCTM highway model has been validated to link based traffic data, however when compared to the commissioned Manual Classified Turning Count data, the model is shown to match well at these key junctions in terms of GEH when comparing modelled and observed turn flows in the majority of cases.
9.1.11. The forecast matrices have been built in accordance with TAG Unit M4 definitions of modelling assumptions to be considered in a 'Core' scenario for 2022 and 2037. These trip matrices have been built with the latest TEMPRO data set available as of November 2017 with site specific development taken in to account within the district of Waveney.
9.1.12. It is considered the SCTM highway model has shown to provide a sufficient match to observed traffic count and journey time data within Lowestoft. The SCTM highway model provides a robust basis from which to create forecast assignments for future scheme and development testing.

## Appendix A

SCREENLINE PERFORMANCE

| Screenline |  |  |
| :---: | :---: | :---: |
| ID | Name | Type |
| 1 | Screenline 18 South - <br> Inbound | Calibration |
| 2 | Screenline 18 South - <br> Outbound | Calibration |
| 3 | Lowestoft Screenline <br> - NB | Calibration |
| 4 | Lowestoft Screenline <br> $1-$ SB | Calibration |
| 5 | Lowestoft Screenline <br> $2-$ NB | Calibration |
| 6 | Lowestoft Screenline <br> $2-$ SB | Calibration |
| 7 | Screenline 18 North - <br> Inbound | Calibration |
| 8 | Screenline 18 North - <br> Outbound | Calibration |
| 9 | Lowestoft Screenline <br> $4-$ EB | Validation |
| 10 | Lowestoft Screenline <br> $4-$ WB | Validation |
| 11 | Lowestoft Screenline <br> $5-$ NB | Validation |
| 12 | Lowestoft Screenline <br> $5-$ SB | Validation |
| 13 | Lowestoft Screenline <br> $6-$ NB | Validation |
| 14 | Lowestoft Screenline <br> $6-$ SB | Validation |


| AM Peak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| All |  |  |  | Car |  |  |  | LGV |  |  |  | HGV |  |  |  |
| Observed | Modelled | Difference | GEH | Observed | Modelled | Difference | GEH | Observed | Modelled | Difference | GEH | Observed | Modelled | Difference | GEH |
| 1421 | 1452 | 2\% | 0.824 | 1191 | 1191 | 0\% | 0.007 | 194 | 195 | 0\% | 0.065 | 36 | 66 | 83\% | 4.203 |
| 1330 | 1364 | 3\% | 0.941 | 1115 | 1117 | 0\% | 0.080 | 182 | 182 | 0\% | 0.058 | 34 | 65 | 92\% | 4.421 |
| 2207 | 2207 | 0\% | 0.015 | 1846 | 1845 | 0\% | 0.009 | 301 | 301 | 0\% | 0.027 | 60 | 60 | 0\% | 0.019 |
| 1685 | 1693 | 0\% | 0.190 | 1408 | 1413 | 0\% | 0.134 | 230 | 230 | 0\% | 0.010 | 47 | 50 | 6\% | 0.422 |
| 2866 | 2872 | 0\% | 0.114 | 2402 | 2408 | 0\% | 0.128 | 391 | 391 | 0\% | 0.019 | 73 | 73 | 0\% | 0.023 |
| 2095 | 2114 | 1\% | 0.413 | 1755 | 1768 | 1\% | 0.291 | 286 | 286 | 0\% | 0.012 | 53 | 60 | 13\% | 0.921 |
| 1194 | 1191 | 0\% | 0.094 | 978 | 979 | 0\% | 0.035 | 179 | 175 | -2\% | 0.305 | 37 | 37 | -1\% | 0.045 |
| 1454 | 1452 | 0\% | 0.050 | 1251 | 1253 | 0\% | 0.075 | 165 | 160 | -3\% | 0.404 | 38 | 39 | 2\% | 0.099 |
| 1747 | 1626 | -7\% | 2.947 | 1376 | 1276 | -7\% | 2.745 | 297 | 277 | -7\% | 1.185 | 74 | 73 | -1\% | 0.118 |
| 1741 | 1541 | -11\% | 4.927 | 1394 | 1201 | -14\% | 5.355 | 266 | 273 | 3\% | 0.430 | 80 | 67 | -17\% | 1.605 |
| 2419 | 2308 | -5\% | 2.288 | 2038 | 1963 | -4\% | 1.679 | 280 | 283 | 1\% | 0.152 | 101 | 62 | -38\% | 4.284 |
| 1732 | 1883 | 9\% | 3.551 | 1400 | 1562 | 12\% | 4.201 | 247 | 270 | 9\% | 1.426 | 85 | 51 | -40\% | 4.070 |
| 1359 | 1344 | -1\% | 0.408 | 1155 | 1164 | 1\% | 0.265 | 164 | 146 | -11\% | 1.460 | 40 | 35 | -15\% | 0.962 |
| 1024 | 1052 | 3\% | 0.846 | 844 | 861 | 2\% | 0.588 | 151 | 155 | 3\% | 0.388 | 30 | 35 | 18\% | 0.923 |



| AM Peak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| All |  |  |  | Car |  |  |  | LGV |  |  |  | HGV |  |  |  |
| Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEL |
| 19 | 48 | 156\% | 5.075 | 16 | 42 | 164\% | 4.820 | 3 | 7 | 158\% | ${ }^{1.886}$ | 0 | 0 | -100\% | 0.978 |
| 34 | 0 | -100\% | 8.271 | 29 | 0 | -100\% | 7.572 | 5 | 0 | -100\% | ${ }^{3.056}$ | 1 | 0 | -100\% | 1.320 |
| 20 | 19 | -8\% | 0.352 | 17 | 17 | 0\% | 0.003 | 3 | 1 | -49\% | 0.952 | 1 | 0 | -32\% | 0.255 |
| 634 | 751 | 18\% | 4.449 | 531 | 610 | 15\% | 3.293 | 87 | 105 | 22\% | 1.921 | 16 | 36 | 121\% | 3.849 |
| 713 | 634 | -11\% | 3.061 | 598 | 522 | -13\% | 3.191 | 97 | 81 | -16\% | 1.682 | 18 | 30 | 66\% | 2.439 |
| 1421 | 1452 | 2\% | 0.824 | 1191 | 1191 | 0\% | 0.007 | 194 | 195 | 0\% | 0.065 | 36 | 66 | 83\% | 4.203 |
| 14 | ${ }^{22}$ | 56\% | 1.832 | 12 | 19 | 65\% | 1.923 | 2 | 2 | 20\% | 0.267 | 0 | 0 | -60\% | 0.430 |
| 44 | 0 | -100\% | 9.366 | 37 | 0 | -100\% | 8.574 | 6 | 0 | -100\% | 3.460 | 1 | 0 | -100\% | 1.495 |
| 17 | 22 | 26\% | 1.032 | 15 | 20 | 40\% | 1.395 | 2 | 1 | -62\% | 1.147 | 0 | 1 | 48\% | 0.285 |
| 665 | 812 | 22\% | 5.418 | 557 | 668 | 20\% | 4.467 | 91 | 107 | 18\% | 1.645 | 17 | 37 | 120\% | 3.905 |
| 590 | 509 | -14\% | 3.461 | 494 | 410 | -17\% | 3.974 | 81 | 72 | -11\% | 0.975 | 15 | 27 | 79\% | 2.593 |
| 1330 | 1364 | 3\% | 0.941 | 1115 | 1117 | 0\% | 0.080 | 182 | 182 | 0\% | 0.058 | 34 | 65 | 92\% | 4.421 |
| 449 | 420 | -6\% | 1.357 | 376 | 369 | -2\% | 0.359 | ${ }^{61}$ | 48 | -22\% | ${ }^{1.862}$ | 11 | 4 | -67\% | 2.762 |
| 817 | 909 | 11\% | 3.143 | 685 | 761 | 11\% | 2.860 | 111 | 114 | 2\% | 0.244 | ${ }^{21}$ | 34 | 62\% | 2.461 |
| 307 | 454 | 48\% | 7.528 | 258 | 380 | 47\% | 6.851 | 42 | 68 | 61\% | 3.458 | 8 | 7 | -13\% | 0.364 |
| 475 | 370 | -22\% | 5.127 | 398 | 294 | -26\% | 5.609 | 65 | ${ }_{63}$ | -4\% | 0.294 | 12 | 13 | 11\% | 0.362 |
| 159 | 53 | -67\% | 10.337 | 129 | 41 | -68\% | 9.577 | 22 | 9 | -58\% | 3.220 | 8 | 3 | -67\% | 2.316 |
| 2207 | 2207 | 0\% | 0.015 | 1846 | 1845 | 0\% | 0.009 | 301 | 301 | 0\% | 0.027 | 60 | 60 | 0\% | 0.019 |
| 354 | 459 | 30\% | 5.214 | 296 | 381 | 29\% | 4.609 | 48 | 75 | 55\% | ${ }^{3.363}$ | 9 | 3 | -68\% | ${ }^{2.508}$ |
| 378 | 400 | 6\% | 1.122 | 317 | 330 | 4\% | 0.761 | 52 | 58 | 12\% | 0.865 | 10 | 12 | 21\% | ${ }^{0.623}$ |
| 356 | 438 | 23\% | 4.134 | 298 | 347 | 16\% | 2.695 | 49 | 68 | 39\% | 2.490 | 9 | 24 | 165\% | ${ }^{3.686}$ |
| 432 | 309 | -29\% | 6.399 | 362 | 274 | -24\% | 4.946 | 59 | ${ }^{24}$ | -59\% | 5.419 | 11 | 11 | 0\% | 0.015 |
| 165 | 87 | -48\% | 7.005 | 135 | 81 | -40\% | 5.178 | 22 | 5 | -76\% | 4.552 | 8 | 0 | -94\% | 3.756 |
| 1685 | 1693 | 0\% | 0.190 | 1408 | 1413 | 0\% | 0.134 | 230 | 230 | 0\% | 0.010 | 47 | 50 | 6\% | 0.422 |
| 408 | 302 | -26\% | 5.600 | 342 | 256 | -25\% | 4.980 | 56 | 34 | -39\% | 3.249 | 10 | ${ }^{13}$ | 22\% | 0.676 |
| 670 | 587 | -12\% | ${ }^{3.298}$ | 561 | 511 | -9\% | 2.193 | 91 | 62 | -32\% | ${ }^{3.322}$ | 17 | 14 | -16\% | 0.702 |
| 135 | 117 | -13\% | 1.625 | 113 | 90 | -20\% | 2.300 | 18 | 24 | 30\% | 1.205 | 3 | 3 | -17\% | 0.338 |
| 393 | 326 | -17\% | 3.530 | 329 | 258 | -22\% | 4.135 | 54 | 62 | 15\% | 1.076 | 10 | 6 | -42\% | ${ }^{1.503}$ |
| 681 | 945 | 39\% | 9.254 | 571 | ${ }^{783}$ | 37\% | 8.161 | ${ }^{93}$ | 135 | 45\% | ${ }^{3.896}$ | 17 | ${ }^{27}$ | 58\% | 2.116 |
| 579 | 595 | 3\% | 0.640 | 485 | 510 | 5\% | 1.111 | 79 | 74 | -6\% | 0.545 | 15 | 10 | $-31 \%$ | 1.273 |
| 2866 | 2872 | 0\% | 0.114 | 2402 | 2408 | 0\% | 0.128 | 391 | 391 | 0\% | 0.019 | 73 | 73 | 0\% | ${ }^{0.023}$ |
| 133 | 266 | 100\% | 9.445 | 111 | 223 | 101\% | 8.657 | 18 | 35 | 93\% | 3.285 | 3 | 8 | 133\% | 1.892 |



| AM Peak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| All |  |  |  | Car |  |  |  | LGV |  |  |  | HGV |  |  |  |
| Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH |
| 566 | 338 | -40\% | 10.767 | 475 | 284 | -40\% | 9.767 | 77 | 48 | -37\% | ${ }^{3.636}$ | 14 | 5 | -68\% | 3.165 |
| 98 | 86 | -13\% | 1.318 | 82 | 66 | -19\% | 1.851 | 13 | 14 | 7\% | 0.241 | 2 | 5 | 97\% | 1.259 |
| 315 | 393 | 25\% | 4.152 | 264 | 341 | 29\% | 4.440 | 43 | 41 | -5\% | 0.303 | 8 | 11 | 36\% | 0.928 |
| 503 | 542 | 8\% | 1.723 | 421 | 438 | 4\% | 0.820 | 69 | 83 | 21\% | 1.633 | 13 | 21 | 64\% | 1.988 |
| 480 | 490 | 2\% | 0.433 | 402 | 415 | 3\% | 0.605 | 66 | 64 | -2\% | 0.182 | 12 | 11 | -10\% | 0.358 |
| 2095 | 2114 | 1\% | 0.413 | 1755 | 1768 | 1\% | 0.291 | 286 | 286 | 0\% | 0.012 | 53 | 60 | 13\% | 0.921 |
| 178 | 230 | 29\% | ${ }^{3.651}$ | 149 | 192 | 29\% | 3.288 | ${ }^{24}$ | 34 | 42\% | 1.872 | 5 | 4 | -21\% | 0.470 |
| 83 | ${ }^{23}$ | -72\% | 8.259 | 70 | 21 | -69\% | 7.176 | 11 | 2 | -85\% | 3.779 | 2 | 0 | -100\% | 2.061 |
| 50 | 0 | -100\% | 9.939 | 42 | 0 | -100\% | 9.098 | 7 | 0 | -100\% | 3.672 | 1 | 0 | -100\% | 1.590 |
| 883 | 938 | 6\% | 1.810 | 717 | 765 | 7\% | 1.776 | 137 | 139 | 2\% | 0.189 | 29 | 33 | 14\% | 0.724 |
| 1194 | 1191 | 0\% | 0.094 | 978 | 979 | 0\% | 0.035 | 179 | 175 | -2\% | 0.305 | 37 | 37 | -1\% | 0.045 |
| 112 | 142 | 27\% | 2.635 | 94 | 117 | 25\% | 2.242 | 15 | 19 | 24\% | ${ }^{0.873}$ | 3 | 6 | 107\% | 1.460 |
| 51 | 23 | -55\% | 4.617 | ${ }^{43}$ | 16 | -62\% | 4.839 | 7 | 7 | -7\% | 0.175 | 1 | 0 | -100\% | 1.611 |
| 61 | 51 | -16\% | 1.313 | 51 | 51 | -1\% | 0.070 | 8 | 0 | -94\% | 3.725 | 2 | 0 | -100\% | 1.754 |
| 1230 | 1236 | 1\% | 0.180 | 1063 | 1069 | 1\% | 0.197 | 135 | 134 | 0\% | 0.046 | 32 | 33 | 1\% | 0.069 |
| 1454 | 1452 | 0\% | 0.050 | 1251 | 1253 | 0\% | 0.075 | 165 | 160 | -3\% | 0.404 | 38 | 39 | 2\% | 0.099 |
| 437 | 439 | 0\% | 0.060 | 367 | 367 | 0\% | ${ }^{0.013}$ | 60 | 50 | -17\% | 1.359 | 11 | ${ }^{22}$ | 99\% | 2.711 |
| 233 | 151 | -35\% | 5.948 | 190 | 140 | -26\% | ${ }^{3.873}$ | 32 | 11 | -66\% | 4.520 | 12 | 0 | -100\% | 4.874 |
| 346 | 299 | -14\% | 2.636 | 282 | 263 | -7\% | 1.119 | 47 | 33 | -30\% | 2.254 | 18 | 3 | -83\% | 4.568 |
| 729 | 737 | 1\% | 0.279 | 537 | 505 | -6\% | 1.399 | 158 | 183 | 16\% | 1.916 | 33 | 48 | 43\% | 2.268 |
| 1747 | 1626 | -7\% | 2.947 | 1376 | 1276 | -7\% | 2.745 | 297 | 277 | -7\% | 1.185 | 74 | 73 | -1\% | 0.118 |
| 341 | 399 | 17\% | 3.029 | 285 | 297 | 4\% | 0.670 | 46 | 77 | 66\% | 3.908 | 9 | 25 | 185\% | 3.931 |
| 231 | 55 | -76\% | 14.762 | 188 | 48 | -74\% | 12.865 | 31 | 6 | -80\% | 5.766 | 12 | 0 | -99\% | 4.768 |
| 381 | 352 | -8\% | 1.533 | 310 | 280 | -10\% | 1.740 | 52 | 68 | 32\% | 2.156 | 19 | 3 | -83\% | 4.805 |
| 788 | 736 | -7\% | 1.879 | 610 | 576 | -6\% | 1.416 | 137 | 121 | -11\% | 1.349 | 41 | 38 | -5\% | 0.323 |
| 1741 | 1541 | -11\% | 4.927 | 1394 | 1201 | -14\% | 5.355 | 266 | 273 | 3\% | 0.430 | 80 | 67 | -17\% | 1.605 |
| 1007 | 1069 | 6\% | 1.934 | 833 | 895 | 7\% | 2.098 | 118 | 149 | 26\% | 2.663 | 56 | ${ }^{26}$ | -54\% | 4.707 |
| -1412 | . 1238 | -12\% | - 4.767 | -1205 | 1068 | -11\% | 4.056 | -162 | - 134 | -17\% | - 2320 | 45 | 36 | - $19 \%$ | 1.347 |
| 921 | 1038 | 13\% | ${ }^{2.728}$ | 742 | 869 | 17\% | 4.476 | 130 | $\stackrel{1}{135}$ | 4\% | 0.440 | 49 | ${ }^{62}$ | -31\% | ${ }^{4.284}$ |
| 811 | 845 | 4\% | -1.192 | 658 | 693 | 5\% | -1.332 | 117 | 135 | 15\% | 1.592 | 36 | 18 | -51\% | 3.505 |
| -1732 | -1883 | 9\% | -3.551 | -1400 | 1562 | 12\% | 4.201 | 247 | 270 | 9\% | ${ }_{1} 1.426$ | 85 | 51 | -40\% | 4.070 |
| 95 | 98 | 2\% | 0.229 | 80 | 87 | 9\% | 0.808 | ${ }^{13}$ | 10 | -23\% | 0.871 | 2 | 0 | -90\% | 1.886 |
| 458 | 454 | -1\% | 0.183 | 384 | 396 | 3\% | 0.617 | 62 | 50 | -19\% | 1.596 | 12 | 8 | -35\% | 1.319 |
| - 806 | 793 | -2\% | 0.473 | 691 | 681 | -2\% | -0.402 | 88 | 85 | -4\% | 0.345 | 26 | 27 | 1\% | 0.075 |
| 1359 | 1344 | -1\% | 0.408 | 1155 | 1164 | 1\% | 0.265 | 164 | 146 | -11\% | 1.460 | 40 | 35 | -15\% | 0.962 |
| 101 | 72 | -29\% | 3.152 | 85 | 59 | -31\% | 3.075 | 14 | 11 | -18\% | 0.708 | 3 | 2 | -29\% | 0.511 |
| 249 | 276 | 11\% | 1.657 | 209 | 228 | 9\% | 1.319 | 34 | 41 | 19\% | 1.074 | 6 | 7 | 13\% | 0.306 |
| 674 | 704 | 4\% | 1.130 | 550 | 574 | 4\% | 0.998 | 103 | 104 | 1\% | 0.073 | 21 | 27 | 25\% | 1.074 |



Screenline

|  | Screenine |  |
| :---: | :---: | :---: |
|  |  |  |
| 1 | Screenline 18 South Inbound |  |
| 2 | Screenline 18 South Outbound |  |
| 3 | Lowestoft Screenline $1-\mathrm{NB}$ | c |
| 4 | Lowestoft Screenline $1-S B$ | C |
| 5 | Lowestoft Screenline $2-N B$ | C |
| 6 | Lowestoft Screenline $2-S B$ | C |
| 7 | Screenline 18 North Inbound |  |
| 8 | Screenline 18 North Outbound |  |
| 9 | Lowestoft Screenline $4-\text { EB }$ |  |
| 10 | Lowestoft Screenline $4-W B$ |  |
| 11 | Lowestoft Screenline $5-\mathrm{NB}$ |  |
| 12 | Lowestoft Screenline $5-\text { SB }$ |  |
| 13 | Lowestoft Screenline $6-\mathrm{NB}$ |  |
| 14 | Lowestoft Screenline $6-S B$ |  |


| Interpeak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| All |  |  |  | Car |  |  |  | LGV |  |  |  | HGV |  |  |  |
| Observed | Modelled | Difference | GEH | Observed | Modelled | Difference | GEH | Observed | Modelled | Difference | GEH | Observed | Modelled | Difference | GEH |
| 1326 | 1339 | 1\% | 0.369 | 1116 | 1115 | 0\% | 0.024 | 174 | 175 | 0\% | 0.048 | 36 | 49 | 38\% | 2.096 |
| 1347 | 1361 | 1\% | 0.369 | 1134 | 1135 | 0\% | 0.032 | 177 | 176 | -1\% | 0.100 | 36 | 50 | 38\% | 2.110 |
| 1826 | 1828 | 0\% | 0.046 | 1528 | 1530 | 0\% | 0.049 | 243 | 243 | 0\% | 0.005 | 54 | 55 | 0\% | 0.020 |
| 1873 | 1876 | 0\% | 0.058 | 1567 | 1570 | 0\% | 0.063 | 250 | 250 | 0\% | 0.004 | 56 | 56 | 0\% | 0.012 |
| 2473 | 2481 | 0\% | 0.164 | 2082 | 2089 | 0\% | 0.173 | 325 | 325 | 0\% | 0.009 | 66 | 67 | 1\% | 0.054 |
| 2635 | 2628 | 0\% | 0.135 | 2218 | 2213 | 0\% | 0.110 | 346 | 344 | -1\% | 0.112 | 71 | 71 | 1\% | 0.042 |
| 1052 | 1030 | -2\% | 0.689 | 871 | 851 | -2\% | 0.704 | 148 | 147 | -1\% | 0.149 | 32 | 32 | 1\% | 0.042 |
| 1027 | 1009 | -2\% | 0.557 | 855 | 838 | -2\% | 0.560 | 143 | 141 | -1\% | 0.123 | 30 | 30 | 0\% | 0.004 |
| 1665 | 1587 | -5\% | 1.939 | 1336 | 1281 | -4\% | 1.498 | 252 | 249 | -1\% | 0.177 | 77 | 56 | -27\% | 2.597 |
| 1662 | 1595 | -4\% | 1.662 | 1326 | 1282 | -3\% | 1.220 | 252 | 247 | -2\% | 0.316 | 84 | 66 | -21\% | 2.078 |
| 1857 | 2036 | 10\% | 4.047 | 1592 | 1724 | 8\% | 3.231 | 197 | 259 | 31\% | 4.085 | 68 | 53 | -22\% | 1.898 |
| 2013 | 2188 | 9\% | 3.810 | 1728 | 1831 | 6\% | 2.447 | 208 | 294 | 42\% | 5.461 | 77 | 62 | -20\% | 1.809 |
| 940 | 936 | -1\% | 0.157 | 789 | 781 | -1\% | 0.306 | 127 | 129 | 2\% | 0.193 | 25 | 26 | 6\% | 0.312 |
| 905 | 936 | 3\% | 1.020 | 754 | 776 | 3\% | 0.795 | 123 | 132 | 8\% | 0.846 | 28 | 28 | -2\% | 0.112 |


| ID | Name | Link ID | Status | $\begin{gathered} \text { Site } \\ \text { Location } \end{gathered}$ | Direction | A-Node | B-Node | ID Rep | Interpeak | Off Peak |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Screenline 18South -Inbound | 548 | Calloration | MutfordwRusmmereRoadGiselamAl46AecclesBeadR12A12LondonRoad | EB | 4567 | 5070 |  |  |  |
|  |  | 550 | Calibraion |  | Eb | 4566 | 5000 |  |  |  |
|  |  | 552 | Calloration |  | NB | 9600 | 5010 |  |  |  |
|  |  | 554 | Calibration |  | EB | 4512 | 2000 |  |  |  |
|  |  | 558 | Calibration |  | NB | 5390 | 1000 |  |  |  |
|  |  | TOTAL |  |  |  | Calibration |  |  | Yes | Yes |
| 2 | $\left.\begin{gathered} \text { Screenline } 18 \\ \text { South - } \\ \text { Outbound } \end{gathered} \right\rvert\,$ | 549 | Calibration | Mutfordw <br> Rushmere <br> Road <br> Gisleham <br> A146 <br> Beccles <br> Road <br> A12 <br> London <br> Road | wB | 5070 | 4567 |  |  |  |
|  |  | 551 | Calibraion |  | wв | 5000 | 4566 |  |  |  |
|  |  | 553 | Calloration |  | SB | 5010 | 9600 |  |  |  |
|  |  | 555 | Calibration |  | wв | 2000 | 4512 |  |  |  |
|  |  | 559 | Calibration |  | SB | 1000 | 5390 |  |  |  |
|  |  | total |  |  |  | Calliration |  |  | Yes | Yes |
| 3 | LowestoftScreenline 1NB | 560 | Callioration | London <br> A12 Tom <br> Crisp Way <br> A1117 <br> A146 <br> Aeccles <br> Boad <br> Road <br> Long <br> Road | NEB | 9606 | 1040 |  |  |  |
|  |  | 562 | Calibration |  | NEB | 3000 | 6314 |  |  |  |
|  |  | 564 | Callibration |  | NB | 3030 | 3040 |  |  |  |
|  |  | 566 | Calibration |  | neb | 6406 | 6404 |  |  |  |
|  |  | 1618 | Calliralion |  | NB | 3010 | 5180 |  |  |  |
|  |  | TOTAL |  |  |  | Calibration |  |  | Yes | Yes |
| 4 | LowestoftScreenline 1SB | 561 | Calibration |  | swB | 1040 | 9606 |  |  |  |
|  |  | 563 | Calibration |  | SWB | 6314 | 3000 |  |  |  |
|  |  | 565 | Callibration |  | SB | 3040 | 3030 |  |  |  |
|  |  | 567 | Calibration |  | swb | 6404 | 6406 |  |  |  |
|  |  | 1617 | Calibration |  | SB | 5180 | 3010 |  |  |  |
|  |  | total |  |  |  | Calibration |  |  | Yes | Yes |
| 5 | LowestoftScreenline 2NB | 574 | Calibration | Katwivik <br> A12 <br> Battery <br> Green <br> Rroad <br> Roterdam <br> Peto Way <br> A1117 <br> Normanst <br> on Drive <br> B1375 <br> Gorleston | NB | 6040 | 6431 |  |  |  |
|  |  | 576 | Calibration |  | NB | 6160 | 6150 |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  | 584 | Calloration |  | NEB | 7210 | 9130 |  |  |  |
|  |  | 586 | Calibration |  | NB | 10190 | 7060 |  |  |  |
|  |  | 588 | Calibration |  | NEB | 7050 | 7060 |  |  |  |
|  |  | 594 | Calibration |  | NB | 8030 | 8040 |  |  |  |
|  |  | total |  |  |  | Callbration |  |  | Yes | Yes |
|  |  | 575 | Callibration | Katwij | SB | 6431 | 6040 |  |  |  |


| Interpeak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| All |  |  |  | Car |  |  |  | LGV |  |  |  | HGV |  |  |  |
| Observed | Modelled | Difierence | GEH | Obsenved | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH |
| 20 | ${ }^{42}$ | 105\% | 3.830 | 17 | ${ }^{38}$ | 121\% | ${ }^{3.947}$ | 3 | 4 | 41\% | ${ }^{0.612}$ | 1 | 0 | -85\% | 0.823 |
| 50 | 0 | -100\% | 10.037 | 42 | 0 | -100\% | ${ }^{9.209}$ | 7 | 0 | -100\% | ${ }^{3.638}$ | 1 | 0 | -100\% | 1.643 |
| 20 | 59 | 200\% | 6.298 | 17 | 58 | 248\% | 6.756 | 3 | 1 | -61\% | 1.174 | 1 | 0 | -7\% | 0.054 |
| 591 | 695 | 18\% | 4.123 | 497 | 573 | 15\% | 3.291 | 78 | 96 | 24\% | 1.963 | 16 | 26 | 64\% | 2.215 |
| 645 | 543 | -16\% | 4.171 | 543 | 446 | -18\% | 4.337 | 85 | 74 | -12\% | 1.184 | 17 | ${ }^{23}$ | 31\% | 1.201 |
| 1326 | 1339 | 1\% | 0.369 | 1116 | 1115 | 0\% | 0.024 | 174 | 175 | 0\% | 0.048 | 36 | 49 | 38\% | 2.096 |
| 18 | 49 | 180\% | 5.459 | 15 | 45 | 208\% | 5.597 | 2 | 3 | 49\% | 0.664 | 0 | 0 | -73\% | 0.630 |
| 52 | 0 | -100\% | 10.183 | 44 | 0 | -100\% | ${ }^{9.343}$ | 7 | 0 | -100\% | ${ }^{3.691}$ | 1 | 0 | -100\% | 1.667 |
| ${ }^{23}$ | 33 | 44\% | 1.909 | 19 | 32 | 67\% | 2.542 | 3 | 0 | -85\% | 1.933 | 1 | 0 | -44\% | 0.386 |
| 590 | 706 | 20\% | 4.573 | 496 | 585 | 18\% | 3.825 | 77 | 97 | 25\% | 2.056 | 16 | 24 | 52\% | 1.850 |
| 666 | 573 | -14\% | 3.715 | 561 | 473 | -16\% | 3.861 | 87 | 75 | -14\% | 1.364 | 18 | 25 | 42\% | 1.630 |
| 1347 | 1361 | 1\% | 0.369 | 1134 | 1135 | 0\% | 0.032 | 177 | 176 | -1\% | 0.100 | 36 | 50 | 38\% | 2.110 |
| 386 | ${ }^{341}$ | -12\% | 2.361 | 325 | 291 | -10\% | 1.945 | 51 | 45 | -11\% | 0.793 | 10 | 5 | -52\% | 1.951 |
| 491 | 552 | 12\% | 2.651 | 413 | 446 | 8\% | ${ }^{1.583}$ | 65 | 83 | 29\% | 2.173 | 13 | 22 | 69\% | 2.152 |
| 384 | 474 | 24\% | 4.377 | 323 | 389 | 20\% | 3.505 | 50 | 70 | 38\% | 2.472 | 10 | 16 | 52\% | 1.491 |
| 418 | 409 | -2\% | 0.434 | 352 | 355 | 1\% | 0.184 | 55 | 42 | -23\% | 1.832 | 11 | 12 | 4\% | 0.139 |
| 147 | 52 | -65\% | 9.572 | 115 | 49 | -58\% | 7.341 | 23 | 3 | -87\% | 5.509 | 9 | 0 | -99\% | 4.268 |
| 1826 | 1828 | 0\% | 0.046 | 1528 | 1530 | 0\% | 0.049 | 243 | 243 | 0\% | 0.005 | 54 | 55 | 0\% | 0.020 |
| 409 | 449 | 10\% | 1.933 | 345 | 385 | 12\% | 2.115 | 54 | 57 | 7\% | ${ }^{0.472}$ | ${ }^{11}$ | 7 | -35\% | 1.281 |
| 493 | 539 | 9\% | 2.001 | 415 | 433 | 4\% | 0.859 | 65 | 77 | 19\% | ${ }^{1.480}$ | 13 | 29 | 116\% | 3.348 |
| 406 | 446 | 10\% | 1.924 | 342 | 351 | 3\% | 0.483 | 53 | 79 | 48\% | 3.167 | 11 | 16 | 45\% | 1.351 |
| 400 | 361 | -10\% | 2.037 | 337 | 322 | $-4 \%$ | 0.804 | 53 | ${ }^{33}$ | -37\% | 2.940 | 11 | 5 | -55\% | 2.105 |
| 164 | 81 | -50\% | 7.481 | 129 | 79 | -39\% | 4.911 | 25 | 3 | -89\% | 6.039 | 11 | 0 | -98\% | 4.489 |
| 1873 | 1876 | 0\% | 0.058 | 1567 | 1570 | 0\% | 0.063 | 250 | 250 | 0\% | 0.004 | 56 | 56 | 0\% | 0.012 |
| 324 | 193 | -41\% | 8.169 | ${ }^{273}$ | 167 | -39\% | ${ }^{7} .155$ | ${ }^{43}$ | 22 | -49\% | ${ }^{3.675}$ | 9 | 4 | -51\% | 1.731 |
| 467 | 388 | -17\% | 3.788 | 393 | 315 | -20\% | 4.114 | 61 | 61 | 0\% | 0.006 | 13 | 12 | -8\% | 0.275 |
| 146 | 125 | -15\% | 1.836 | 123 | 97 | -21\% | 2.514 | 19 | 22 | 13\% | 0.562 | 4 | 6 | 62\% | 1.077 |
| 447 | 528 | 18\% | 3.689 | 376 | 475 | 26\% | 4.809 | 59 | ${ }^{43}$ | -26\% | 2.150 | 12 | 10 | -20\% | 0.737 |
| 643 | 795 | 24\% | 5.684 | 541 | 662 | 22\% | 4.922 | 84 | 111 | 32\% | 2.721 | 17 | 22 | 28\% | 1.077 |
| 447 | 452 | 1\% | 0.250 | 376 | 374 | -1\% | 0.114 | 59 | 65 | 11\% | 0.826 | 12 | 13 | 8\% | 0.285 |
| 2473 | 2481 | 0\% | 0.164 | 2082 | 2089 | 0\% | 0.173 | 325 | 325 | 0\% | 0.009 | 66 | 67 | 1\% | 0.054 |
| 205 | 325 | 59\% | 7.380 | 173 | 263 | 53\% | ${ }^{6.150}$ | 27 | 52 | 93\% | 3.975 | 5 | 10 | 80\% | 1.587 |


| ID | Name | Link ID | Status | $\begin{gathered} \text { Site } \\ \text { Location } \end{gathered}$ | Direction | A-Node | B-Node | ID Rep | Interpeak | Off Peak |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6 | LowestoftScreenline $2-$SB | 577 | Calibration | A12 <br> Battery <br> Green <br> Road | SB | 6150 | 6160 |  |  |  |
|  |  | 585 | Callibration | Rotterdam | swb | 9130 | 7210 |  |  |  |
|  |  | 587 | Calibration | Peto Way <br> A1117 <br> Normanst <br> on Drive <br> B1375 <br> Gorleston <br> Road | SB | 7060 | 10190 |  |  |  |
|  |  | 589 | Calibration |  | swb | 7060 | 7050 |  |  |  |
|  |  | 595 | Calibration |  | SB | 8040 | 8030 |  |  |  |
|  |  | TOTAL |  |  |  | Calibration |  |  | Yes | Yes |
| 7 | Screenline 18North -Inbound | 606 | Calibration | B1074 <br> Fixton <br> Road <br> Coast <br> JTC <br> J_A-A- <br> In | NB | 6324 | 9440 |  |  |  |
|  |  | 609 | Calibraion |  | SB | 5656 | 4563 |  |  |  |
|  |  | 611 | Callibation |  | SB | 6326 | 9510 |  |  |  |
|  |  | 1464 | Calibration |  | 0 | 6280 | 6270 |  |  |  |
|  |  | TOTAL |  |  |  | Calibration |  |  | Yes | Yes |
| 8 | Screenline 18North -Outbound | 607 | Calibration | B1074 <br> Fixton <br> Road <br> Coast <br> JTC <br> JTA-A- <br> Out | SB | 9440 | 6324 |  |  |  |
|  |  | 608 | Calibration |  | NB | 4563 | 5656 |  |  |  |
|  |  | 610 | Callibration |  | NB | 9510 | 6326 |  |  |  |
|  |  | 1465 | Calibration |  | 0 | 6270 | 6280 |  |  |  |
|  |  | TOTAL |  |  |  | Calibration |  |  | Yes | Yes |
| 9 | LowestoftScreenline 4EB | 556 | Validation | A1145 <br> Lowestoft <br> Road <br> Cartinn <br> Colvile <br> London <br> JTC 1 I.E- <br> Out | EB | 5110 | 5060 |  |  |  |
|  |  | 1633 | Validation |  | EB | 6335 | 9604 |  |  |  |
|  |  | 1724 | Validation |  | nB | 6345 | 1025 |  |  |  |
|  |  | 1443 | Validation |  | 0 | 1020 | 10258 |  |  |  |
|  |  | total |  |  |  | Validation |  |  | Yes | Yes |
| 10 | LowestoftScreenline 4-WB | 557 | Validation |  | WB | 5060 | 5110 |  |  |  |
|  |  | 1634 | Validation |  | wB | 9604 | 6335 |  |  |  |
|  |  | 1723 | Validation |  | SB | 1025 | 6345 |  |  |  |
|  |  | 1442 | Validation |  | 0 | 10258 | 1020 |  |  |  |
|  |  | total |  |  |  | Validation |  |  | Yes | Yes |
| 11 | LowestoftScreenline 5 | 1769 | Validation | ${ }_{0}$ | NB | 2070 1260 | 7000 6322 |  |  |  |
|  |  | TOTAL |  |  |  | 1260 | Validazion |  | Yes | Yes |
| 12 |  <br> Lowestoft <br> Screenline 5 | 1770 | Validation | 0 | SB | 7000 | ${ }^{2070}$ |  |  |  |
|  |  | 1788 | Validation | 0 | SB | 6322 | 1260 |  |  |  |
|  |  | TOTAL |  |  |  |  | Validation |  | Yes | Yes |
| 13 | LowestoftScreenline $6-$NB | 600 | Validation | ${ }^{\text {B1385 }}$ | NB | 9460 | 9480 |  |  |  |
|  |  | 604 | Validation | B1375 Parkhill | nB | 6300 | 8070 |  |  |  |
|  |  | 1501 | Validation | JTC 9.A- | 0 | 6250 | 6260 |  |  |  |
|  |  | total |  |  |  | Validation |  |  | Yes | Yes |
| 14 | LowestoftScreenline 6SB | 601 | Validation |  | SB | 9480 | 9460 |  |  |  |
|  |  | 605 | Validation | Bi375 Parkhill | SB | 8070 | 6300 |  |  |  |
|  |  | 1500 | Validation | JTC 9_A- | 0 | 6260 | 6250 |  |  |  |


| Interpeak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| All |  |  |  | Car |  |  |  | LGV |  |  |  | HGV |  |  |  |
| Observed | Modelled | Difierence | GEE | Obsenved | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH |
| 612 | 527 | -14\% | 3.548 | 515 | 443 | -14\% | 3.286 | 80 | 67 | -17\% | 1.556 | 16 | 17 | 4\% | 0.149 |
| 141 | 113 | -20\% | 2.443 | 118 | 88 | -25\% | 2.945 | 18 | 20 | 10\% | 0.409 | 4 | 4 | 17\% | 0.314 |
| 551 | 412 | -25\% | 6.334 | 464 | 370 | -20\% | 4.592 | 72 | 32 | -55\% | 5.528 | 15 | 10 | -35\% | 1.498 |
| 652 | 775 | 19\% | 4.593 | 549 | 649 | 18\% | 4.089 | 86 | 106 | 24\% | 2.113 | 17 | 19 | 11\% | 0.444 |
| 474 | 476 | 0\% | 0.065 | 399 | 399 | 0\% | 0.023 | 62 | 66 | 6\% | 0.479 | 13 | 11 | -16\% | 0.576 |
| 2635 | 2628 | 0\% | 0.135 | 2218 | 2213 | 0\% | 0.110 | 346 | 344 | -1\% | 0.112 | 71 | 71 | 1\% | 0.042 |
| 117 | 186 | 60\% | 5.649 | 98 | 155 | 58\% | 5.025 | 15 | 26 | 73\% | 2.432 | 3 | 5 | 60\% | 0.934 |
| 71 | 18 | -75\% | 8.086 | 60 | 16 | -73\% | 7.130 | 9 | 1 | -88\% | 3.625 | 2 | 0 | -84\% | 1.518 |
| 60 | 0 | -99\% | 10.855 | 51 | 0 | -99\% | 9.942 | 8 | 0 | -100\% | 3.973 | 2 | 0 | -100\% | 1.794 |
| 804 | 825 | 3\% | 0.765 | 662 | 679 | 3\% | 0.657 | 116 | 119 | 3\% | 0.301 | 25 | 27 | 6\% | 0.305 |
| 1052 | 1030 | -2\% | 0.689 | 871 | 851 | -2\% | 0.704 | 148 | 147 | -1\% | 0.149 | 32 | 32 | 1\% | 0.042 |
| 102 | 160 | 58\% | 5.124 | 85 | ${ }^{133}$ | 55\% | 4.517 | 13 | ${ }^{23}$ | 73\% | 2.288 | 3 | 4 | 62\% | 0.888 |
| 49 | 16 | -67\% | 5.769 | 41 | ${ }^{13}$ | -68\% | 5.399 | 6 | 3 | -53\% | 1.588 | 1 | 0 | -100\% | 1.624 |
| 63 | 1 | -98\% | 10.897 | 53 | 1 | -99\% | 10.116 | 8 | 0 | -98\% | 3.942 | 2 | 0 | -74\% | 1.217 |
| 814 | 832 | 2\% | 0.633 | 675 | 692 | 3\% | 0.650 | 115 | 115 | 0\% | 0.029 | 24 | 25 | 4\% | 0.177 |
| 1027 | 1009 | -2\% | 0.557 | 855 | 838 | -2\% | 0.560 | 143 | 141 | -1\% | 0.123 | 30 | 30 | 0\% | 0.004 |
| 307 | 343 | 12\% | 2.003 | 259 | 270 | 4\% | 0.681 | 40 | 58 | 44\% | 2.534 | 8 | 16 | 88\% | 2.111 |
| 205 | 114 | -45\% | 7.240 | 160 | 105 | -35\% | 4.839 | 31 | 8 | -74\% | 5.204 | 13 | 1 | -95\% | 4.733 |
| 351 | 287 | -18\% | 3.573 | 275 | 242 | -12\% | 2.066 | 54 | 42 | -23\% | 1.773 | ${ }^{23}$ | 4 | -81\% | 5.027 |
| 801 | 842 | 5\% | 1.429 | 642 | 665 | 4\% | 0.926 | 127 | 141 | 12\% | 1.284 | 33 | 36 | 7\% | 0.415 |
| 1665 | 1587 | -5\% | 1.939 | 1336 | 1281 | -4\% | 1.498 | 252 | 249 | -1\% | 0.177 | 77 | 56 | -27\% | 2.597 |
| 323 | 368 | 14\% | 2.414 | 272 | 283 | 4\% | 0.680 | 42 | 64 | 50\% | 2.924 | 9 | ${ }^{21}$ | 141\% | 3.185 |
| 197 | 102 | -48\% | 7.736 | 154 | 95 | -38\% | 5.303 | 30 | 6 | -80\% | 5.689 | 13 | 1 | -89\% | 4.240 |
| 372 | 322 | -13\% | 2.688 | 291 | 269 | -7\% | 1.271 | 57 | 46 | -19\% | 1.526 | 24 | 6 | -74\% | 4.597 |
| 770 | 803 | 4\% | 1.169 | 609 | 634 | 4\% | 1.006 | 122 | 131 | 7\% | 0.790 | 39 | 38 | -3\% | 0.193 |
| 1662 | 1595 | -4\% | 1.662 | 1326 | 1282 | -3\% | 1.220 | 252 | 247 | -2\% | 0.316 | 84 | 66 | -21\% | 2.078 |
| ${ }_{911}^{947}$ | 1187 849 | 25\% | $7.352$ | 811 782 | $\begin{aligned} & 1000 \\ & 724 \end{aligned}$ | 23\% | $6.293$ | 100 | ${ }^{155}$ | 55\% | 4.891 | ${ }_{31}^{37}$ | ${ }^{32}$ | ${ }^{-12 \%}$ | ${ }^{0.755}$ |
| -1857 | 2036 | 10\% | --4.047 | 1592 | 1724 | 8\% | -3.231 | -197 | 259 | -71\% | - -0.085 | 68 | 53 | -22\% | -2.023 |
| 964 | 1063 | 10\% | ${ }^{3.120}$ | 824 | 892 | 8\% | 2.321 | 99 | 145 | 46\% | 4.119 | 40 | 26 | -35\% | 2.458 |
| 1049 | 1125 | 7\% | -2.284 | . 904 | 939 | 4\% | -1.160 | 109 | 150 | 38\% | 3.612 | 37 | 36 | -3\% | 0.158 |
| --2013 | 2188 | -9\% | -3.810 | 1728 | 1831 | 6\% | - 2.447 | 208 | 294 | 42\% | 5.461 | 77 | 62 | -20\% | 1.809 |
| 88 | 66 | -25\% | 2.544 | 74 | 55 | -26\% | 2.419 | 12 | 10 | -13\% | 0.465 | 2 | 1 | -58\% | ${ }^{1.052}$ |
| 264 | 278 | 5\% | 0.833 | 223 | 229 | 3\% | 0.415 | 35 | 40 | 14\% | 0.803 | 7 | 10 | 37\% | 0.898 |
| 588 | -592 | .1\% | -0.155 | 493 | 497 | 1\% | -0.207 | 80 | 79 | -1\% | -0.133 | 15 | 16 | 2\% | 0.087 |
| 940 | 936 | -1\% | 0.157 | 789 | 781 | -1\% | 0.306 | 127 | 129 | 2\% | 0.193 | 25 | 26 | 6\% | 0.312 |
| 99 | 69 | -30\% | ${ }^{3.283}$ | ${ }^{83}$ | 59 | -29\% | 2.837 | ${ }^{13}$ | 9 | -30\% | ${ }^{1.186}$ | 3 | 0 | -82\% | 1.742 |
| 194 | 250 | 29\% | ${ }^{3.757}$ | 163 | 205 | 25\% | ${ }^{3.059}$ | 25 | 38 | 51\% | 2.283 | 5 | 7 | 30\% | 0.646 |
| 612 | 617 | 1\% | 0.204 | 507 | 512 | 1\% | 0.197 | 84 | 85 | 1\% | 0.064 | 20 | 20 | 0\% | 0.001 |



| Screenline |  |  |
| :---: | :---: | :---: |
| ID | Name | Type |
| 1 | Screenline 18 South - <br> Inbound | Calibration |
| 2 | Screnline 18 South - <br> Outbound | Calibration |
| 3 | Lowestoft Screenline <br> $1-$ NB | Calibration |
| 4 | Lowestoft Screenline <br> $1-$ SB | Calibration |
| 5 | Lowestoft Screenline <br> $2-$ NB | Calibration |
| 6 | Lowestoft Screenline <br> $2-$ SB | Callibration |
| 7 | Screenline 18 North - <br> Inbound | Calibration |
| 8 | Screenline 18 North - <br> Outbound | Calibration |
| 9 | Lowestoft Screenline <br> $4-$ EB | Validation |
| 10 | Lowestoft Screenline <br> $4-$ WB | Validation |
| 11 | Lowestoft Screenline <br> - NB | Validation |
| 12 | Lowestoft Screenline <br> $5-$ SB | Validation |
| 13 | Lowestoft Screenline <br> $6-$ NB | Validation |
| 14 | Lowestoft Screenline <br> $6-$ SB | Validation |


| PM Peak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| All |  |  |  | Car |  |  |  | LGV |  |  |  | HGV |  |  |  |
| Observed | Modelled | Difference | GEH | Observed | Modelled | Difference | GEH | Observed | Modelled | Difference | GEH | Observed | Modelled | Difference | GEH |
| 1792 | 1801 | 1\% | 0.225 | 1592 | 1593 | 0\% | 0.033 | 189 | 188 | -1\% | 0.088 | 11 | 20 | 90\% | 2.416 |
| 1517 | 1536 | 1\% | 0.482 | 1348 | 1354 | 0\% | 0.162 | 160 | 161 | 0\% | 0.044 | 9 | 21 | 138\% | 3.172 |
| 2145 | 2145 | 0\% | 0.007 | 1902 | 1902 | 0\% | 0.010 | 227 | 227 | 0\% | 0.025 | 16 | 17 | 2\% | 0.067 |
| 2284 | 2292 | 0\% | 0.163 | 2025 | 2032 | 0\% | 0.146 | 242 | 243 | 0\% | 0.059 | 17 | 18 | 2\% | 0.066 |
| 2471 | 2473 | 0\% | 0.036 | 2195 | 2197 | 0\% | 0.039 | 261 | 261 | 0\% | 0.015 | 15 | 15 | 1\% | 0.044 |
| 3219 | 3228 | 0\% | 0.157 | 2860 | 2869 | 0\% | 0.174 | 340 | 341 | 0\% | 0.033 | 19 | 18 | -5\% | 0.242 |
| 1851 | 1829 | -1\% | 0.512 | 1631 | 1608 | -1\% | 0.576 | 209 | 209 | 0\% | 0.015 | 10 | 12 | 14\% | 0.437 |
| 1211 | 1207 | 0\% | 0.112 | 1080 | 1077 | 0\% | 0.095 | 122 | 121 | -1\% | 0.084 | 9 | 9 | 2\% | 0.052 |
| 2021 | 1899 | -6\% | 2.763 | 1761 | 1652 | -6\% | 2.641 | 234 | 232 | -1\% | 0.128 | 27 | 15 | -42\% | 2.467 |
| 1725 | 1784 | 3\% | 1.404 | 1505 | 1579 | 5\% | 1.886 | 195 | 183 | -6\% | 0.858 | 26 | 22 | -13\% | 0.704 |
| 2041 | 2113 | 4\% | 1.570 | 1775 | 1887 | 6\% | 2.621 | 237 | 208 | -12\% | 1.929 | 29 | 17 | -41\% | 2.463 |
| 2528 | 3001 | 19\% | 8.992 | 2279 | 2706 | 19\% | 8.558 | 222 | 280 | 26\% | 3.673 | 27 | 14 | -47\% | 2.793 |
| 1129 | 1116 | -1\% | 0.389 | 1021 | 996 | -2\% | 0.784 | 101 | 113 | 12\% | 1.182 | 8 | 7 | -4\% | 0.123 |
| 1598 | 1699 | 6\% | 2.492 | 1412 | 1506 | 7\% | 2.464 | 178 | 183 | 3\% | 0.363 | 8 | 11 | 26\% | 0.713 |


| ID | Name | Link ID | Status | $\begin{gathered} \text { Site } \\ \text { Location } \\ \hline \end{gathered}$ | Direction | A-Node | B-Node | ID Rep | AM Peak | PM Peak | Ofif Peak |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Screenline 18 South Inbound | 548 | Calloration |  | EB | 4567 | 5070 |  |  |  |  |
|  |  | 550 | Callibation |  | Eb | 4566 | 5000 |  |  |  |  |
|  |  | 552 | Callibation |  | NB | 9600 | 5010 |  |  |  |  |
|  |  | 554 | Calibration |  | EB | 4512 | 2000 |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 558 | Calibration |  | NB | 5390 | 1000 |  |  |  |  |
|  |  | total |  |  |  | Calliration |  |  | Yes | Yes | Yes |
| 2 | $\left\|\begin{array}{c} \text { Screenline } 18 \\ \text { South - } \\ \text { Outbound } \end{array}\right\|$ | 549 | Callibation |  | wB | 5070 | 4567 |  |  |  |  |
|  |  | 551 | Calibration |  | wв | 5000 | 4566 |  |  |  |  |
|  |  | 553 | Calloration |  | SB | 5010 | 9600 |  |  |  |  |
|  |  | 555 | Calibration |  | wB | 2000 | 4512 |  |  |  |  |
|  |  | 559 | Calibration |  |  |  |  |  |  |  |  |
|  |  | TOTAL |  |  |  | Callibation |  |  | Yes | Yes | Yes |
| 3 | LowestoftScreenline 1NB | 560 | Callibation | London <br> A12 Tom <br> Cris Way <br> A1117 <br> A146 <br> Aeccles <br> B <br> Road <br> Long <br> Load | NEB | 9606 | 1040 |  |  |  |  |
|  |  | 562 | Callibation |  | NEB | 3000 | 6314 |  |  |  |  |
|  |  | 564 | Callibation |  | NB | 3030 | 3040 |  |  |  |  |
|  |  | 566 | Calliration |  | NEB | 6406 | 6404 |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 1618 | Calliration |  | NB | 3010 | 5180 |  |  |  |  |
|  |  | total |  |  |  | Callbation |  |  | Yes | Yes | Yes |
| 4 | Lowestoft <br> Screenline 1 <br> SB | 561 | Callioration | London <br> A12 Tom <br> Crisp Way <br> A1117 <br> A116 <br> Beccles <br> BRad <br> Roang <br> Road <br> Roan | SWB | 1040 | 9606 |  |  |  |  |
|  |  | 563 | Callibation |  | swb | 6314 | 3000 |  |  |  |  |
|  |  | 565 | Calloration |  | SB | 3040 | 3030 |  |  |  |  |
|  |  | 567 | Calibration |  | swb | 6404 | 6406 |  |  |  |  |
|  |  | 1617 | Calibration |  | SB | 5180 | 3010 |  |  |  |  |
|  |  | total |  |  |  | Calibration |  |  | Yes | Yes | Yes |
| 5 | Lowestoft <br> Screenline $2-$ <br> NB | 574 | Callibation | Katwik <br> A12 <br> Battery <br> Green <br> Road <br> Roterdam <br> Peeto Way <br> A1117 <br> Normant <br> on Prive <br> B135 <br> Borseston <br> Goad | NB | 6040 | 6431 |  |  |  |  |
|  |  | 576 | Callibation |  | NB | 6160 | 6150 |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 584 | Callibation |  | neb | 7210 | 9130 |  |  |  |  |
|  |  | 586 | Calibration |  | NB | 10190 | 7060 |  |  |  |  |
|  |  | 588 | Calliration |  | NEB | 7050 | 7060 |  |  |  |  |
|  |  | 594 | Calibration |  | NB | 8030 | 8040 |  |  |  |  |
|  |  | total |  |  |  | Calibration |  |  | Yes | Yes | Yes |
|  |  | 575 | Callibation | Katwik | SB | 6431 | 6040 |  |  |  |  |


| PM Peak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| All |  |  |  | Car |  |  |  | LGV |  |  |  | HGV |  |  |  |
| Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH |
| 29 | 47 | 60\% | 2.838 | 26 | 44 | 69\% | 3.011 | 3 | 3 | -4\% | 0.078 | 0 | 0 | -90\% | 0.500 |
| 77 | 0 | -100\% | 12.431 | 69 | 0 | -100\% | 11.718 | 8 | 0 | -100\% | 4.043 | 0 | 0 | -99\% | 0.939 |
| 42 | 56 | 32\% | 1.938 | 37 | 55 | 46\% | 2.548 | 4 | 1 | -79\% | 2.134 | 0 | 0 | -96\% | 0.666 |
| 890 | 1058 | 19\% | 5.395 | 791 | 938 | 19\% | 5.015 | 94 | 108 | 15\% | 1.396 | 5 | 12 | 132\% | 2.346 |
| 754 | 641 | -15\% | 4.263 | 669 | 557 | -17\% | 4.542 | 80 | 76 | -4\% | 0.389 | 4 | 8 | 76\% | 1.359 |
| 1792 | 1801 | 1\% | 0.225 | 1592 | 1593 | 0\% | 0.033 | 189 | 188 | -1\% | 0.088 | 11 | 20 | 90\% | 2.416 |
| ${ }^{21}$ | 52 | 146\% | 5.082 | 19 | 45 | 143\% | 4.712 | 2 | 6 | 184\% | 1.983 | 0 | 0 | -100\% | 0.497 |
| 41 | 0 | -100\% | 9.011 | 36 | 0 | -100\% | 8.493 | 4 | 0 | -100\% | 2.930 | 0 | 0 | -100\% | 0.690 |
| 26 | 29 | 12\% | 0.583 | ${ }^{23}$ | 27 | 17\% | 0.794 | 3 | 1 | -56\% | 1.103 | 0 | 1 | 413\% | 0.926 |
| 659 | 833 | 26\% | 6.368 | 586 | 739 | 26\% | 5.943 | 70 | 84 | 21\% | 1.631 | 4 | 11 | 174\% | 2.500 |
| 770 | 621 | -19\% | 5.623 | 684 | 542 | -21\% | 5.720 | 81 | 69 | -15\% | 1.381 | 5 | 10 | 118\% | 1.983 |
| 1517 | 1536 | 1\% | 0.482 | 1348 | 1354 | 0\% | 0.162 | 160 | 161 | 0\% | 0.044 | 9 | 21 | 138\% | 3.172 |
| 517 | 580 | 12\% | 2.718 | 459 | 520 | 13\% | 2.769 | 55 | 59 | 8\% | 0.545 | 3 | 1 | -57\% | 1.178 |
| 499 | 550 | 10\% | 2.225 | 443 | 481 | 9\% | 1.787 | 53 | ${ }^{63}$ | 19\% | 1.315 | 3 | 5 | 87\% | 1.237 |
| 418 | 495 | 19\% | 3.635 | 371 | 434 | 17\% | 3.138 | 44 | 58 | 32\% | 1.986 | 2 | 3 | 20\% | 0.304 |
| 532 | 446 | -16\% | ${ }^{3.874}$ | 472 | 401 | -15\% | 3.414 | 56 | 38 | -32\% | 2.619 | 3 | 7 | 118\% | 1.656 |
| 180 | 74 | -59\% | 9.428 | 156 | 65 | -58\% | 8.636 | 19 | 9 | -55\% | 2.866 | 5 | 0 | -98\% | 3.024 |
| 2145 | 2145 | 0\% | 0.007 | 1902 | 1902 | 0\% | 0.010 | 227 | 227 | 0\% | 0.025 | 16 | 17 | 2\% | 0.067 |
| 496 | 587 | 18\% | 3.919 | 440 | 528 | 20\% | ${ }^{3.961}$ | 52 | 56 | 7\% | 0.504 | 3 | 3 | 12\% | 0.192 |
| 703 | 825 | 17\% | 4.440 | 624 | 751 | 20\% | 4.834 | 74 | 67 | -9\% | 0.836 | 4 | 7 | 73\% | 1.265 |
| 414 | 469 | 13\% | 2.597 | 368 | 395 | 7\% | ${ }^{1.382}$ | 44 | 70 | 60\% | ${ }^{3.482}$ | 2 | 4 | 54\% | 0.744 |
| 487 | 336 | -31\% | 7.456 | 433 | 291 | -33\% | 7.460 | 52 | ${ }^{43}$ | -17\% | 1.285 | 3 | 2 | -19\% | 0.329 |
| 184 | 75 | -59\% | 9.615 | 160 | 67 | -58\% | 8.673 | 20 | 7 | -67\% | 3.651 | 5 | 1 | -78\% | 2.226 |
| 2284 | 2292 | 0\% | 0.163 | 2025 | 2032 | 0\% | 0.146 | 242 | 243 | 0\% | 0.059 | 17 | 18 | 2\% | 0.066 |
| 305 | 183 | -40\% | 7.830 | 271 | 170 | -37\% | 6.841 | 32 | 12 | -62\% | 4.250 | 2 | 1 | -40\% | 0.596 |
| 454 | 376 | -17\% | ${ }^{3.806}$ | 403 | 328 | -19\% | 3.909 | 48 | 45 | -6\% | 0.398 | 3 | 3 | -2\% | 0.031 |
| 149 | 118 | -21\% | 2.764 | ${ }^{133}$ | 107 | -19\% | 2.361 | 16 | 11 | -34\% | 1.463 | 1 | 0 | -89\% | 1.121 |
| 420 | 587 | 40\% | 7.457 | 373 | 529 | 42\% | 7.341 | 44 | 58 | 30\% | 1.841 | 2 | 1 | -70\% | 1.351 |
| 604 | 697 | 15\% | ${ }^{3.665}$ | 536 | 623 | 16\% | 3.589 | 64 | 68 | 7\% | 0.519 | 4 | 6 | 81\% | 1.287 |
| 539 | 512 | -5\% | 1.190 | 479 | 441 | -8\% | 1.787 | 57 | 68 | 18\% | 1.329 | 3 | 4 | 18\% | 0.300 |
| 2471 | 2473 | 0\% | 0.036 | 2195 | 2197 | 0\% | 0.039 | 261 | 261 | 0\% | 0.015 | 15 | 15 | 1\% | 0.044 |
| 223 | 407 | 82\% | 10.331 | 199 | 367 | 85\% | 10.033 | 24 | 39 | 66\% | 2.787 | 1 | 0 | -75\% | 1.083 |



| All |  |  |  | Car PM Peak LGV |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | HGV |
| Observed | Modelled | Difierence | GEH |  |  |  |  |  |  |  |  | Observed | Modelled | Difierence | GEH | Observed | Modelled | Difierence | GEH | Observed | Modelled | Difference | GEH |
| 905 | 791 | -13\% | 3.919 | 804 | 722 | -10\% | 2.977 | 96 | 66 | -31\% | 3.265 | 5 | 3 | -47\% | 1.243 |
| 145 | 124 | -15\% | 1.872 | 129 | 110 | -15\% | 1.727 | 15 | 13 | -17\% | 0.685 | 1 | 1 | -29\% | 0.286 |
| 605 | 398 | -34\% | 9.242 | 537 | 362 | -33\% | 8.256 | 64 | 31 | -52\% | 4.852 | 4 | 5 | 43\% | 0.732 |
| 644 | 843 | 31\% | 7.309 | 572 | 749 | 31\% | 6.865 | 68 | 91 | 34\% | 2.586 | 4 | 4 | -4\% | 0.081 |
| 697 | 666 | -4\% | 1.191 | 619 | 560 | -10\% | 2.457 | 74 | 101 | 37\% | 2.913 | 4 | 5 | 32\% | 0.609 |
| 3219 | 3228 | 0\% | 0.157 | 2860 | 2869 | 0\% | 0.174 | 340 | 341 | 0\% | 0.033 | 19 | 18 | -5\% | 0.242 |
| 217 | 215 | -1\% | 0.144 | 193 | 179 | -7\% | ${ }^{1.0172}$ | ${ }^{23}$ | 34 | 49\% | 2.100 | 1 | 2 | 36\% | 0.377 |
| 75 | 45 | -40\% | 3.916 | 67 | 40 | -41\% | 3.734 | 8 | 5 | -33\% | 1.037 | 0 | 0 | -100\% | 0.942 |
| 68 | 1 | -99\% | 11.508 | 61 | 1 | -99\% | 10.831 | 7 | 0 | -100\% | 3.785 | 0 | 0 | -100\% | 0.894 |
| 1489 | 1568 | 5\% | 2.001 | 1310 | 1388 | 6\% | 2.124 | 171 | 169 | -1\% | 0.122 | 8 | 10 | 23\% | 0.608 |
| 1851 | 1829 | -1\% | 0.512 | 1631 | 1608 | -1\% | 0.576 | 209 | 209 | 0\% | 0.015 | 10 | 12 | 14\% | 0.437 |
| 142 | 181 | 28\% | 3.098 | 126 | 158 | 26\% | 2.736 | 15 | ${ }^{21}$ | 40\% | 1.420 | 1 | 2 | 84\% | 0.644 |
| 56 | 18 | -68\% | 6.267 | 49 | 16 | -69\% | 5.948 | 6 | 2 | -63\% | 1.856 | 0 | 0 | -100\% | 0.808 |
| 51 | 27 | -48\% | 3.922 | 46 | 27 | -41\% | 3.144 | 5 | 0 | -98\% | ${ }^{3} .204$ | 0 | 0 | -100\% | 0.774 |
| 962 | 981 | 2\% | 0.616 | 859 | 877 | 2\% | 0.579 | 96 | 98 | 2\% | 0.211 | 7 | 7 | 1\% | 0.030 |
| 1211 | 1207 | 0\% | 0.112 | 1080 | 1077 | 0\% | 0.095 | 122 | 121 | -1\% | 0.084 | 9 | 9 | 2\% | 0.052 |
| 456 | 495 | 8\% | ${ }^{1.754}$ | 405 | 422 | 4\% | ${ }^{0.823}$ | 48 | 65 | 35\% | 2.265 | 3 | 7 | 166\% | 2.007 |
| 219 | 101 | -54\% | 9.286 | 190 | 94 | -50\% | 8.014 | ${ }^{23}$ | 7 | -70\% | 4.221 | 6 | 0 | -96\% | 3.210 |
| 566 | 446 | -21\% | 5.325 | 490 | 398 | -19\% | 4.350 | 61 | 46 | -25\% | 2.039 | 15 | 2 | -88\% | 4.552 |
| 780 | 857 | 10\% | 2.681 | 675 | 737 | 9\% | 2.308 | 102 | 114 | 12\% | 1.184 | 3 | 6 | 101\% | 1.434 |
| 2021 | 1899 | -6\% | 2.763 | 1761 | 1652 | -6\% | 2.641 | 234 | 232 | -1\% | 0.128 | 27 | 15 | -42\% | 2.467 |
| 444 | 525 | 18\% | ${ }^{3.662}$ | 395 | 469 | 19\% | ${ }^{3.581}$ | 47 | 48 | 1\% | 0.075 | 3 | 8 | 217\% | 2.427 |
| 207 | 149 | -28\% | 4.278 | 179 | 140 | -22\% | 3.084 | 22 | 9 | -57\% | 3.182 | 5 | 0 | -100\% | 3.313 |
| 355 | 360 | 2\% | 0.288 | 307 | 324 | 6\% | 0.954 | 38 | 34 | -11\% | 0.706 | 9 | 2 | -77\% | 3.026 |
| 720 | 749 | 4\% | 1.101 | 624 | 646 | 3\% | 0.857 | 87 | 92 | 5\% | 0.483 | 8 | 12 | 45\% | 1.163 |
| 1725 | 1784 | 3\% | 1.404 | 1505 | 1579 | 5\% | 1.886 | 195 | 183 | -6\% | 0.858 | 26 | 22 | -13\% | 0.704 |
| 1055 986 | 1243 870 | - $\begin{gathered}18 \% \\ -12 \%\end{gathered}$ | 5.536 <br> 3.811 | 892 883 | 1104 784 | 24\% ${ }_{\text {24\% }}$ | 6.698 3.443 | 144 93 | 128 80 80 | ${ }^{-11 \%}$ | 1.394 1.345 1 | 19 | ${ }^{11}$ | ${ }^{-40 \%}$ | 1.968 1.482 1.4 |
| 2041 | 2113 | 4\% | - 1.570 | 1775 | 1887 | 6\% | --3.621 | - 237 | 208 | 㱓 | -1.929 | 29 | $\stackrel{17}{17}$ | -42\% | 2.463 |
| 1092 | 1248 | 14\% | 4.547 | 975 | 1100 | 13\% | 3.868 | 105 | 141 | 35\% | 3.281 | 12 | 6 | -46\% | 1.810 |
| 1436 | -1753 | -22\% | 7.945 | - 1304 | 1607 | -23\% | -7.934 | 117 | -139 | -19\% | 1.1226 | 15 | 8 | -48\% | -2.128 |
| 2528 | 3001 | -19\% | ${ }^{8.992}$ | 2279 | 2706 | -19\% | -8.558 | 222 | 280 | 26\% | -3.673 | 27 | 14 | -47\% | -2.793 |
| 103 | 85 | -18\% | ${ }^{1.880}$ | 92 | 76 | -17\% | ${ }^{1.664}$ | 11 | 9 | -22\% | 0.763 | 1 | 0 | -100\% | 1.100 |
| 312 | 310 | -1\% | 0.148 | 277 | 261 | -6\% | 0.989 | 33 | 45 | 38\% | 1.981 | 2 | 3 | 66\% | 0.774 |
| 713 | 721 | 1\% | -0.291 | 652 | 658 | .1\% | -0.257 | 57 | 59 | 4\% | -0.286 | 5 | 4 | -19\% | -0.437 |
| 1129 | 1116 | -1\% | 0.389 | 1021 | 996 | -2\% | 0.784 | 101 | 113 | 12\% | 1.182 | 8 | 7 | -4\% | 0.123 |
| 92 | 84 | -8\% | 0.828 | 82 | 76 | -7\% | 0.685 | 10 | 9 | -12\% | 0.385 | 1 | 0 | -98\% | 1.004 |
| 482 | 587 | 22\% | 4.558 | 428 | 518 | 21\% | 4.140 | 51 | 65 | 27\% | 1.785 | 3 | 5 | 63\% | 0.923 |
| 1024 | 1028 | 0\% | 0.112 | 902 | 912 | 1\% | 0.338 | 117 | 110 | -6\% | 0.706 | 5 | 6 | 19\% | 0.404 |



## Appendix B




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\end{tabular}










# Appendix C 

JOURNEY TIME GRAPHS
いゝ|"

## AM Peak






















Inter Peak





















PM Peak





















## Appendix D

ROUTE CHOICE VALIDATION
いい|

## AM Peak

From Zone 408 To Zone 465 - User Class 2


## From Zone 408 To Zone 465 - User Class 10



From Zone 465 To Zone 408 - User Class 2


## From Zone 465 To Zone 408 - User Class 10



From Zone 84 To Zone 593 - User Class 2


From Zone 84 To Zone 593 - User Class 10


From Zone 593 To Zone 84 - User Class 2


From Zone 593 To Zone 84 - User Class 10


From Zone 767 To Zone 434 - User Class 2


From Zone 767 To Zone 434 - User Class 10


From Zone 434 To Zone 767 - User Class 2


From Zone 434 To Zone 767 - User Class 10


From Zone 84 To Zone $\mathbf{7 8 2}$ - User Class 2


From Zone 84 To Zone 782 - User Class 10


From Zone 782 To Zone 84 - User Class 2


From Zone 782 To Zone 84 - User Class 10


From Zone 586 To Zone 781 - User Class 2


From Zone 586 To Zone 781 - User Class 10


From Zone 781 To Zone 586 - User Class 2


From Zone 781 To Zone 586 - User Class 10


From Zone 408 To Zone 582 - User Class 2


From Zone 408 To Zone 582 - User Class 10


From Zone 582 To Zone 408 - User Class 2


From Zone 582 To Zone 408 - User Class 10


From Zone 427 To Zone 781 - User Class 2


From Zone 427 To Zone 781 - User Class 10


From Zone 781 To Zone 427 - User Class 2


From Zone 781 To Zone 427 - User Class 10


From Zone 639 To Zone 463 - User Class 2


From Zone 639 To Zone 463 - User Class 10


From Zone 463 To Zone 639 - User Class 2


From Zone 463 To Zone 639 - User Class 10


From Zone 409 To Zone 584 - User Class 2


From Zone 409 To Zone 584 - User Class 10


From Zone 584 To Zone 409 - User Class 2


From Zone 584 To Zone 409 - User Class 10


From Zone 586 To Zone 762 - User Class 2


From Zone 586 To Zone 762 - User Class 10


From Zone 762 To Zone 586 - User Class 2


From Zone 762 To Zone 586 - User Class 10


From Zone 432 To Zone 761 - User Class 2


From Zone 432 To Zone 761 - User Class 10


From Zone 761 To Zone 432 - User Class 2


From Zone 761 To Zone 432 - User Class 10


From Zone 416 To Zone 588 - User Class 2


## From Zone 416 To Zone 588 - User Class 10



From Zone 588 To Zone 416 - User Class 2


From Zone 588 To Zone 416 - User Class 10


From Zone 593 To Zone 598 - User Class 2


From Zone 593 To Zone 598 - User Class 10


From Zone 598 To Zone 593 - User Class 2


From Zone 598 To Zone 593 - User Class 10


From Zone 773 To Zone 782 - User Class 2


From Zone 773 To Zone 782 - User Class 10


From Zone 782 To Zone 773 - User Class 2


From Zone 782 To Zone 773 - User Class 10


From Zone 409 To Zone 589 - User Class 2


From Zone 409 To Zone 589 - User Class 10


From Zone 589 To Zone 409 - User Class 2


From Zone 589 To Zone 409 - User Class 10


From Zone 587 To Zone 779 - User Class 2


## From Zone 587 To Zone 779 - User Class 10



From Zone 779 To Zone 587 - User Class 2


## From Zone 779 To Zone 587 - User Class 10



From Zone 766 To Zone 427 - User Class 2


From Zone 766 To Zone 427 - User Class 10


From Zone 427 To Zone 766 - User Class 2


From Zone 427 To Zone 766 - User Class 10


From Zone 819 To Zone 588 - User Class 2


From Zone 819 To Zone 588 - User Class 10


From Zone 588 To Zone 819 - User Class 2


From Zone 588 To Zone 819 - User Class 10


## Inter Peak

From Zone 408 To Zone 465 - User Class 6


From Zone 408 To Zone 465 - User Class 10


From Zone 465 To Zone 408 - User Class 6


From Zone 465 To Zone 408 - User Class 10


From Zone 84 To Zone 593 - User Class 6


From Zone 84 To Zone 593 - User Class 10


From Zone 593 To Zone 84 - User Class 6


From Zone 593 To Zone 84 - User Class 10


From Zone 767 To Zone 434 - User Class 6


From Zone 767 To Zone 434 - User Class 10


From Zone 434 To Zone 767 - User Class 6


From Zone 434 To Zone 767 - User Class 10


From Zone 84 To Zone 782 - User Class 6


From Zone 84 To Zone 782 - User Class 10


From Zone 782 To Zone 84 - User Class 6


From Zone 782 To Zone 84 - User Class 10


From Zone 586 To Zone 781 - User Class 6


From Zone 586 To Zone 781 - User Class 10


From Zone 781 To Zone 586 - User Class 6


From Zone $\mathbf{7 8 1}$ To Zone $\mathbf{5 8 6}$ - User Class 10


From Zone 408 To Zone 582 - User Class 6


From Zone 408 To Zone 582 - User Class 10


From Zone 582 To Zone 408 - User Class 6


From Zone 582 To Zone 408 - User Class 10


From Zone 427 To Zone 781 - User Class 6


From Zone 427 To Zone 781 - User Class 10


From Zone 781 To Zone 427 - User Class 6


From Zone 781 To Zone 427 - User Class 10


From Zone 639 To Zone 463 - User Class 6


From Zone 639 To Zone 463 - User Class 10


From Zone 463 To Zone 639 - User Class 6


From Zone 463 To Zone 639 - User Class 10


From Zone 409 To Zone 584 - User Class 6


From Zone 409 To Zone 584 - User Class 10


From Zone 584 To Zone 409 - User Class 6


From Zone 584 To Zone 409 - User Class 10


From Zone 586 To Zone 762 - User Class 6


From Zone 586 To Zone 762 - User Class 10


From Zone 762 To Zone 586 - User Class 6


From Zone 762 To Zone 586 - User Class 10


From Zone 432 To Zone 761 - User Class 6


From Zone 432 To Zone 761 - User Class 10


From Zone 761 To Zone 432 - User Class 6


From Zone 761 To Zone 432 - User Class 10


From Zone 416 To Zone 588 - User Class 6


## From Zone 416 To Zone 588 - User Class 10



From Zone 588 To Zone 416 - User Class 6


From Zone 588 To Zone 416 - User Class 10


From Zone 593 To Zone 598 - User Class 6


From Zone 593 To Zone 598 - User Class 10


From Zone 598 To Zone 593 - User Class 6


From Zone 598 To Zone 593 - User Class 10


From Zone 773 To Zone 782 - User Class 6


From Zone 773 To Zone 782 - User Class 10


From Zone 782 To Zone 773 - User Class 6


From Zone 782 To Zone 773 - User Class 10


From Zone 409 To Zone 589 - User Class 6


From Zone 409 To Zone 589 - User Class 10


From Zone 589 To Zone 409 - User Class 6


From Zone 589 To Zone 409 - User Class 10


From Zone 587 To Zone 779 - User Class 6


## From Zone 587 To Zone 779 - User Class 10



From Zone 779 To Zone 587 - User Class 6


From Zone 779 To Zone 587 - User Class 10


From Zone 766 To Zone 427 - User Class 6


From Zone 766 To Zone 427 - User Class 10


From Zone 427 To Zone 766 - User Class 6


From Zone 427 To Zone 766 - User Class 10




From Zone 588 To Zone 819 - User Class 6


From Zone 588 To Zone 819 - User Class 10


PM Peak
Project Name: OD Tree Plots -
From Zone 408 To Zone 465 - User Class 1


## From Zone 408 To Zone 465 - User Class 10



From Zone 465 To Zone 408 - User Class 1


## From Zone 465 To Zone 408 - User Class 10



From Zone 84 To Zone 593 - User Class 1


From Zone 84 To Zone 593 - User Class 10


From Zone 593 To Zone 84 - User Class 1


From Zone 593 To Zone 84 - User Class 10


From Zone 767 To Zone 434 - User Class 1


From Zone 767 To Zone 434 - User Class 10


From Zone 434 To Zone 767 - User Class 1


From Zone 434 To Zone 767 - User Class 10


From Zone 84 To Zone $\mathbf{7 8 2}$ - User Class 1


From Zone 84 To Zone 782 - User Class 10


From Zone 782 To Zone 84 - User Class 1


From Zone 782 To Zone 84 - User Class 10


From Zone 586 To Zone 781 - User Class 1


From Zone 586 To Zone 781 - User Class 10


From Zone 781 To Zone 586 - User Class 1


From Zone 781 To Zone 586 - User Class 10


From Zone 408 To Zone 582 - User Class 1


From Zone 408 To Zone 582 - User Class 10


From Zone 582 To Zone 408 - User Class 1


From Zone 582 To Zone 408 - User Class 10


From Zone 427 To Zone 781 - User Class 1


From Zone 427 To Zone 781 - User Class 10


From Zone 781 To Zone 427 - User Class 1


From Zone 781 To Zone 427 - User Class 10


From Zone 639 To Zone 463 - User Class 1


From Zone 639 To Zone 463 - User Class 10


From Zone 463 To Zone 639 - User Class 1


From Zone 463 To Zone 639 - User Class 10


From Zone 409 To Zone 584 - User Class 1


From Zone 409 To Zone 584 - User Class 10


From Zone 584 To Zone 409 - User Class 1


From Zone 584 To Zone 409 - User Class 10


From Zone 586 To Zone 762 - User Class 1


From Zone 586 To Zone 762 - User Class 10


From Zone 762 To Zone 586 - User Class 1


From Zone 762 To Zone 586 - User Class 10


From Zone 432 To Zone 761 - User Class 1


From Zone 432 To Zone 761 - User Class 10


From Zone 761 To Zone 432 - User Class 1


From Zone 761 To Zone 432 - User Class 10


From Zone 416 To Zone 588 - User Class 1


## From Zone 416 To Zone 588 - User Class 10



From Zone 588 To Zone 416 - User Class 1


## From Zone 588 To Zone 416 - User Class 10



From Zone 593 To Zone 598 - User Class 1


From Zone 593 To Zone 598 - User Class 10


From Zone 598 To Zone 593 - User Class 1


From Zone 598 To Zone 593 - User Class 10


From Zone 773 To Zone 782 - User Class 1


From Zone 773 To Zone 782 - User Class 10


From Zone 782 To Zone 773 - User Class 1


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From Zone 782 To Zone 773 - User Class 10


From Zone 409 To Zone 589 - User Class 1


From Zone 409 To Zone 589 - User Class 10


From Zone 589 To Zone 409 - User Class 1


From Zone 589 To Zone 409 - User Class 10


From Zone 587 To Zone 779 - User Class 1


From Zone 587 To Zone 779 - User Class 10


From Zone 779 To Zone 587 - User Class 1


## From Zone 779 To Zone 587 - User Class 10



From Zone 766 To Zone 427 - User Class 1


From Zone 766 To Zone 427 - User Class 10


From Zone 427 To Zone 766 - User Class 1


From Zone 427 To Zone 766 - User Class 10


From Zone 819 To Zone 588 - User Class 1


From Zone 819 To Zone 588 - User Class 10


From Zone 588 To Zone 819 - User Class 1


From Zone 588 To Zone 819 - User Class 10


## Appendix E

## SITE SPECIFIC DEVELOPMENTS

| oistred | Ste cose | Deve | LPA | Loation | （ $\times$ | ：m | somece | Pooty | （4an）mor tiac |  |  | （eateme | ear |  |  | Smin | 202 | 2207 |  |  |  |  |  |  | Oid |  |  |  | oin | ${ }_{\text {cosin }}^{\text {2037 PM－}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Lown ocistasirul | Housing | Wasene T |  | s50774 | 2081 | ssa | Low3 |  | Na | NA | － | 207718 | Nacameman | ${ }^{2} 8$ |  | \％ | － | ， | 2017 |  | 1983 |  | ${ }^{1.81}$ | ${ }^{1.17}$ |  | ${ }^{168}$ |  | ${ }^{1.7}$ |  |
| Waeney | Lowa cochaz22FuL | Houstg | wasene 9 |  | 1 s6m7e | 20324 | ssa | Lowa |  | NA | NA | $\bigcirc$ | ${ }^{201647}$ | Necomimin | ${ }_{562}^{56}$ |  | $\stackrel{\square}{2}$ | $\bigcirc$ |  | ${ }^{2016}$ | 168 ${ }^{168}$ 230 | ${ }^{217}{ }^{217}{ }^{216}$ |  | ${ }^{181}$ | ${ }^{13}$ |  | ${ }^{189}$ |  | ${ }^{129}$ |  |
| Weaner Leater | ${ }_{\text {Lown }}^{\text {Lown }}$ | Heosing | Weanem |  | ${ }_{\text {creser }} 5$ | ${ }^{2093686}$ | ssa | Lows | Hesing（etiomombesese） |  | ${ }_{\text {Na }}^{\text {Na }}$ | ${ }_{6} 9$ | ${ }_{202122}^{202122}$ | Natemem | ${ }_{\text {as }}^{51}$ |  | ${ }_{8}$ | ${ }_{8}^{89}$ |  | 2017 2019 |  |  | 1288 1587 | ${ }_{\substack{988 \\ 120}}$ | 7,19 880 8， |  | ${ }_{\text {1288 }}^{1028}$ |  | 1082 1325 |  |
| Waneer Lor | Lows | Housing | Wasene N |  | 1 s5000 | $2{ }^{2389} 9$ | ssa | Lows | Housing | NA | na | ${ }_{45}$ | $2{ }^{20223}$ | Naramem | ${ }^{49}$ |  | ${ }_{45}$ | ${ }_{45}$ |  | 2019 | ${ }_{8,17}^{17}$ | ${ }^{1087} 1088$ |  | 907 | ${ }_{650} 8$ |  | ${ }^{14}$ |  | ${ }^{9} 9$ |  |
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| Wamene 0 | ос142324am | Housing y | Waenem P |  | s5xas | 223009 | pp | Whatat | Hosasg | NA | NA | 10 | 20920 | Nercoman | 420 |  | 10 | 10 |  | 2016 | ${ }_{182} \quad 258$ | 248240 |  | 202 | 1.4 | ${ }^{2,13}$ | 2.10 | 209 | 221 |  |
| Waneay 0 | Octisaliful | Housing | Waenem | Tmade Peses，wolusan Reast owesotr | Sstas？ | ${ }^{22928}$ | ${ }^{\text {pp }}$ | WMatala | Howes | NA | NA | 15 | ${ }^{201687}$ | Nercmam | ${ }^{578}$ |  | ${ }^{15}$ | ${ }^{15}$ |  |  | ${ }_{272}{ }^{383}$ | 362 |  |  | ${ }^{220}$ |  | ${ }^{3} 15$ |  |  |  |
| Wameny 0 | осо⿱宀八刀s86amm | Houstg | Wasene P | Phase Pearkeadus Oillon | $1{ }^{52935}$ | 20829 | PP | Lerictasat | Hesame | NA | NA | ${ }^{19}$ | ${ }^{201687}$ | Narcoman | ${ }^{566}$ |  | ${ }^{19}$ | ${ }^{19}$ |  | 2015 | ${ }^{21850} 83038$ | ${ }^{2274} \begin{array}{ll}2861\end{array}$ |  |  | 17.46 |  | ${ }^{2488}$ | 2.48 | 2280 |  |
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# Appendix F 

## TEMPRO ALTERNATIVE PLANNING

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## ASSUMPTIONS

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WSP House
70 Chancery Lane

## "

## Suffolk County Council

## DEMAND MODEL VALIDATION REPORT

Suffolk County Transport Model


Suffolk County Council

# DEMAND MODEL VALIDATION REPORT 

Suffolk County Transport Model

TYPE OF DOCUMENT (VERSION) CONFIDENTIAL
PROJECT NO. 70016133
OUR REF. NO. SCTM - DEMAND MODEL VALIDATION REPORT
DATE: NOVEMBER 2017

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## 1 INTRODUCTION

### 1.1 MODEL OVERVIEW

1.1.1. Suffolk County Council (SCC) has commissioned WSP to fully upgrade the existing modelling tools available to SCC and develop an integrated county-wide multi-modal model known as the Suffolk County Transport Model (SCTM).
1.1.2. The SCTM comprises a full Transport Demand Model (TDM), with a separate Highway Assignment Model (HAM) and Public Transport Assignment Model (PTAM).
1.1.3. The aim of the SCTM is to provide a multi-purpose transport modelling tool for SCC to test a range of potential transport schemes and policies. These may include:

- Major highway scheme appraisal
- Major public transport scheme appraisal
- Inputs for transport business cases and funding applications
- Inputs for environmental appraisal
- Local plan / core strategy assessment
- Development impact assessment
1.1.4. The SCTM has been developed to the extent that it is able to serve as a high-level strategic assessment tool for all such applications; however, the level of detail required for any specific application should be reviewed prior to testing. It may be necessary to enhance a particular local area for a specific testing purpose.


### 1.2 ABOUT THIS REPORT

1.2.1. The purpose of this report is to document the development, assumptions and calibration of the SCTM TDM. It presents the results of the realism tests which have been undertaken to demonstrate that the model aligns well with Department for Transport (DfT) Transport Analysis Guidance (TAG) and is fit for the purposes outlined above.
1.2.2. This report is structured into the following chapters:

- Chapter 2: Model overview
- Chapter 3: Generalised cost calculations
- Chapter 4: Derivation of travel demand
- Chapter 5: Iteration and demand/supply convergence
- Chapter 6: Realism testing
- Chapter 7: Summary of model development


## 2 MODEL OVERVIEW

2.1.1. This chapter is an overview of the SCTM TDM. It mainly covers the model structure, the zoning system, the modes and submodes modelled, the time periods, demand segments, and the modelling of park-and-ride.

### 2.2 MODEL STRUCTURE

2.2.1. The SCTM TDM models behavioural responses relating to trip frequency, time period choice, main mode choice, trip distribution, and submode choice (i.e. the choice between park-and-ride and pure public transport). It also interacts with highway and public transport assignment models, allowing schemes such as bus priority measures that impact the highway and public transport assignments to be tested.
2.2.2. The hierarchy of the TDM is shown in Figure 1. The approach is a fairly conventional incremental model, with a nested hierarchy which is in line with DfT's TAG Unit M2 section 4.5 (Variable Demand Modelling, March 2017).
2.2.3. In the absence of local evidence, park-and-ride (submode) choice has been positioned below destination choice, as suggested in section 3.2.3 in TAG Unit M5.1 (Modelling of Parking and Park-and-ride, January 2014).
2.2.4. The two assignment models sit at the bottom of the hierarchy, with skim matrices extracted from the assignments and used within the demand model to calculate the generalised costs of travel and derive the corresponding demand responses.
2.2.5. The skims extracted from the SCTM HAM are listed below:

- Distance
- Time
2.2.6. The skims extracted from the SCTM PTAM are listed below:
- Distance
- In-vehicle time
- Origin wait time
- Transfer wait time
- Walk time
- Access time
- Egress time
- Number of transfers
- Fares


Figure 1 SCTM TDM Choice Hierarchy

### 2.3 STUDY AREA AND ZONING SYSTEM

2.3.1. The TDM zoning system is consistent with the HAM's and PTAM's and comprises 1200 zones. Suffolk County has been modelled in detail and has therefore been divided into relatively small zones; and larger zones have been used in adjacent counties within the East of England (Norfolk, Cambridgeshire, Essex, Hertfordshire, and Bedfordshire) and in the remainder of the UK.
2.3.2. 307 zones in the zone system do not represent any geographical area in the base year assignment models; they have been included to facilitate the forecasting process and help maintain the same zone structure in both the base year and forecasting models.
2.3.3. Rail station car parks and park-and-ride locations within lpswich have been modelled as separate zones to allow the modelling of park-and-ride; these account for 26 zones in total. This approach facilitates the process of combining costs from the highway and public transport assignment models and the process of splitting park-and-ride trips into individual car and public transport legs prior to assignment.
2.3.4. The zones are grouped to create 33 sectors. The sectors are used in the process of converting demand matrices from peak hours to peak periods, as explained in section 2.7.
2.3.5. The sectoring system focusing on Suffolk can be seen in Figure 2. These sectors are defined by a combination of district boundaries and the major towns within the county.


Figure 2 Internal Sectors Plan
2.3.6 Figure 3 shows the remainder of the sectoring system outside of Suffolk which was based on county and government office region boundaries.
2.3.7 More information on the zoning and sectoring systems can be found in the SCTM LMVR (November 2017).


Figure 3 External Sectors

### 2.4 SOFTWARE PLATFORM

2.4.1. The SCTM TDM and PTAM are run in PTV Visum 15.00-15 and the HAM is run in SATURN (version 11.3.12 U).
2.4.2. The HAM and PTAM interact under the control of the demand model using the COM interface in PTV Visum which provides a convenient method of applying Visual Basic scripts to customise the operation of the demand model.

### 2.5 MODE AND SUBMODE CHOICES

2.5.1. The SCTM TDM models the following main modes:

- Car
- Public Transport (PT)
2.5.2. In the absence of local evidence and as outlined in the Model Specification Report (February 2016), park-andride has been positioned as a submode of public transport as car has been assumed to act like an access mode for relatively long public transport legs.
2.5.3. The SCTM TDM models the following submodes as part of the incremental model:
- Park-and-ride
- PT modes only
2.5.4. The SCTM TDM does not model walk and cycle as separate main modes. This reflects the expected usage of the model and the need to be proportionate, given the extra resource that would be required to obtain and analyse appropriate data and the likely limited impact of any further functionality. To model the transfer between active and mechanised modes, trip frequency has been modelled, as suggested in section 4.6 .3 in DfT's TAG Unit M2 (Variable Demand Modelling, March 2017).


### 2.6 PARK-AND-RIDE

2.6.1. The SCTM TDM covers rail-based park-and-ride using rail station car parks and available parking facilities in the areas surrounding rail stations. As mentioned in section 2.3, these car parks are modelled as separate zones in the model. Table 1 shows the list of car parks that have been included along with their respective total capacities and parking charges.

Table 1 Modelled Car Parks and Characteristics

| Zone | Station Name | Road Name | Capacity <br> (spaces) | Charges for <br> Commuters <br> (in pence) | Charges for <br> Non-Commuters <br> (in pence) |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 739 | Ipswich | Ipswich Station | 444 | 731 | 610 |
| 771 | Lowestoft | Lowestoft Station | 112 | 324 | 340 |
| 787 | Stowmarket | Stowmarket Station | 382 | 430 | 320 |
| 788 | Sudbury | Sudbury Station | 140 | 106 | 0 |
| 789 | Newmarket | Newmarket Station | 12 | 0 | 0 |
| 790 | Bury St Edmunds | Bury St Edmunds Station | 23 | 350 | 350 |
| 791 | Thurston | Station Hill | 12 | 0 | 0 |
| 792 | Brandon | Brandon Station | 5 | 0 | 0 |
| 793 | Needham Market | Station Yard | 22 | 0 | 0 |
| 794 | Manningtree | Station Rd | 570 | 578 | 450 |
| 795 | Woodbridge | Woodbridge Station | 72 | 85 | 0 |
| 796 | Melton | Wilford Bridge Rd | 27 | 0 | 0 |
| 797 | Wickham Market | Wickham Market Station | 48 | 0 | 0 |
| 798 | Saxmundham | Station Approach | 18 | 0 | 0 |
| 799 | Darsham | Darsham Station | 22 | 0 | 0 |
| 800 | Halesworth | Station Rd | 15 | 0 | 0 |
| 801 | Beccles | Beccles Station | 10 | 0 | 0 |
| 802 | Oulton Broad South | Oulton Broad South Station | 6 | 0 | 0 |
| 803 | Oulton Broad North | Oulton Broad North Station | 4 | 0 | 0 |
| 804 | Somerleyton | Station Rd | 8 | 0 | 0 |
| 805 | Diss | Station Rd | 326 | 394 | 360 |
| 806 | Trimley | Trimley Station | 6 | 0 | 0 |
| 807 | Bures | Station Hill | 20 | 0 | 0 |

2.6.2. Total car park capacities have been collected from National Rail Enquiries website. Parking costs have also been derived from information on the same website: for non-commuters (employers' business and other trips) the parking charge has been set to the off-peak rate, whereas for commuters it was assumed to be the annual rate divided by 235 working days. More information on the need for capacities and parking charges in the model can be found in section 0 .
2.6.3. The SCTM TDM does not explicitly model residential, workplace, on-street, or informal parking.

### 2.7 TIME PERIODS

2.7.1. There are differences in the time periods which the SCTM models represent, as shown in Table 2.

Table 2 SCTM Model Time Periods

| Time Periods | AM Peak | Inter Peak | PM Peak | Off-Peak |
| :--- | :--- | :--- | :--- | :--- |
| TDM | $07: 00-10: 00$ | $10: 00-16: 00$ | $16: 00-19: 00$ | $19: 00-07: 00$ |
| HAM | $08: 00-09: 00$ | Average hour <br> $10: 00-16: 00$ | $17: 00-18: 00$ | n/a |
| PTAM | $08: 00-09: 00$ | $11: 00-12: 00$ | $17: 00-18: 00$ | n/a |

2.7.2. As outlined in section 2.5.6 in DfT's TAG Unit M2 (Variable Demand Modelling, March 2017), the demand model is expected to operate at a 24-hour level in Production-Attraction (PA) format, so it is necessary to represent costs in the off peak period. However, validated base assignment models of the off-peak period have not been built for SCTM, as available data is much more restricted in this time period, and they are unlikely to provide significant benefits for any scheme assessment. Instead, the off peak models have been estimated based on the validated interpeak models and a simple factor has been applied to the validated interpeak matrix by sector to estimate the off peak demand. The estimated off peak models are run to produce skim matrices which are then used to calculate off peak costs.
2.7.3. To convert TDM outputs to HAM time periods, the ATC counts have been summed over the TDM and HAM time periods and the ratio of the sums taken for each time period and each site. The ratios have then been averaged across directions for each site, then across sites for each sector. For the sectors with no traffic count data available, the average factors have been applied. The ATC locations are illustrated in Figure 4.


Figure 4 Commissioned Traffic Survey Locations
2.7.4. To convert TDM outputs to PTAM time periods, the onboard bus passenger counts have been summed over the TDM and PTAM time periods, and the ratio of the sums taken for each time period and each site in Ipswich. The ratios have then been averaged across sites. The Ipswich bus data has been used rather than Bury's or Lowestoft's as it has the largest flows and therefore the most robust. Bus counts have also been used in favour of rail given the vast majority of PT trips are by bus and bus is the only PT mode for which passenger flows are available - boarding and alighting data is available at rail stations but this informs trips ends, not flows.
2.7.5. HAM to TDM and PTAM to TDM conversion factors are shown in Table 3, along with IP to OP conversion factors. PT count data is only available from 0700 to 1900 , therefore IP to OP conversion factors for the PTAM were assumed to be the same as for the HAM.

Table 3 Peak Hour to Peak Period and IP to OP Conversion Factors for the HAM

| Sector Name | Sector <br> Number | HAM |  |  | PTAM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Number of Counts | AM Peak Hour to Peak Period | PM Peak <br> Hour to <br> Peak <br> Period | IP to OP period | AM Peak Hour to Peak Period | PM Peak Hour to Peak Period |
| Waveney | 722 | 21 | 2.57 | 2.62 | 0.36 | 2.74 | 2.72 |
| Suffolk Coastal North | 717 | 20 | 2.55 | 2.68 | 0.34 | 2.74 | 2.72 |
| St Edmundsbury South | 706 | 19 | 2.54 | 2.60 | 0.45 | 2.74 | 2.72 |
| Suffolk Coastal Central | 718 | 16 | 2.52 | 2.62 | 0.33 | 2.74 | 2.72 |
| Bury St Edmunds | 704 | 15 | 2.46 | 2.70 | 0.38 | 2.74 | 2.72 |
| Babergh West | 712 | 13 | 2.61 | 2.59 | 0.41 | 2.74 | 2.72 |
| Sudbury | 711 | 12 | 2.62 | 2.58 | 0.41 | 2.74 | 2.72 |
| Mid Suffolk Central | 709 | 12 | 2.41 | 2.55 | 0.43 | 2.74 | 2.72 |
| St Edmundsbury North | 705 | 12 | 2.64 | 2.55 | 0.45 | 2.74 | 2.72 |
| Babergh Central | 713 | 9 | 2.63 | 2.53 | 0.39 | 2.74 | 2.72 |
| Felixstowe/Trimley | 716 | 10 | 2.29 | 2.62 | 0.49 | 2.74 | 2.72 |
| Mid Suffolk North | 708 | 9 | 2.47 | 2.56 | 0.43 | 2.74 | 2.72 |
| Newmarket | 700 | 8 | 2.62 | 2.53 | 0.45 | 2.74 | 2.72 |
| Forest Heath South | 702 | 7 | 2.72 | 2.67 | 0.51 | 2.74 | 2.72 |
| Stowmarket | 707 | 7 | 2.49 | 2.60 | 0.43 | 2.74 | 2.72 |
| Mid Suffolk South | 710 | 6 | 2.38 | 2.62 | 0.37 | 2.74 | 2.72 |
| Haverhill | 703 | 6 | 2.73 | 2.75 | 0.50 | 2.74 | 2.72 |
| Beccles/Worlingham | 721 | 3 | 2.87 | 2.67 | 0.39 | 2.74 | 2.72 |
| Babergh East | 714 | 5 | 2.65 | 2.66 | 0.50 | 2.74 | 2.72 |
| Forest Heath North | 701 | 5 | 2.75 | 2.56 | 0.51 | 2.74 | 2.72 |
| Suffolk Coastal South | 719 | 5 | 2.66 | 2.63 | 0.42 | 2.74 | 2.72 |
| Essex County_N | 727 | 1 | 2.69 | 2.36 | 0.46 | 2.74 | 2.72 |
| Cambridgeshire County_SE | 725 | 1 | 2.71 | 2.86 | 0.42 | 2.74 | 2.72 |
| Average | n/a | n/a | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |
| Sector Name | Sector Number | HAM |  |  | PTAM |  |  |
|  |  | Number of Counts | AM Peak Hour to Peak Period | PM Peak <br> Hour to <br> Peak <br> Period | IP to OP period | AM Peak Hour to Peak Period | PM Peak Hour to Peak Period |
| Essex County_S | 728 | 0 | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |
| East of England_W | 729 | 0 | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |
| Norfolk West | 723 | 0 | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |
| Norfolk East | 724 | 0 | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |
| Cambridgeshire County_NW | 726 | 0 | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |
| Great Britain North | 731 | 0 | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |
| Greater London | 730 | 0 | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |
| Ipswich District | 715 | 0 | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |
| Great Britain South | 732 | 0 | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |
| Lowestoft | 720 | 0 | 2.59 | 2.61 | 0.43 | 2.74 | 2.72 |

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### 2.8 DEMAND SEGMENTATION AND USER CLASSES

2.8.1. Traffic demand is split into the following two car availability segments in order to generate the correct responses in the demand model:

- Car Available (CA)
- No Car Available (NCA)
2.8.2. Demand is also segmented into the following journey purposes:
- Home-based work (HBW)
- Home-based employer's business (HBEB)
- Home-based other (HBO)
- Non-home-based employer's business (NHBEB)
- Non-home-based other (NHBO)
2.8.3. Table 4 shows the combinations of trip purpose and car availability that make trips within the SCTM TDM.

Table $4 \quad$ SCTM TDM Demand Strata

| Stratum | Description | Constraint <br> Type | Demand <br> Form |
| :--- | :--- | :--- | :--- |
| 1 | HBEB_CA | Singly | PA |
| 2 | HBEB_NCA | Singly | PA |
| 3 | HBO_CA | Singly | PA |
| 4 | HBO_NCA | Singly | PA |
| 5 | HBW_CA | Doubly | PA |
| 6 | HBW_NCA | Doubly | PA |
| 7 | NHBEB_CA | Singly | OD |
| 8 | NHBEB_NCA | Singly | OD |
| 9 | NHBO_CA | Singly | OD |
| 10 | NHBO_NCA | Singly | OD |

2.8.4. All demand strata are singly (production-end) constrained, except HBW-CA and HBW-NCA, as suggested in DfT's TAG Unit M2 section 4.9.10 (Variable Demand Modelling, March 2017).
2.8.5. All home-based demand strata in the SCTM TDM are treated at tour or Production/Attraction (P/A) level and all non-home-based demand strata are treated at individual trip or Origin/Destination (O/D) level, as no overall tour assumptions can be incorporated without considerably more information and complexity. With tour modelling, demand responses are modelled on the basis of the total cost of travel for the outbound and return journeys combined, while with trip-based modelling the cost of travel for each individual trip is used separately.

### 2.9 REFERENCE DEMAND

2.9.1. All home-based and non-home-based reference demand matrices that are input into the TDM are derived from the validated assignment matrices using a series of conversion procedures.
2.9.2. The validated assignment matrices are first converted from HAM and PTAM time periods to TDM time periods using the conversion factors described in section 2.7.
2.9.3. All car reference demand matrices are then converted from PCUs to person units using the relevant base year occupancy rates for each demand strata and each time period. The occupancy factors used are outlined in Table 12. At this point, non-home-based matrices are ready to use in the TDM; however home-based matrices still need to be converted from O/D to P/A level.
2.9.4. To allow the conversion from $\mathrm{O} / \mathrm{D}$ to $\mathrm{P} / \mathrm{A}$, outbound and inbound home-based matrices are aggregated across time periods then averaged to derive total daily home-based demand by mode and demand stratum. Tour proportions are then used to split these daily demand matrices into time period combinations. The tour proportions used are taken from the DIADEM 5.0 (SATURN) user manual (Appendix C).
2.9.5. The default tour proportions in DIADEM 5.0 are available for car, bus and rail, and have been derived from National Travel Survey data. As the bus and rail matrices are combined into one main mode in the SCTM TDM, a weighted average of the bus and rail tour proportions has been taken, using bus and rail patronages derived from observed data ( $82 \%$ for bus and $18 \%$ for rail). The car and combined PT tour proportions have then been furnessed to validated trip ends for application in the SCTM TDM. The furnessed proportions are shown in Table 5, Table 6, and Table 7, for car and public transport (CA and NCA), respectively.

| Table 5 | Tour Proportions for Car |  |  |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Car | AM- <br> AM | AM-IP | AM-PM | AM- <br> OP | IP-IP | IP-PM | IP-OP | PM- <br> PM | PM- <br> OP | OP-OP |
| Employer's <br> Business | $0 \%$ | $15.85 \%$ | $49.83 \%$ | $2.32 \%$ | $6.28 \%$ | $10.17 \%$ | $0.56 \%$ | $6.08 \%$ | $1.91 \%$ | $4.20 \%$ |
| Other | $4.45 \%$ | $14.85 \%$ | $4.63 \%$ | $0.11 \%$ | $31.23 \%$ | $12.35 \%$ | $0.49 \%$ | $10.8 \%$ | $2.15 \%$ | $17.24 \%$ |
| Commuting | $2.06 \%$ | $13.38 \%$ | $52.16 \%$ | $3.70 \%$ | $5.00 \%$ | $10.85 \%$ | $2.97 \%$ | $1.17 \%$ | $0.73 \%$ | $1.70 \%$ |

Table 6 Tour Proportions for Public Transport (CA segments)

| PT | AM- <br> AM | AM-IP | AM-PM | AM- <br> OP | IP-IP | IP-PM | IP-OP | PM- <br> PM | PM- <br> OP | OP-OP |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Employer's <br> Business | $0 \%$ | $16.79 \%$ | $52.96 \%$ | $3.25 \%$ | $5.47 \%$ | $8.89 \%$ | $0.64 \%$ | $3.53 \%$ | $1.46 \%$ | $4.64 \%$ |
| Other | $3.82 \%$ | $19.30 \%$ | $10.82 \%$ | $0.13 \%$ | $25.28 \%$ | $14.02 \%$ | $0.77 \%$ | $6.96 \%$ | $2.01 \%$ | $16.09 \%$ |
| Commuting | $2.37 \%$ | $11.42 \%$ | $66.00 \%$ | $3.38 \%$ | $2.26 \%$ | $7.19 \%$ | $1.44 \%$ | $0.66 \%$ | $0.30 \%$ | $0.95 \%$ |

Table $7 \quad$ Tour Proportions for Public Transport (NCA segments)

| PT | AM- <br> AM | AM-IP | AM-PM | AM- <br> OP | IP-IP | IP-PM | IP-OP | PM- <br> PM | PM- <br> OP | OP-OP |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Employer's <br> Business | $0 \%$ | $16.96 \%$ | $53.44 \%$ | $3.62 \%$ | $5.45 \%$ | $8.85 \%$ | $0.71 \%$ | $3.43 \%$ | $1.57 \%$ | $4.11 \%$ |
| Other | $4.77 \%$ | $18.53 \%$ | $10.63 \%$ | $0.13 \%$ | $25.05 \%$ | $14.22 \%$ | $0.79 \%$ | $6.95 \%$ | $2.02 \%$ | $16.06 \%$ |
| Commuting | $2.36 \%$ | $12.12 \%$ | $64.59 \%$ | $3.94 \%$ | $2.38 \%$ | $6.96 \%$ | $1.66 \%$ | $0.63 \%$ | $0.34 \%$ | $1.07 \%$ |

2.9.6. The furnessed tour proportions in the tables above are used to split the total daily home-based demand into P/A combinations.
2.9.7. The resulting split matrices are aggregated by outbound and inbound time periods and subtracted from the initial matrices on a cell by cell basis to calculate fitting-on factors. These factors help ensure that the O/D to P/A conversion process is reversible and also address the inaccuracies resulting from the use of national datasets and the omission of trips that occur in time period combinations that are not modelled in the SCTM TDM (IP-AM, PM-AM, PM-IP, OP-AM, OP-IP, and OP-PM).

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### 2.10 DIMENSIONS AND UNITS

2.10.1. The dimensions and units used in the SCTM TDM are outlined in Table 8

Table 8 Dimensions and Units of Input

| Skims | Units |
| :--- | :--- |
| Time skims | Minutes (min) |
| Distance skims | Kilometres (km) |
| All Public Transport time skims (in-vehicle <br> time, origin wait time, transfer wait time, walk <br> time, access time and egress time) | Minutes (min) |
| Public Transport fare skims |  |
| Public Transport number of interchange skims | Pence (p) |


| Input Parameters | Units |
| :--- | :--- |
| Value of Time | Pence/minute (p/min) |
| Weight applied to time spent walking | $\mathrm{n} / \mathrm{a}$ |
| Weight applied to time spent waiting | $\mathrm{n} / \mathrm{a}$ |
| Occupancy rates | People/car |
| Value of Operating Cost (VOC) | Pence/Kilometre (p/km) |

## 3 GENERALISED COSTS

3.1.1. This chapter outlines how the generalised costs have been calculated in the SCTM TDM and the equations and parameters used.

### 3.2 GENERALISED COST COMPONENTS

3.2.1. People's travel choices depend on both the monetary and time costs of the alternatives available to them. In the SCTM TDM, the generalised costs associated with each mode are all formulated as recommended in section 3 of DfT's TAG Unit M2 (Variable Demand Modelling, March 2017).
3.2.2. The generalised costs of travel in the TDM are expressed in units of time (in minutes) and include both the times and monetary costs associated with each trip.
3.2.3. The generalised costs have been derived using variables relating to the trips under consideration and others relating to the choice-making individuals. The variables relating to the trips (like ride time or trip distance) have been exported as skim matrices from the HAM and PTAM; and the variables relating to the choicemaking individuals (like their values of time) have been extracted from the DfT's TAG data book (July 2017, release v1.8).

### 3.3 CAR GENERALISED COST FORMULATION

3.3.1. The car generalised costs for a specific OD pair, time of day and demand stratum are calculated in the SCTM TDM as follows:

$$
G_{c a r}=t_{\text {ride }}+\frac{d * V O C}{o c c * V O T}
$$

Where:

- $G_{c a r}$ is the car generalised cost (in min);
- $t_{\text {ride }}$ is the journey time spent in the car (in min);
- $d$ is the journey distance travelled by car (in km );
- VOC is the vehicle operating cost per km for the trip purpose (in $\mathrm{p} / \mathrm{km}$ );
- occ is the number of people in the car (who are assumed to share the cost); and
- VOT is the value of time for the demand stratum (in $\mathrm{p} / \mathrm{min}$ ).
3.3.2. $\quad t_{\text {ride }}$ and $d$ are exported as skim matrices from the calibrated base year HAM for each time period and each assigned user class. More details on these can be found in the SCTM LMVR (November 2017).
3.3.3. The walk time and parking cost components of the equation in section 3.1.6 in DfT's TAG Unit M2 (Variable Demand Modelling , March 2017) are not used in SCTM due to the lack of information on residential, onstreet, and workplace car parks and on walking trips to and from car parks within the fully modelled (internal) area. Parking charges at the modelled park-and-ride sites have been included in the park-and-ride generalised cost formulation.


## Intra-zonal Distances and Times

3.3.4. Intra-zonal costs are required in order to represent trips within a zone and must be realistic to allow the choice between intra-zonal and inter-zonal trips in the distribution model. DfT's TAG M2 A.1.17 (Variable Demand Modelling, March 2017) states:

Care should be taken when dealing with intra-zonal trips. Because most assignment models do not output intra-zonal costs (since intra-zonal trips are not assigned) there may be problems with using incremental models where there are observed intra-zonal trips in the base year trip matrix. It is desirable that robust estimates of intra-zonal costs should be estimated in these instances. These could be some function of the inter-zonal costs, for example half the minimum inter-zonal costs for that zone (of course factors such as the nature of juxtaposition of other zones and the size of the
zone itself are considerations). Power function elasticity models will be particularly sensitive to very small intra-zonal costs, and this is one reason why they should be avoided when this is the case.
3.3.5. In line with this advice, intra-zonal distances and times have been derived by finding the minimum value for journeys to/from all other zones (i.e. the row and column minimum) and halving this. This calculation is updated at each iteration of the model.

### 3.4 PUBLIC TRANSPORT GENERALISED COST FORMULATION

3.4.1. The public transport generalised cost for a specific OD pair, time of day and demand stratum is the sum of the costs of the individual stages of the journey, such as accessing and waiting for public transport, riding on public transport, and egressing at the destination. It is calculated as follows:

$$
G_{P T}=t_{\text {walk }} * v_{\text {walktime }}+t_{\text {wait }} * v_{\text {waittime }}+t_{\text {ride }}+\frac{c_{\text {fare }}}{V O T}+c_{\text {interchange }}
$$

Where:

- $G_{P T}$ is the public transport generalised cost (in min);
- $t_{\text {walk }}$ is the total walking time to and from the PT service (in min );
- $t_{\text {wait }}$ is the total waiting time for all PT services used on the journey (in min);
- $v_{\text {walktime }}$ and $v_{\text {waittime }}$ are the weights applied to time spent walking and waiting, respectively, to reflect increases in perceived time when walking and waiting for public transport;
- $t_{\text {ride }}$ is the total in-vehicle journey time (in min);
- VOT is the value of time for the demand stratum (in $\mathrm{p} / \mathrm{min}$ );
- $c_{\text {fare }}$ is the fare (in pence); and
- $c_{\text {interchange }}$ is the interchange penalty if the journey involves transferring from one PT service to another (in min ). It is calculated as a time penalty multiplied by the number of transfers.
3.4.2. $t_{\text {walk }}$ consists of:
- Access time (from real origin to PT stop);
- Egress time (from PT stop to real destination); and
- Transfer time (between PT stops).
3.4.3. $t_{\text {wait }}$ consists of:
- Origin wait time (time spent waiting for the first service on path); and
- Transfer wait time (time spent waiting for subsequent services).
3.4.4. The number of transfers, access time, egress time, transfer time, origin wait time, transfer wait time, and invehicle time attributes are exported as skim matrices from the calibrated base year PTAMs for each time period and user class. Passenger fares are not included in the PTAM; the section below outlines how they have been derived.


## Intra-zonal Distances and Times

3.4.5. Given the size of the internal zones, it is assumed that no intra-zonal trips are made by public transport. The generalised costs on all intra-zonal OD pairs are set to a very large value, which prevents the TDM from allocating any trips to them.

## Derivation of Public Transport Fares

3.4.6. Bus and rail fare structures vary considerably from one operator to another, which makes them challenging to model. Following an analysis of the fares across the boroughs of Suffolk County, the following assumptions have been made:

- Bus fares are fixed fares applied per boarding
- Rail fares are distance-based
3.4.7. Table 9 outlines some sample bus fares in the study area, showing that there is no strong correlation between distance and fare.

Table 9 Sample Bus Fares in the Study Area

| From | To | Distance (km) | Day Ticket (£) |
| :--- | :--- | :--- | :--- |
| Ipswich | Stowmarket | 28 | $£ 1.65$ |
| Ipswich | Felixstowe | 24 | $£ 1.83$ |
| Ipswich | Aldeburgh | 45 | $£ 1.93$ |
| Ipswich | Melton | 16 | $£ 1.83$ |
| Southwold | Lowestoft | 22 | $£ 1.75$ |
| Ipswich | Ipswich | 4 | $£ 1.20$ |
| Ipswich | Ipswich | 9 | $£ 1.20$ |
| Mildenhall | Thetford | 40 | $£ 2.70$ |
| Bury St. Edmunds | Thetford | 24 | $£ 1.98$ |
| Bury St. Edmunds | Bury St. Edmunds | 15 | $£ 0.68$ |
| Sudbury | Colchester | 33 | $£ 3.50$ |
| Bury St. Edmunds | Stowmarket | 25 | $£ 2.90$ |

3.4.8. Bus fares are assumed to be fixed and the same across all bus lines and operators within the study area.

They have been set to $£ 2.00$ for commuters and $£ 3.00$ for others and have also been assumed not to vary by time period.
3.4.9. Rail fares per kilometre have been interpolated in PTV Visum based on the actual fares on selected flows with high passenger volumes. Distances on trips to and from London have been increased by 50 km to reflect the higher fares on the corresponding services. A minimum fare of $£ 3.00$ has been set for commuters and $£ 4.00$ for others
3.4.10. Table 10 shows some sample rail fares in the study area.

Table 10 Sample Rail Fares in the Study Area

| From | To | Distance <br> $(\mathrm{km})$ | Daily Equivalent <br> of Season Ticket <br> $(1-6$ months) | Any Time <br> (Single) | Off Peak <br> (Single) |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Ipswich | Colchester | 25 | $£ 4.39$ | $£ 7.60$ | $£ 7.60$ |
| Ipswich | Chelmsford | 60 | $£ 8.18$ | $£ 14.10$ | $£ 14.10$ |
| Ipswich | Stratford (London) | 101 | $£ 14.93$ | $£ 49.20$ | $£ 39.40$ |
| Ipswich | London Liverpool | 107 | $£ 15.81$ | $£ 49.20$ | $£ 39.80$ |
| Street |  |  |  |  |  |
| Ipswich | Norwich | 71 | $£ 8.25$ | $£ 15.20$ | $£ 15.20$ |
| Stowmarket | Norwich | 53 | $£ 6.96$ | $£ 13.00$ | $£ 13.00$ |
| Cambridge | Norwich | 104 | $£ 10.88$ | $£ 24.80$ | $£ 17.60$ |

3.4.11. In the SCTM, there is only one ticket type available for each demand segment. For employer's business and other purposes, anytime single ticket fares have been used for the AM peak, and off-peak fares for the other time periods. For commuting purposes, a daily equivalent fare has been derived from weekly / monthly / season tickets.

### 3.5 PARK-AND-RIDE GENERALISED COST FORMULATION

3.5.1. In the SCTM TDM, park-and-ride is modelled as a submode of public transport. Modelled park-and-ride trips involve people driving from their real origins to their respective train stations, parking their cars at the station car parks or nearby parking facilities, then using public transport services to reach their final destinations. In this model, people are assumed to use soft modes at the destination end of their trips i.e. from their last train stops to their real destinations (where they undertake their activity) - this part of their journeys is not modelled in the SCTM TDM. Walk movements between car parks and train stations have also been ignored for simplicity.
3.5.2. Additional procedure steps have been coded into the model in which parking sites are selected for each OD pair with park-and-ride demand. These steps make use of the in-built ' $P+R$ Lot Choice' procedure in PTV Visum as described below.

## PTV Visum 'P+R Lot Choice’ Procedure

3.5.3. The objective of the ' $P+R$ Lot Choice' procedure is to find the optimum park-and-ride sites for each OD pair and distribute the park-and-ride demand across them while accounting for the total park-and-ride capacity. Once a balanced distribution is reached, the procedure also determines the combined utility of travel using park-and-ride for each real origin to real destination pair.
3.5.4. For each zone where parking supply is represented, definitions of capacity and price are required. More information on modelled car park capacities and prices can be found in Table 1. Additional input parameters include generalised utilities of travel on the inbound (car) and outbound (public transport) legs, and a volumedelay function to account for capacity-dependent costs. Restrictions on parking durations and enforcement penalties are not covered.
3.5.5. As suggested in section 2.3.30 in DfT's TAG Unit M5.1 (Modelling Parking and Park-and-ride, January 2014), an iterative process is built into the procedure, during which the proportionate demand per demand stratum, OD pair and park-and-ride site is a variable that undergoes continuous adjustment throughout subsequent iterations.
3.5.6. The following section in the PTV Visum 15 manual provides more information on the iteration process:

At the beginning of each iteration, a shortest path search is performed for each OD pair and its total demand is distributed across the park-and-ride site found. This normally does not represent optimum distribution. The demand distributed across a single path is then scaled with the factor $\lambda$ and added to the formula for distribution of the previous iteration step as follows:

$$
R^{\prime}=(1-\lambda) R+\lambda \Gamma
$$

## Where:

$R^{\prime}$ is the new distribution calculated with this iteration step
$R$ is the distribution according to the previous iteration step
$\Gamma$ is the distribution of total demand according to shortest path search
$\lambda$ is a scaling factor between 0 and 1
The scaling factor $\lambda$ is first adjusted to reduce the gap. The gap expresses the ratio of a) the actual utilities, taking into account the distribution of demand across all parking lots in reach, and b) the utilities arising from distribution of the total demand across the shortest path found. Gap utility calculation is performed across all OD pairs and demand strata.
The procedure is finished when either the maximum number of iterations has been reached or the gap value defined is undershot.
3.5.7. The maximum number of iterations for the ' $P+R$ Lot Choice' procedure has been set to 100 and the maximum gap to 0.01.

### 3.6 GENERALISED COST PARAMETERS

3.6.1. The value of time (VOT) is used to represent the relative importance of cost versus time for different trip purposes and demand strata. In the SCTM TDM, separate values have been used for:

- Business travel (travel in work time)
- Journeys to work (commuting)
- Other personal travel (recreation, shopping, etc).
3.6.2. As recommended in DfT's TAG Unit M2 (Variable Demand Modelling, March 2017) section 3.3.6, the values of time used in the generalised cost calculations have been assumed to vary with distance as follows:

$$
\operatorname{VOT}_{d}=\operatorname{VOT} \cdot\left(\frac{d}{d_{0}}\right)^{n_{C}}
$$

Where:

- $d$ is the skimmed trip length (in kilometres)
- $d_{0}$ is the distance (in kilometres) underpinning the national average values of time. These distances have been taken from Table C1 in DfT's TAG Unit M2 (Variable Demand Modelling, March 2017)
- VOT is the average value of time listed in Table 11
- $n_{c}$ is the distance elasticity assumed to be 0.248 for commuting, 0.315 for other, 0.387 for employers' business and 0.435 for rail employers' business
3.6.3. The average values of time in the equation above are listed in Table 11 by TAG trip purpose. They have been derived from the July 2017 version of the DfT's TAG data book (release v1.8) using Table A1.3.2 and 2016 perceived cost values. They are expressed in pence / minute.

Table 11 Average Values of Time (pence / min) per TAG Trip Purpose

| TAG Trip Purpose | Value of Time <br> (p/min) |
| :--- | :--- |
| Work | 28.99 |
| Other | 8.14 |
| Commuting | 17.82 |

3.6.4. The vehicle occupancy rates per trip have been derived from Table A1.3.3 in the DfT's TAG data book (July 2017, release v1.8). They are listed in Table 12 by time period and TAG trip purpose.

Table 12 Occupancy Rates per TAG Trip Purpose

| TAG Trip Purpose | AM Peak | Interpeak | PM Peak | Off-peak |
| :--- | :--- | :--- | :--- | :--- |
| Work | 1.20 | 1.19 | 1.17 | 1.18 |
| Other | 1.68 | 1.65 | 1.71 | 1.66 |
| Commuting | 1.17 | 1.15 | 1.16 | 1.18 |

3.6.5. Vehicle operating costs (VOCs) have been implemented in the SCTM TDM to reconstruct the monetary cost of car journeys. They represent the monetary cost to users of running a specific type of vehicle over a certain distance. This cost consists of both fuel costs and non-fuel costs (e.g. oil, tyres, maintenance and depreciation). Table 13 summarises the VOC values (in pence / kilometre) used in the SCTM TDM by TAG trip purpose. They have been calculated using an average network speed of 57.3 kph in the AM peak, 60.7 kph in the IP and the OP, and 56.5 kph in the PM peak.

Table 13 Vehicle Operating Costs per TAG Trip Purpose

| TAG Trip Purpose | AM Peak VOC <br> $(\mathrm{p} / \mathrm{km})$ | Interpeak VOC <br> $(\mathrm{p} / \mathrm{km})$ | PM Peak VOC <br> $(\mathrm{p} / \mathrm{km})$ | Off-peak VOC <br> $(\mathrm{p} / \mathrm{km})$ |
| :--- | :--- | :--- | :--- | :--- |
| Work | 12.15 | 11.97 | 12.19 | 11.97 |
| Other | 5.79 | 5.74 | 5.80 | 5.74 |
| Commuting | 5.79 | 5.74 | 5.80 | 5.74 |

WSP
3.6.6. The correspondence between TAG and SCTM trip purposes is outlined in Table 14.

Table 14 Correspondence between SCTM Trip Purposes and DfT's TAG Trip Purposes

| SCTM Trip <br> Purpose | TAG Trip <br> Purpose |
| :--- | :--- |
| HBEB | Work |
| HBO | Other |
| HBW | Commuting |
| NHBEB | Work |
| NHBO | Other |

3.6.7. Weights are applied to the time spent walking and waiting for public transport to reflect increases in perceived time when walking to and from stops and waiting for services. They are taken as the midpoint values of the ranges in section 3.1.5 of DfT's TAG Unit M3.2 (Public Transport Assignment Modelling, January 2014) and are assumed to be 1.75 and 2 times in-vehicle time, respectively.
3.6.8. The public transport transfer time penalty is assumed to be 7.5 minutes of in-vehicle time per interchange, which is also the midpoint of the range recommended in section 3.1.5 of DfT's TAG Unit M3.2 (Public Transport Assignment Modelling, January 2014).
3.6.9. Table 15 summarises the TAG-recommended ranges as well as the values chosen in SCTM for the interchange penalty and the walk time and wait time weightings. These are consistent with the values used within the SCTM PTAM.

Table 15 TAG-Recommended Ranges and SCTM Values for the Interchange Penalty, Value of Walk Time, and Value of Wait Time

| GC Component | Minimum <br> Recommended | Maximum <br> Recommended | Chosen <br> Value |
| :--- | :--- | :--- | :--- |
| Value of walk time (factor of <br> in-vehicle time) | 1.5 | 2 | 1.75 |
| Value of wait time (factor of in- <br> vehicle time) | 1.5 | 2.5 | 2 |
| Interchange penalty (in <br> minutes of in-vehicle time per <br> interchange) | 5 | 10 | 7.5 |

### 3.7 COST DAMPING

3.7.1. In most models, using generalised costs directly as calculated above results in the model's elastic response to car fuel price changes being dominated by very long trips in a way that does not seem to accord with actual experience. According to Section 3.3 of DfT's TAG Unit M2 (Variable Demand Modelling, March 2017), there is also evidence that the impact of changes in generalised costs on demand responses reduces with increasing trip length. It is therefore common practice to apply some form of cost damping to long distance trips in order to reduce the elasticity of response and get satisfactory realism test results.
3.7.2. The SCTM TDM applies the damping function below to car and public transport generalised costs and to all purposes:

$$
G^{\prime}=A G^{\gamma}
$$

Where:

- $G^{\prime}$ is the damped generalised cost (in min);
- $G$ is the generalised cost calculated as described in sections 3.3, 3.4, 3.5 (in min)
- $A$ and $\gamma$ are positive cost damping coefficients determined by trial and error to meet the requirements of realism tests. They vary by mode and trip purpose, as shown in Table 16. These variations were essential to achieve acceptable realism test results.

Table 16 Cost Damping Parameters

| Demand Stratum | $A_{\text {Car }}$ | $A_{P T}$ | $\gamma_{\text {Car }}$ | $\gamma_{P T}$ |
| :--- | :---: | :---: | :---: | :---: |
| HBEB_CA | 4.79 | 2.08 | 0.65 | 0.85 |
| HBEB_NCA | 4.79 | 4.33 | 0.65 | 0.70 |
| HBO_CA | 3.82 | 2.15 | 0.70 | 0.85 |
| HBO_NCA | 3.82 | 4.62 | 0.70 | 0.70 |
| HBW_CA | 3.77 | 2.09 | 0.70 | 0.85 |
| HBW_NCA | 3.77 | 4.37 | 0.70 | 0.70 |
| NHBEB_CA | 4.79 | 3.39 | 0.65 | 0.75 |
| NHBEB_NCA | 4.79 | 5.53 | 0.65 | 0.65 |
| NHBO_CA | 4.78 | 2.15 | 0.65 | 0.85 |
| NHBO_NCA | 4.78 | 4.62 | 0.65 | 0.70 |

3.7.3. For public transport travel, the damping is set at $\gamma=0.70$ for most NCA strata and at $\gamma=0.85$ for the corresponding CA strata on the assumption that CA segments are more sensitive to changes in PT costs. For car travel, a milder damping is applied to most purposes ( $\gamma=0.65$ ), except HBO and HBW which use $\gamma=0.7$.

## 4 DERIVATION OF TRAVEL DEMAND

4.1.1. This chapter outlines how the travel demand has been derived in the SCTM TDM and the equations and parameters used in the modelling process.

### 4.2 MODEL STRUCTURE

4.2.1. As outlined in section 2.2, the SCTM TDM is an incremental model, where the following choice levels have been considered in the order outlined below:

- Trip frequency;
- Main mode choice;
- Macro time of day choice;
- Trip distribution; and
- Submode choice.
4.2.2. This hierarchy is shown in Figure 5.


Figure 5 SCTM TDM Hierarchy
4.2.3. Appendix E in DfT's TAG Unit M2 (Variable Demand Modelling, March 2017) outlines the equations to use in incremental models that include mode choice, time period choice and trip distribution choice; however it does not explicitly cover cases where submode choice is also modelled. The general methodology in section E. 1 has therefore been applied to understand the formulation to use in the SCTM TDM.
4.2.4. This section explains how the (composite) utility of travel, conditional probabilities, and updated trip matrices are derived for each level of the choice structure.
4.2.5. In the figures and equations in this section, the following notations are used for the indices:

- $p$ refers to trip purposes
- $c$ refers to person types
- $i$ refers to trip origins
- $j$ refers to trip destinations
- $m$ refers to trip modes: $C$ refers to car, and $P$ refers to public transport
- $t$ refers to trip time periods
- $s$ refers to trip submodes
4.2.6. To avoid confusion, composite utilities are labelled as $\Delta U^{*}$ and changes in utility are labelled as $\Delta U$.
4.2.7. In the SCTM TDM, the TAG-compliant composite utility and conditional probability calculations are derived using PTV's Nested Demand Model add-in. The formulae in the add-in have been revised so that a TAGcompliant choice formula with both lambda and theta parameters varying by mode could be used.


### 4.3 COMPOSITE UTILITIES

4.3.1. The base utility $\Delta U_{i j m t s p c}$ determines choices at the lowest level of the hierarchy (assignment). It is derived from the cost in the relevant assignment model of a journey between two zones $(i, j)$ in a specific time period $t$.
4.3.2. At higher stages in the hierarchy, costs across the more sensitive choices are combined for use in calculating the less sensitive choices. This is called "composite utility". The composite utilities at each of the higher levels of the choice model are derived by successively aggregating utilities from the lower levels. Table 17 shows how the composite utilities relate to the overall model hierarchy.
4.3.3. Sensitivity parameters are introduced at each stage of the hierarchy to reflect reducing sensitivity moving up the hierarchy. By convention, the sensitivity parameter at the level above assignment is denoted by $\lambda$. At higher levels, it is represented by $\theta$, a scaling parameter which reflects the sensitivity of a choice level relative to another. At the bottom level of the hierarchy, $\lambda$ is treated as a negative, to reflect the fact that cost is a disincentive to travel. At higher levels, this negative term is already incorporated into the formulation, so $\theta$ is positive. Both values are less than or equal to 1 , in absolute terms, with smaller values reflecting lower levels of sensitivity.

## Submode Choice

4.3.4. The submode choice model allows for the interaction between the highway and public transport networks, and is needed to accurately model specific park-and-ride services. It is only applied to car available (CA) person types.
4.3.5. The change in utility for the lowest level of the hierarchy is calculated using the equation below:

$$
\Delta U_{i j P t s p c}=-\lambda_{\text {submode }}\left(C_{i j P t s p c}-C_{i j P t s p c}^{0}\right)
$$

Where:
$\Delta U_{i j P t s p c}$ refers to the change in the utility of travel by public transport for zone origin $i$, zone destination $j$, time of day $t$, and submode $s$
$C_{i j P t s p c}^{0}$ is the corresponding reference generalised cost calculated from the base year calibrated and validated highway and public transport assignment models (in minutes)
$C_{i j P t s p c}$ is the corresponding forecast generalised cost (in minutes)
$\lambda_{\text {submode }}$ is the sensitivity parameter for the submode choice stage
4.3.6. For home-based purposes, the generalised costs in the equation above refer to the total costs on tours. This combination of outbound and inbound costs is calculated internally as part of the in-built calculations in PTV Visum. As outlined in section 2.8.5, this is done by using the cost of travel from the origin zone to the destination zone in the outbound time period and the cost of travel from the destination zone back to the origin zone in the inbound time period (the input utility matrices always refer to a single macro time period). While this methodology is reasonable for the calculation of car OR public transport (single-mode) return costs, it causes two main issues when calculating park-and-ride return costs:

- For a specific OD pair, the origin and destination zones may not be associated with the same park-and-ride site.
- For a specific OD pair, the first leg of the inbound and outbound journey is always made by car and the second leg is always made by public transport.
This is a limitation in the software which PTV is aware of and has registered as a task for software development.


## Trip Distribution

4.3.7. When calculating trip distribution, trips may be considered as either singly-constrained, where constraints apply only at the production end of the trip, or doubly-constrained, where constraints apply at both production and attraction ends. In the SCTM TDM, commuting and education trips are doubly-constrained, with other purposes being singly-constrained.
4.3.8. Trip distribution is at the bottom of the hierarchy under the car branch; the change in the utility of travel at this level is therefore calculated as follows:

$$
\Delta U_{i j c t p c}=-\lambda_{\text {dest }}\left(C_{i j C t p c}-C_{i j C t p c}^{0}\right)
$$

Where:
$\Delta U_{i j c t p c}$ refers to the change in the utility of travel by car for zone origin $i$, zone destination $j$, and time of day $t$ $C_{i j c t p c}^{0}$ is the corresponding reference generalised cost calculated from the base year calibrated and validated highway assignment models (in minutes)
$C_{i j c t p c}$ is the corresponding forecast generalised cost (in minutes)
$\lambda_{\text {dest }}$ is the sensitivity parameter for the trip distribution stage
4.3.9. Under the public transport nest, trip distribution is not the lowest level in the hierarchy; therefore the change in utility at this level is the composite change over the submode choice alternatives, as shown below:

$$
\Delta U_{i j P t p c}^{*}=\ln \sum_{s} p_{s \mid i j P t p c}^{0} \exp \left(\Delta U_{i j P t s p c}\right)
$$

Where $\Delta U_{i j P t s p c}$ is calculated as above and $p_{s \mid i j P t p c}^{0}$ is the reference case probability calculated from the input reference demand for public transport $T_{i j P t s p c}^{0}$ as follows:

$$
p_{s \mid i j P t p c}^{0}=\frac{T_{i j P t s p c}^{0}}{\sum_{s} T_{i j P t s p c}^{0}}=\frac{T_{i j P t s p c}^{0}}{T_{i j P t p c}^{0}}
$$

## Time Period Choice

4.3.10. The change in the composite utility at the time period choice level under the car nest is calculated using the equation below:

$$
\Delta U^{*}{ }_{i C t p c}=\ln \sum_{j} B_{j p} p_{j \mid i C t p c}^{0} \exp \left(\Delta U_{i j c t p c}\right)
$$

Where :
$B_{j p}$ is the balancing factor for doubly constrained purposes
$\Delta U_{i j C t p c}$ is calculated as above, and
$p_{(j \mid i C t p c)}^{0}$ is the reference case probability calculated from the input reference demand for car $T_{i j C t s p c}^{0}$ as follows:

$$
p_{j \mid i C t p c}^{0}=\frac{T_{i j c t p c}^{0}}{\sum_{j} T_{i j c t p c}^{0}}=\frac{T_{i j c t p c}^{0}}{T_{i c t p c}^{0}}
$$

4.3.11. The change in the composite utility at the time period choice level under the public transport nest is calculated using the equation below:

$$
\Delta U^{*}{ }_{i P t p c}=\ln \sum_{j} B_{j p} p_{j \mid i P t p c}^{0} \exp \left(\theta_{\text {dest }} \Delta U^{*}{ }_{i j P t p c}\right)
$$

Where:
$\theta_{\text {dest }}$ is the scaling parameter for the destination choice stage
$\Delta U^{*}{ }_{i j P t p c}$ is calculated as above, and
$p_{j \mid i P t p c}^{0}$ is the reference case probability calculated from the input reference demand for public transport $T_{i j P t s p c}^{0}$ as follows:

$$
p_{j \mid i P t p c}^{0}=\frac{\sum_{s} T_{i j P t s p c}^{0}}{\sum_{s j} T_{i j P t s p c}^{0}}=\frac{T_{i j P t p c}^{0}}{T_{i P t p c}^{0}}
$$

## Main Mode Choice

4.3.12. The main mode choice model is only applied to car available (CA) person types.
4.3.13. The change in the composite utility at the main mode choice level is calculated using the equation below:

$$
\Delta U^{*}{ }_{i m p c}=\ln \sum_{t} p_{t \mid i m p c}^{0} \exp \left(\theta_{t i m e} \Delta U^{*}{ }_{i m t p c}\right)
$$

Where:
$\theta_{\text {time }}$ is the scaling parameter for the time period choice stage
$\Delta U^{*}{ }_{i m t p c}$ is calculated for car and public transport as outlined above, and
$p_{t \mid i m p c}^{0}$ is the reference case probability calculated from the input reference demand $T_{i j m t s p c}^{0}$ as follows:

$$
\begin{aligned}
& \text { Car: } p_{t \mid i C p c}^{0}=\frac{\sum_{j} T_{i j c t p c}^{0}}{\sum_{j t} T_{i j c t p c}^{0}}=\frac{T_{i C t p c}^{0}}{T_{i C p c}^{0}} \\
& \text { PT: } p_{t \mid i P p c}^{0}=\frac{\sum_{s j} T_{i j P t s p c}^{0}}{\sum_{s j t} T_{i j P t s p c}^{0}}=\frac{T_{i P t p c}^{0}}{T_{i P p c}^{0}}
\end{aligned}
$$

## Trip Frequency

4.3.14. Trip frequency is included as a trip response since the slow modes are not explicitly modelled in the SCTM TDM, so trip frequency acts as a proxy for this choice. It has been applied to all modelled demand strata (purposes and car availability characteristics).
4.3.15. Trip frequency is invariably the least sensitive response in the hierarchy. The composite cost at this level is calculated using the equation below:

$$
\Delta U_{i p c}^{*}=\ln \sum_{m} p_{m \mid i p c}^{0} \exp \left(\theta_{m o d e} \Delta U^{*}{ }_{i m p c}\right)
$$

Where $\theta_{\text {mode }}$ is the scaling parameter for the main mode choice stage
$\Delta U^{*}{ }_{i m p c}$ is calculated as above, and
$p_{m \mid i p c}^{0}$ is the reference case probability calculated from the input reference demand $T_{i j m t s p c}^{0}$ as follows:

$$
\begin{aligned}
& \text { Car: } p_{C \mid i p c}^{0}=\frac{\sum_{j t} T_{i j \operatorname{tpc}}^{0}}{\sum_{j t m} T_{i j m t p c}^{0}}=\frac{T_{i C p c}^{0}}{T_{i p c}^{0}} \\
& \text { PT: } p_{P \mid i p c}^{0}=\frac{\sum_{s j t} T_{i j P t s p c}^{0}}{\sum_{s j t m} T_{i j m t s p c}^{0}}=\frac{T_{i P p c}^{0}}{T_{i p c}^{0}}
\end{aligned}
$$

## Summary of Utility Equations

4.3.16. Table 17 shows how the composite costs relate to the overall model hierarchy. At each of the higher levels of the choice model, they are derived by successively aggregating costs from the lower levels.

Table 17 Composite Utility Equations


### 4.4 CONDITIONAL PROBABILITIES

4.4.1. In order to update the trip matrix, conditional probabilities need to be calculated for each level using the composite utilities and reference case probabilities outlined above.

## Main Mode Choice

4.4.2. The conditional probability at the main mode choice level is calculated as follows:

$$
p_{m \mid i p c}=\frac{p_{m \mid i p c}^{0} \exp \left(\theta_{\text {mode }} \Delta U^{*}{ }_{i m p c}\right)}{\sum_{k} p_{k \mid i p c}^{0} \exp \left(\theta_{\text {mode }} \Delta U^{*}{ }_{i k p c}\right)}
$$

Where:
$\theta_{\text {mode }}$ is the scaling parameter for the main mode choice stage, and
$p_{m \mid i p c}^{0}$ is the reference case probability calculated from the input reference demand $T_{i j m t s p c}^{0}$ as follows:

$$
\begin{aligned}
& \text { Car: } p_{C \mid i p c}^{0}=\frac{\sum_{j t} T_{i j c t p c}^{0}}{\sum_{j t m} T_{i j c t p c}^{0}}=\frac{T_{i C p c}^{0}}{T_{i p c}^{0}} \\
& \text { PT: } p_{P \mid i p c}^{0}=\frac{\sum_{s j t} T_{i j P t s p c}^{0}}{\sum_{s j t m} T_{i j P t s p c}^{0}}=\frac{T_{i p p c}^{0}}{T_{i p c}^{0}}
\end{aligned}
$$

## Time Period Choice

4.4.3. The conditional probability at the time period choice level is calculated as follows:

$$
p_{t \mid i m p c}=\frac{p_{t \mid i m p c}^{0} \exp \left(\theta_{\text {time }} \Delta U^{*}{ }_{\text {imtpc }}\right)}{\sum_{k} p_{k \mid i m p c}^{0} \exp \left(\theta_{\text {time }} \Delta U^{*}{ }_{\text {imkpc }}\right)}
$$

Where:
$p_{t \mid i m p c}^{0}$ for car and public transport are $p_{t \mid i C p c}^{0}$ and $p_{t \mid i P p c}^{0}$ respectively, and
$\Delta U^{*}{ }_{\text {imtpc }}$ for car and public transport are $\Delta U^{*}{ }_{i C t p c}$ and $\Delta U^{*}{ }_{i P t p c}$, respectively.

## Trip Distribution

4.4.4. The conditional probabilities at the trip distribution choice level for the car and public transport modes are respectively calculated as follows:

$$
\begin{gathered}
p_{j \mid i C t p c}=\frac{B_{j p} p_{j \mid i C t p c}^{0} \exp \left(\Delta U_{i j c t p c}\right)}{\sum_{k} B_{k p} p_{k \mid i C t p c}^{0} \exp \left(\Delta U_{i k c t p c}\right)} \\
p_{j \mid i P t p c}=\frac{B_{j p} p_{j \mid i P t p c}^{0} \exp \left(\theta_{\text {dest }} \Delta U^{*}{ }_{i j P t p c}\right)}{\sum_{k} B_{k p} p_{k \mid i P t p c}^{0} \exp \left(\theta_{\text {dest }} \Delta U^{*}{ }_{i k P t p c}\right)}
\end{gathered}
$$

## Submode Choice

4.4.5. The conditional probability at the submode choice level is calculated as follows:

$$
p_{s \mid i j P t p c}=\frac{p_{s i j p t p c}^{0} \exp \left(\Delta U_{i j P t s p c}\right)}{\sum_{k} p_{s i j P t p c}^{0} \exp \left(\Delta U_{i j P t k p c}\right)}
$$

### 4.5 UPDATED TRIP MATRIX

4.5.1. Having derived the conditional probabilities, the updated trip matrix can then be calculated for car and public transport using the formulae below, respectively:

$$
\begin{gathered}
T_{i j C t p c}=T_{i p c}^{0} p_{C \mid i p c} p_{t \mid i C p c} p_{j \mid i C t p c} \\
T_{s i j P t p c}=T_{i p c}^{0} p_{P \mid i p c} p_{t \mid i P p c} p_{j \mid i P t p c} p_{s \mid i j P t p c}
\end{gathered}
$$

### 4.6 APPLICATION OF TRIP FREQUENCY

4.6.1. As suggested in section E. 8 of DfT's TAG Unit M2 (Variable Demand Modelling, March 2017), the trip frequency model is only applied after the above process has converged. It is applied at each demand/supply iteration using the equation below:

$$
\begin{aligned}
T_{i j \text { }}^{\prime} \text { tpc } & =\exp \left(\theta_{\text {freq }} \Delta U^{*}{ }_{i p c}\right) T_{i j c t p c} \\
T^{\prime}{ }_{\text {sijptpc }} & =\exp \left(\theta_{\text {freq }} \Delta U^{*}{ }_{i p c}\right) T_{\text {sijptpc }}
\end{aligned}
$$

4.6.2. This gives the final trip and tour matrices for each demand/supply iteration.

### 4.7 PARAMETERS

4.7.1. Sensitivity parameters are introduced at each stage of the hierarchy to reflect reducing sensitivity moving up the hierarchy. They have been calibrated to achieve acceptable realism test results.

## Mode Choice Scaling Parameters

4.7.2. As suggested in DfT's TAG Unit M2 section 5.6.17 (Variable Demand Modelling, March 2017), the mode choice and time of day sensitivity parameters ( $\lambda_{\text {mode }}$ and $\lambda_{\text {time }}$, respectively) have been assumed to be equal in all cases in the SCTM TDM. The mode choice scaling parameter $\theta_{\text {mode }}$, defined as the ratio of $\lambda_{\text {mode }}$ to $\lambda_{\text {time }}$, has thus been set to 1 .

## Time of Day Choice Scaling Parameters

4.7.3. Table 5.2 in DfT's TAG Unit M2 (Variable Demand Modelling, March 2017) lists illustrative values for the sensitivity of mode choice relative to destination choice ( $\lambda_{\text {mode }} / \lambda_{\text {dist }}$ ). However, the sensitivity parameter of mode choice ( $\lambda_{\text {mode }}$ ) has been assumed to be equal to that of time of day choice $\left(\lambda_{\text {time }}\right)$, see section above. Therefore, the values in DfT's TAG also represent the ratio of $\lambda_{\text {time }}$ to $\lambda_{\text {dist }}$ i.e. $\theta_{\text {time }}$.
4.7.4. The time of day scaling parameters used in SCTM for car and public transport are outlined in Table 18, along with the illustrative ranges listed in DfT's TAG, for comparison.
4.7.5. All $\theta_{\text {time }}$ values are less than or equal to one, which is consistent with the hierarchy adopted where destination choice follows time of day choice.

Table 18 Time of Day Choice Scaling Parameters

| Trip Purpose | SCTM |  | Tag Illustrative Values |  |  | Cost Units |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\theta_{\text {time, }, \text { ar }}$ | $\theta_{\text {time }, \text { PT }}$ | Minimum | Median | Maximum |  |
| HBW | 0.51 | 0.68 | 0.50 | 0.68 | 0.83 | Two-way |
| HBEB | 0.36 | 0.45 | 0.26 | 0.45 | 0.65 | Two-way |
| HBO | 0.40 | 0.53 | 0.27 | 0.53 | 1.00 | Two-way |
| NHBEB | 0.55 | 0.73 | 0.73 | 0.73 | 0.73 | One-way |
| NHBO | 0.65 | 0.81 | 0.62 | 0.81 | 1.00 | One-way |

## Trip Distribution and Submode choice Sensitivity Parameters

4.7.6. The trip distribution sensitivity parameters adopted in SCTM are outlined in Table 19 and Table 20 for car and public transport, respectively. These tables also show the illustrative ranges listed in DfT's TAG Unit M2 Table 5.1 (Variable Demand Modelling, March 2017), for comparison. As stated in paragraph 5.6.6 in Dft's TAG Unit M2, the illustrative sensitivity parameters relate to trip-based models and one-way generalised costs; they have thus been halved for home-based purposes in Table 19 and Table 20 for comparison with the values in SCTM.
4.7.7. $\quad \lambda_{\text {dist,m }}$ tends to be numerically larger where there is more freedom to choose. Thus more optional travel, such as HBO trips, tends to be more elastic and has a numerically larger lambda value than, say, travel to work.

Table 19 Destination Choice Sensitivity Parameters for Car ( $\lambda_{\text {dest,car }}$ )

| Trip Purpose | SCTM | Tag Illustrative Values |  |  | Cost Units |
| :--- | :--- | :--- | :--- | :--- | :--- |
|  |  | Minimum | Median | Maximum |  |
| HBW | 0.024 | 0.027 | 0.033 | 0.057 | Two-way |
| HBEB | 0.027 | 0.019 | 0.034 | 0.053 | Two-way |
| HBO | 0.034 | 0.037 | 0.045 | 0.080 | Two-way |
| NHBEB | 0.061 | 0.069 | 0.081 | 0.107 | One-way |
| NHBO | 0.058 | 0.073 | 0.077 | 0.105 | One-way |

Table 20 Destination Choice Scaling Parameters for Public Transport ( $\boldsymbol{\theta}_{\text {dest }, \text { PT }}$ ) and Submode Choice Sensitivity Parameters $\lambda_{\text {submode }}$

| Trip Purpose | SCTM |  | Tag Illustrative Values |  |  | Cost Units |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | $\theta_{\text {dest,PT }}$ | $\lambda_{\text {submode }}$ | Minimum | Median | Maximum |  |
| HBW | 1.000 | 0.021 | 0.012 | 0.017 | 0.022 | Two-way |
| HBEB | 0.800 | 0.018 | 0.015 | 0.018 | 0.022 | Two-way |
| HBO | 1.000 | 0.018 | 0.017 | 0.018 | 0.031 | Two-way |
| NHBEB | 0.750 | 0.038 | 0.038 | 0.042 | 0.045 | One-way |
| NHBO | 0.800 | 0.033 | 0.032 | 0.033 | 0.035 | One-way |

4.7.8. To improve calibration and realism test results, some lambdas and thetas for car have been refined, but nevertheless remain within $\pm 25 \%$ of the DfT's TAG medians. This is in line with TAG's advice in Unit M2 section 5.6.14 (Variable Demand Modelling, March 2017).

## 5 ITERATION AND DEMAND/SUPPLY CONVERGENCE

5.1.1. This chapter outlines the iteration process and the demand/supply convergence.

### 5.2 DEMAND/SUPPLY CONVERGENCE

5.2.1. It is critical that the entire model system converges to a satisfactory level to ensure that derived forecasts are free from model noise. In line with DfT's TAG M2 Section 6.3 (Variable Demand Modelling, March 2017), the recommended criterion for measuring convergence between demand and supply models is the demand/supply gap.
5.2.2. The demand/supply gap is calculated using the in-built 'Nested Demand Model Gap’ procedure in PTV Visum where the gap is defined by the following formula:

$$
\frac{\sum_{a} U_{a} \times\left(\left|D_{a}-P D_{a}\right|\right)}{\sum_{a} U_{a} \times P D_{a}} \times 100
$$

Where:
$P D_{a}$ is cell $a$ in the demand matrices of the previous iteration
$U_{a}$ is cell $a$ in the generalised costs resulting from assigning $P D_{a}$
$D_{a}$ is cell $a$ in the demand matrices of the current iteration
a represents every combination of origin, destination, demand segment, time period, mode, and submode.
5.2.3. According to DfT's TAG, 'tests indicate that gap values of less than $0.1 \%$ can be achieved in many cases, although in more problematic systems this may be nearer to $0.2 \%$ '. The iteration process for the SCTM TDM is set up so that when a demand/supply gap of $0.2 \%$ is achieved, the model stops iterating - there may be a need to review this as part of the forecasting process for specific applications.

### 5.3 ITERATION PROCESS

5.3.1. The iteration process is illustrated in Figure 6.


Figure 6 Iteration Process
5.3.2. Skims from the SCTM HAM and PTAM are imported and generalised costs calculated prior to every demand model run. The demand model procedure is run once before the iteration loop is initiated (iteration 0 ) and then once at every iteration within the iteration loop. For iteration 0, car and PT skims are taken from the calibrated base year HAM and PTAM.
5.3.3. All the parameters needed for each demand model run (lambdas, thetas, cost damping parameters) are kept the same at every iteration. The outputs from each demand model run are P/A demand matrices ( D matrices) by demand segment, mode, time period, and submode.
5.3.4. For each iteration $i$ within the loop:

- The demand matrices produced in iteration $i-1$ are converted into a form directly comparable to the assignment matrices. This means:
- Converting car person trips to car vehicle trips using the relevant base year occupancy rates for each demand stratum and each time period
- Converting from P/A to O/D (except for NHB trips that are modelled as one-way trips) by summing all outbound and return trips in each time period
- Converting from TDM time periods to HAM and PTAM time periods using the conversion factors described in section 2.7.
- The ratio of calculated and reference park-and-ride demand matrices is also derived for each time period and demand stratum. These factors are used to scale the reference demand for car legs of park-and-ride trips. This scaled demand is added to the converted car demand, resulting in $A_{i}$ matrices that are passed on to the HAM at each iteration $i$, one for each assignment time period and user class.
- The AM peak, Interpeak, PM peak, and Off-peak base year HAMs are then assigned with the corresponding assignment matrices ( $A_{i}$ matrices) derived from the demand model run in iteration $i-1$. Skims are then exported from these assignments and used for the demand model run in iteration $i$.
- All PT skim matrices remain the same as in iteration 0 . The PT models do not need to be assigned at each iteration as they do not account for crowding and would thus not produce skims that are different to the ones in iteration 0.
- Once the demand model procedure has been run for iteration $i$, the demand/supply gap is calculated and compared to the target gap of $0.2 \%$. Unless the calculated gap is less than the target or the maximum number of iterations (50) is reached, the loop is restarted.


## 6 REALISM TESTING

6.1.1. This chapter outlines the realism tests that have been carried out and the resulting elasticities.

### 6.2 REALISM TESTS

6.2.1. To validate the sensitivity of the SCTM base year TDM to changes in input values, a number of realism tests have been performed in line with DfT's TAG. This part of the process is ensuring the SCTM TDM validates to DfT's TAG criteria in its responses to changes in generalised costs via the elasticity (realism) tests.
6.2.2. The realism tests performed are the following:

- Test $1: 10 \%$ increase in car fuel cost
- Test 2: $10 \%$ increase in public transport fares
- Test 3: $10 \%$ increase in car journey time
6.2.3. All realism tests calculate an elasticity which is defined in DfT's TAG Unit M2 section 6.4 .5 (Variable Demand Modelling, March 2017) as a measure of the proportionate change in travel in response to a proportionate change in cost. It is calculated as follows:

$$
e=\frac{\log \left(T_{1}\right)-\log \left(T_{0}\right)}{\log \left(C_{1}\right)-\log \left(C_{0}\right)}
$$

Where:
$C_{0}$ and $C_{1}$ are the costs before and after the change, respectively
$T_{0}$ and $T_{1}$ are the travel demands before and after the change in costs, respectively. $T_{0}$ and $T_{1}$ are expressed in vehicle-kms for Test 1 , and person trips for Tests 2 and 3.
6.2.4. Table 21 summarises the elasticity ranges recommended by DfT's TAG for each of the realism tests.

Table 21 TAG-Recommended Elasticity Ranges

| Elasticity Test | TAG Recommended Range |  |
| :--- | :--- | :--- |
|  | High | Low |
| Car Fuel Cost (veh-kms) | -0.35 | -0.25 |
| Public Transport Fare (person trips) | -0.9 | -0.2 |
| Car Journey Time (car trips) | No stronger than -2.0 |  |

6.2.5. To account for congestion and crowding, the car fuel cost and PT fare elasticities have been calculated from converged runs of the demand/supply loop using the iteration procedure outlined in section 5.3. The car journey time test was not iterated because the TAG-recommended target values were derived from stated preference surveys.

### 6.3 CAR FUEL COST ELASTICITY

6.3.1. The car fuel cost elasticity is the percentage change in car vehicle-kms with respect to the percentage change in car fuel cost. It has been calculated for all purposes and time periods both on a matrix and network basis with a fully-converged run of the demand model. The change in car fuel cost has been applied to the car fuel cost component of the VOCs.
6.3.2. As per DfT's TAG Unit M2 section 6.4 (Variable Demand Modelling, March 2017), the combined average fuel cost elasticity should lie within the range of -0.25 to -0.35 across all journey purposes, with:

- Values for employers' business trips being nearer to -0.1
- Values for discretionary trips being nearer to -0.4
- Values for commuting being near the average
- Peak period values being generally lower than Inter-peak ones


## Matrix-based Method

6.3.3. In the matrix-based method, the change in car vehicle-kms is calculated from the skimmed distance matrices and the car trip matrices which relate to the before and after fuel cost change model runs.
6.3.4. The calculations have been carried out on an OD basis, by time period and demand stratum. They have then been aggregated over time periods and demand strata to produce elasticities by trip purpose and an overall average elasticity.
6.3.5. As external to external trips are treated as fixed in the SCTM TDM, matrix-based calculations only use internal-internal, internal-external, and external-internal movements.
6.3.6. It is important to note that elasticity figures quoted are for changes in the demand matrices output from the TDM and have been calculated from a converged run of the demand/supply loop where the demand model reached an acceptable gap 0.06\%.
6.3.7. Table 22 presents the car fuel cost realism results from an iterated run of the SCTM TDM. It shows the elasticity by trip purpose and the overall elasticity. Values by time period are shown in Table 23.

Table 22 Matrix-Based Car Fuel Cost Realism Results by Trip Purpose from Iterated Run

| Trip Purpose | Matrix-Based <br> Elasticity |
| :--- | :--- |
| HBEB | -0.10 |
| HBO | -0.43 |
| HBW | -0.24 |
| NHBEB | -0.12 |
| NHBO | -0.39 |
| Overall | $\mathbf{- 0 . 3 4 9}$ |

Table 23 Matrix-Based Car Fuel Cost Realism Results by Peak Period from Iterated Run

| Time Period | Matrix-Based <br> Elasticity |
| :--- | :--- |
| AM | -0.30 |
| IP | -0.39 |
| PM | -0.31 |
| OP | -0.42 |

6.3.8. The results show that for all purposes and time periods an acceptable level of elasticity is met. There is lower than average elasticities for HBEB and NHBEB, higher elasticities for HBO and NHBO, and lower elasticities in the peaks compared to the interpeak and the off-peak. This shows a good fit to DfT's TAG advice.

## Network-based Method

6.3.9. In the network-based method, the car-kms are accumulated over converged runs of the HAM network before and after the cost change, and compared. The HAM network used for the calculations only covers the internal area over which the HAM has been validated.
6.3.10. The calculations have been carried out by time period and the results are shown in Table 24.

Table 24 Network-Based Car Fuel Cost Realism Results by Time Period

| Time Period | Network-Based <br> Elasticity |
| :--- | :--- |
| AM | -0.21 |
| IP | -0.32 |
| PM | -0.17 |

6.3.11. Table 24 shows that the car fuel cost elasticities are higher in the interpeak period, which is to be expected.
6.3.12. The elasticities in the AM and PM peaks are slightly below the DfT's TAG-recommended range. This occurs because the change in car-kms includes the fixed external-external trips that are passing through the internal area, therefore underestimating the model's responsiveness.

### 6.4 PUBLIC TRANSPORT FARE ELASTICITY

6.4.1. The public transport fare elasticity is the percentage change in public transport trips by all public transport modes with respect to the percentage change in public transport fares.
6.4.2. DfT's TAG recommends the use of separate elasticities for each PT submode (bus and rail), where possible. Since PT submode choice is handled at the assignment stage, it is not possible to calculate PT submode fare elasticities for the SCTM.
6.4.3. The calculations have been carried out on a matrix basis, by time period and demand stratum. They have then been aggregated over time periods and demand strata to produce elasticities by trip purpose and an overall average elasticity.
6.4.4. As external to external trips are treated as fixed in the SCTM TDM, matrix-based calculations have only used internal-internal, internal-external, and external-internal movements.
6.4.5. It is important to note that the elasticity figures quoted are for changes in the demand matrices output from the TDM and have been calculated from a converged run of the demand/supply loop where the demand model reached an acceptable gap of $0.01 \%$.
6.4.6. Table 25 presents the public transport fare realism results by purpose from an iterated run of the SCTM TDM. Results by time period and car availability are shown in Table 26 and Table 27, respectively.

Table 25 Public Transport Fare Realism Results by Trip Purpose from Iterated Run

| Trip Purpose | Matrix-Based <br> Elasticity |
| :--- | :--- |
| HBEB | -0.12 |
| HBO | -0.39 |
| HBW | -0.26 |
| NHBEB | -0.17 |
| NHBO | -0.45 |
| Overall | $-\mathbf{0 . 3 2 9}$ |

Table 26 Public Transport Fare Realism Results by Time Period from Iterated Run

| Time Period | Matrix-Based <br> Elasticity |
| :--- | :--- |
| AM | -0.29 |
| IP | -0.37 |
| PM | -0.30 |
| OP | -0.37 |

Table 27 Public Transport Fare Realism Results by Car Availability from Iterated Run

| Car <br> Availability | Matrix-Based <br> Elasticity |
| :--- | :--- |
| CA | -0.32 |
| NCA | -0.35 |

6.4.7. As advised in sections 6.4.21 and 6.4.22 of DfT's TAG Unit M2 (Variable Demand Modelling, March 2017), the public transport fare elasticity should lie in the range of -0.2 to -0.9 for all journey purposes, with values for non-discretionary purposes being lower than those for discretionary trips and values in the peak periods being lower than values in the interpeak. The results show that for all purposes an acceptable level of realism is
met. There is lower than average elasticity for HBEB and higher elasticity for discretionary purposes, with NHBO trips having the greatest elasticity.

### 6.5 CAR JOURNEY TIME ELASTICITY

6.5.1. The car journey time elasticity is the percentage change in car trips with respect to the change in journey time. Unlike the fuel cost elasticity, it is calculated from a single run of the demand model using complete trips, from real origin to real destination.
6.5.2. Table 28 presents the car journey time realism results.

Table 28 Car Journey Time Realism (Single Run)

| Trip Purpose | Matrix-Based <br> Elasticity |
| :--- | :--- |
| Overall | -0.52 |

6.5.3. The results show that the overall car journey time elasticities is no stronger than -2.0 , which is in line with DfT's TAG.

## 7 SUMMARY OF MODEL DEVELOPMENT

7.1.1. The SCTM comprises a full Transport Demand Model (TDM), with separate Highway Assignment Model (HAM) and Public Transport Assignment Model (PTAM) that interact under the control of the demand model, allowing schemes that impact both the highway and public transport networks to be tested. The SCTM TDM and PTAM are run in PTV Visum 15.00-15 and the HAM is run in SATURN 11.3.12 U.
7.1.2. This Demand Model Validation Report documents the development, assumptions and calibration of the SCTM TDM. It also presents the results of the realism tests which have been undertaken to help demonstrate that the model aligns well with Department for Transport (DfT) Transport Analysis Guidance (TAG).
7.1.3. The SCTM TDM is a fairly conventional incremental model, with a nested hierarchy which is in line with DfT's TAG Unit M2 section 4.5 (Variable Demand Modelling, March 2017). It represents travellers' responses in the order outlined below:

- Trip frequency
- Main mode choice (car vs. public transport)
- Macro time of day choice (AM peak: 0700-1000; Interpeak: 1000-1600, PM peak: 1600-1900, Off-peak: 1900-0700)
- Trip distribution among destinations
- Submode choice (park-and-ride vs. pure public transport)
7.1.4. Travellers are segmented by trip purpose, person type, and car availability. The SCTM TDM represents personal trips including commuting to work and employer's business trips. However it does not deal with demand responses for Heavy Goods Vehicles (HGV) and Light Goods Vehicles (LGV) (e.g. supermarket delivery trips). The SCTM LMVR provides more detail on the representation of HGVs and LGVs.
7.1.5. The two assignment models sit at the bottom of the hierarchy, with skim matrices extracted from the assignments and used within the demand model to calculate the generalised costs of travel and derive the demand responses.
7.1.6. In the SCTM TDM, park-and-ride is modelled as a submode of public transport. Modelled park-and-ride trips involve people driving from their real origins to their respective train stations, parking their cars at the station car parks or nearby parking facilities, then using public transport services to reach their final destinations. In this model, people are assumed to use soft modes at the destination end of their trips i.e. from their last train stops to their real destinations (where they undertake their activity) - this part of their journeys is not modelled in the TDM. Walk movements between car parks and train stations are also ignored for simplicity.
7.1.7. To validate the sensitivity of the SCTM Base Year TDM to changes in input values, a number of realism tests have been performed in line with DfT's guidance. This part of the process is ensuring the SCTM TDM validates to TAG criteria in its responses to changes in generalised costs via the elasticity (realism) tests. Table 29 summarises the results of the realism tests carried out.

Table 29 TAG-Recommended Elasticity Ranges

| Elasticity Test | Overall Elasticity | TAG Recommended Range |  |
| :--- | :--- | :--- | :--- |
|  | (SCTM) | High | Low |
| Car Fuel Cost (veh-kms) | -0.349 | -0.35 | -0.25 |
| Public Transport Fare (person trips) | -0.329 | -0.9 | -0.2 |
| Car Journey Time (car trips) | -0.52 | No stronger than -2.0 |  |

7.1.8. $\quad$ The realism tests show that an acceptable level of elasticity is met using parameters that align with DfT's TAG Unit M2 (Variable Demand Modelling, March 2017). The appropriateness of the model responses (as demonstrated in this report) shows that the SCTM TDM can be used with confidence to test a range of potential transport schemes and policies, including major highway scheme appraisals.
7.1.9. The SCTM has been developed to an extent that it is able to serve as a high-level strategic assessment tool. However, no strategic model is capable of representing a whole county in fine detail, so the level of detail required for each application should be reviewed prior to testing as it may be necessary to enhance a particular local area for a specific testing purpose.

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70 Chancery Lane

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## Suffolk County Council

## SUFFOLK COUNTY TRANSPORT MODEL (SCTM)

Highway Model Local Model Validation Report


## Suffolk County Council

## SUFFOLK COUNTY TRANSPORT MODEL (SCTM)

Highway Model Local Model Validation Report

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Appendix J. 3 - Modelled flow PM

### 1.1 BACKGROUND

1.1.1. WSP has been commissioned by Suffolk County Council (SCC) to upgrade the existing transport modelling tools available to SCC and develop an integrated county-wide multi-modal model known as the Suffolk County Transport Model (SCTM). WSP is working in partnership with Kier under the Suffolk County Council Professional Services Framework.
1.1.2. $\quad$ The SCTM comprises a highway assignment model built in SATURN, as well as a public transport and demand model based in VISUM.
1.1.3. The SCTM highway assignment model incorporates the model networks used within the Ipswich Transport Model (ITM) and Lowestoft Traffic Model (LTM) which were previously used to inform the Outline Business Cases (OBCs) for The Upper Orwell Crossings (TUOC - formerly referred to as Wet Dock) and Lake Lothing Third Crossing.

### 1.2 MODEL PURPOSES

1.2.1. The SCTM will represent a substantial improvement to existing transport modelling tools within Suffolk and allow for a greater range of behavioural responses to be tested than at present. The SCTM will provide a robust evidence base for a range of possible applications.
1.2.2. The aim of the SCTM is that it will become a multi-purpose transport modelling tool for SCC to test a range of potential transport schemes and policies. These may include:

- Highway scheme appraisal
- Major public transport scheme appraisal
- Inputs for transport business cases and funding applications
- Inputs for environmental appraisals
- Local plan / core strategy assessment
- Development impact assessment.
1.2.3. $\quad$ The SCTM has been developed to an extent that it is able to serve as a high-level strategic assessment tool for all such applications. However, no strategic model is capable of representing a whole county in fine detail, so the level of detail required for each application should be reviewed prior to testing. It may be necessary to enhance a particular local area for a specific testing purpose


### 1.3 PURPOSE OF THE HIGHWAY LOCAL MODEL VALIDATION REPORT

1.3.1. The aim of the highway Local Model Validation Report (LMVR) is to demonstrate the SCTM highway model is fit for purpose and has been developed in accordance with WebTAG guidance.

### 1.4 EXISTING MODELS

1.4.1. $\quad$ There are two previously existing traffic models in Suffolk produced for SCC:

- Ipswich Traffic Model (ITM) - Created by AECOM. Validated to a 2008 base year, and recently partially updated to 2015 as part of the Upper Orwell Crossing Transport Business Case work. The model consists of a SATURN highway model and EMME public transport and demand model.
- Lowestoft Traffic Model (LTM) - Previously created by WSP. A highway assignment model using SATURN validated to a 2015 base with Variable Demand Modelling (VDM) carried out using DIADEM. This was recently updated as part of the Lake Lothing Third Crossing Transport Business Case, with demand matrices developed based on ANPR data and traffic survey data collected in 2015.
1.4.2. The networks and zone system for the ITM and LTM were incorporated into the SCTM and used to form the basis of the simulation network for Ipswich and Lowestoft. Road speeds were found to be considerably lower in the ITM compared to the WSP-produced LTM and were updated in order to be consistent with the rest of the SCTM. The network coding in terms of saturation flows and speed flow curves in the ITM and LTM were updated to be made consistent with the rest of the SCTM as per the conventions described in Section 5


### 1.5 PLANNED BASE YEAR MODELLING

1.5.1. Both Ipswich and Lowestoft Traffic Models have been updated substantially to ensure they are well aligned with the Department for Transport's (DfT) Transport Appraisal Guidance (TAG). Then they are then combined together and expanded to cover the entire county.
1.5.2. The SCTM has a base year of 2016 based on an average Monday to Thursday for neutral months.
1.5.3. The following three time periods have been modelled:

- AM peak hour (0800-0900)
- Inter peak average hour (1000-1600)
- PM peak hour (1700-1800)
1.5.4. Justification for the time periods modelled is based on extensive county-wide coverage of ATC, described in Section 3.6 and in the D2 SCTM Data Collection Report (December 2017).


### 1.6 REPORT STRUCTURE

1.6.1. This Local Model Validation Report (LMVR) sets out information relating to the development, calibration and validation of the updated highway assignment model. It is structured as follows:

- Section 2 - Proposed uses of the model and key design considerations
- Section 3 - Model standards
- Section 4 - Key features of the model
- Section 5 - Calibration and validation data
- Section 6 - Network development
- Section 7 - Trip matrix development
- Section 8 - Network calibration and validation
- Section 9 - Route choice calibration and validation
- Section 10 - Trip matrix calibration and validation
- Section 11 - Assignment calibration and validation
- Section 12 - Summary of model development, standards achieved and fitness for purpose
1.6.2. DfT TAG document "Guidance for the Technical Project Manager" (January 2014) details the required content within modelling reports. Section C.1.3 details what is required for an assignment model validation report. This guidance has been input into Table 1 as a checklist which provides details of which sections of the report contain the information required

Table 1 - DfT TAG Content Requirement Checklist for Assignment Model Validation Report

| Requirement | Section of <br> report |
| :--- | :--- |
| Description of the road traffic and public transport passenger assignment <br> model development, including model network and zone plans, details of <br> treatment of congestion on the road system and crowding on the public <br> transport system. | Section 3 |
| Description of the data used in model building and validation with a clear <br> distinction made for any independent validation data. | Section 4 |
| Evidence of the validity of the networks employed, including range checks, <br> link length checks, and route choice evidence. | Section 5 <br> Section 7 |
| Details of the segmentation used, including the rationale for that chosen. | Section 2 <br> Section 6 |
| Validation of the trip matrices, including estimation of measurement and <br> sample errors. | Section 6 |
| Details of any 'matrix estimation' techniques used and evidence of the effect <br> of the estimation process on the scale and pattern of the base travel matrices. | Section 8 |

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| Validation of the trip assignment, including comparisons of flows (on links and <br> across screen-lines/cordons) and, for road traffic models, turning movements <br> at key junctions. | Section 9 |
| :--- | :---: |
| Journey time validation, including, for road traffic models, checks on queue <br> pattern and magnitudes of delays/queues. | Section 9.3 <br> Appendix D |
| Detail of the assignment convergence. | Section 9.1 |
| Present year validation if the model is more than 5 years old. | N/A |
| A diagram of modelled traffic flows, both in the immediate corridor and other <br> relevant corridors. | Appendix J |

## 2 PROPOSED USES OF THE MODEL

### 2.1 INTERVENTIONS TO BE TESTED

2.1.1. It is intended the SCTM will be used to appraise a range of schemes including the following listed below. This is not an exhaustive list of schemes for which the SCTM will be used as more schemes may come forward for testing in the future.

- The Upper Orwell Crossing, Ipswich
- Lake Lothing Third Crossing, Lowestoft
- Suffolk Energy Gateway, Suffolk Coastal District
- Beccles Southern Relief Road
- Bury St. Edmunds Relief Road
- Sudbury Western Bypass
- Local Plan Assessments


### 2.2 SCENARIOS TO BE TESTED

2.2.1. Currently the SCTM highway model has various forecast models based on NTEM 7.2 growth in car traffic and National Road Traffic Forecast 2015 (NRTF15) growth from the National Transport Model (NTM) for LGV and HGV traffic.
2.2.2. The following forecast years have been developed for testing of the Lake Lothing Third Crossing to support the Development Consent Order process:

- 2022
- Lake Lothing Third Crossing scheme opening year
- Uncertainty log based on developments agreed with Waveney District Council
- NTEM 7.2 / NRTF15 growth for other Suffolk districts and external zones
- 2037
- Lake Lothing Third Crossing scheme opening year + 15 years
- Uncertainty log based on developments agreed with Waveney District Council
- NTEM 7.2 / NRTF15 growth for other Suffolk districts and external zones
2.2.3. The following forecast years have been developed for testing of the Suffolk Energy Gateway scheme to support the Outline Business Case:
- 2023
- Suffolk Energy Gateway scheme opening year
- Uncertainty log based on developments agreed with Waveney District Council and Suffolk Coastal District Council
- NTEM 7.2 / NRTF15 growth for other Suffolk districts and external zones
- 2037
- Suffolk Energy Gateway scheme opening year + 15 years
- Uncertainty log based on developments agreed with Waveney District Council and Suffolk Coastal District Council
- NTEM 7.2 / NRTF15 growth for other Suffolk districts and external zones
2.2.4. Further forecast year models will be developed going forward depending on the requirements of SCC.
2.2.5. It is envisaged the SCTM will be used for a range of different studies and local developer tests in the future which could require additional forecast years and growth assumptions to be considered


## 3 KEY FEATURES OF THE MODEL

## $3.1 \quad$ STUDY AREA

3.1.1. The study area is set out in terms of a three-tier hierarchy as specified in WebTAG Unit M3.1, Section 2.2, comprising:

- Area of Detailed Modelling - covers all roads with significant traffic volumes and all realistic route choices available to drivers, with all major junctions modelled
- Rest of Fully Modelled Area - reduced level of detail, with principal strategic routes modelled and capacity restraint through the use of speed/flow curves and strategically important junctions
- External Area - simplified network allowing traffic to enter the Fully Modelled Area at the correct location, without capacity restraint. Skeletal network with approximate distances to allow demand model to capture full trip length.
3.1.2. The Area of Detailed Modelling (ADM) and Rest of the Fully Modelled Area (RoFMA) form the Fully Modelled Area (FMA) in which all modelled links are included as part of the simulation network. The locations designated as part of the ADM are the main urban areas within Suffolk and the local study area specified for the Suffolk Energy Gateway scheme appraisal. The External Area comprises locations outside of Suffolk County and contains the buffer network.
3.1.3. The hierarchy of the model area is shown in Figure 1


Figure 1 - Modelled Area

### 3.2 ZONING SYSTEM

3.2.1. The zoning system for the Mobile Network Data which underpins the traffic demand within the SCTM is based on 2011 Census boundaries:

- Lower Super Output Area (LSOA) level across Suffolk and locations west / north-west of Suffolk
- Middle Supper Output Area (MSOA) within the remainder of the cordon used for the MND
- District level in other adjacent counties within the East of England (Norfolk, Cambridgeshire, Essex, Hertfordshire, Bedfordshire)
- Regional level in remainder of UK outside of the East of England
3.2.2. The MND zoning system comprises 755 zones and was provided to Telefonica for processing the Mobile Network Data which has been used as the basis of the matrices for the SCTM.
Figure 2 shows the zone system at LSOA, MSOA and district level for Suffolk and the surrounding area.


Figure 2 - Mobile Network Data Zone System
3.2.3. Figure 3 shows the full UK extent of the model zone system.


Figure 3 - Mobile Network Data Zone System - UK Extent
3.2.4. Additional detail was added to the MND zone system within Suffolk to create the assignment model zone system. Zones were split to better fit realistic zoning points and land uses, therefore the zone system is more detailed than LSOA level in the simulation network. Outside of Suffolk, the detail in the MND zone system was aggregated as MSOA / LSOA level of detail in external areas is currently not required in the assignment models. The detail for the MND zone system in these areas is such that it allows the opportunity to expand the detail of the simulation network beyond the edges of the Suffolk county boundary if required.
3.2.5. Following the disaggregation and aggregation of the MND zone system, the base year assignment model zone system comprises 857 polygon zones.
3.2.6. An additional 26 zones were created to represent rail station car parks and park \& ride locations within Ipswich.
3.2.7. Figure 4 provides a comparison of the assignment model zone system (blue boundaries) compared to the MND zone system.


Figure 4 - Assignment Model Zone System Compared to Mobile Network Data Zone System
3.2.8. Following disaggregation of the MND zone system and zones related to park \& ride, 883 zones are now present within the base year SCTM highway assignment model, with this zone system also utilised in the associated SCTM Public Transport Model and Demand Model.
3.2.9. Figure 5 details the breakdown of which zones within the assignment model are internal and external to the model.


Figure 5 - Assignment Model Zone System - Internal / External
3.2.10. Appendix A-1 contains plots of the detail of the zone system for each of the Areas of Detailed Modelling specified in Figure 1, namely the following locations:

- Beccles
- Bury St. Edmunds
- Felixstowe
- Haverhill
- Ipswich
- Lowestoft
- Newmarket
- Suffolk Energy Gateway
- Stowmarket
- Sudbury


### 3.3 SECTOR SYSTEM

3.3.1. The MND and assignment model zone systems have been grouped to create 32 sectors, within Suffolk these were defined by a combination of district boundaries and the major towns within the county. The sectoring system has been used as part of the verification of the MND, detailed in Appendix F-2.
3.3.2. The sectoring system focusing on Suffolk can be seen in Figure 6.


Figure 6 - Internal Sectors Plan
3.3.3. Figure 7 shows the remainder of the sectoring system outside of Suffolk which was based on county and government office region boundaries.


Figure 7 - External Sectors

### 3.4 NETWORK STRUCTURE

3.4.1. The area of detailed modelling covers the towns of Bury St Edmunds, Newmarket, Stowmarket, Ipswich, Felixstowe, Beccles, Lowestoft, Haverhill, and Sudbury. In this area, the extent of the network is sufficient to
cover all roads with significant traffic volumes and all realistic route choice available to drivers. All major junctions are modelled. In the rest of the Fully Modelled Area, detail is reduced, with all principal strategic routes modelled and capacity restraint characterised through the use of speed/flow relationships as well as strategically important junctions.
3.4.2. In the External Area, the network is simplified to the extent that traffic is able to enter the Fully Modelled Area at the correct locations constrained by speed flow curves but without delays associated within detailed junction design.
3.4.3. The network structure is shown in Figure 8


Figure 8 - Network Structure
3.4.4. Appendix A-2 contains plots of the detail of the network and junction type coding for each of the Areas of Detailed Modelling specified in Figure 1, namely the following locations:

- Beccles
- Bury St. Edmunds
- Felixstowe
- Haverhill
- Ipswich
- Lowestoft
- Newmarket
- Suffolk Energy Gateway
- Stowmarket
- Sudbury
3.4.5. Figure 9 details the extent of the simulation network which covers all of Suffolk including locations adjacent to Newmarket within Cambridgeshire due to geography of the county boundary. The buffer network is also shown and begins with the counties adjacent to Suffolk.


Figure 9- - Simulation and Buffer Network

### 3.5 CENTROID CONNECTORS

3.5.1. Centroid connectors connect the zoning system to the model network, allowing trips to load onto the network for assignment. It is critical that centroids connectors represent realistic loading points, particularly in the fully modelled area. Centroid connectors in the fully modelled area have been designed to represent actual loading points to specific residential and commercial areas, generally via a spur link to represent the actual access point. In this way, turns into and out of zones can be clearly understood.
3.5.2. The number of centroid connectors has been minimised, with most zones having a single centroid connector except in cases where a zone has clear multiple points of access, and sub-dividing the zone would not be realistic. In total, 807 zones load onto a single connector location. 75 zones are loaded onto the network using two centroid connector locations, 1 zone loads onto three centroid connector locations.
3.5.3. Centroid connectors have been designed so that they do not cross the network, further ensuring that loading is realistic. Connectors for different zones are loaded at different points in the majority of cases, to ensure trips between adjacent zones are loaded on to the network. Centroid connectors are also loaded away from count locations, to avoid inconsistencies between the counted flow and loaded trips.
3.5.4. In the fully modelled area, zones are sufficiently small such that average costs to access the model are sufficiently represented by the spur access links, so centroids themselves do not have costs associated with them.
3.5.5. In the external area, centroid connectors are linked to the network with appropriate parameters for distance and average speed to represent the average cost of accessing the network.

### 3.6 TIME PERIODS

3.6.1. The SCTM highway model comprises the following modelled time periods

- AM peak hour (08:00 - 09:00)
- Average interpeak hour (10:00-16:00)
- PM peak hour (17:00 - 18:00)
3.6.2. The appropriateness of these peak hours is detailed in the D2 SCTM Data Collection Report (December 2017). Analysis was conducted to identify the peak hours at each of the sites, and to confirm that the time periods set out in the D1 SCTM Model Specification Report (February 2016) have correctly been identified as the morning and evening peak hours. Analysis of peak hours is required in section 5 of WebTAG unit M3.1 (January 2014). This analysis considers the peak hours across Monday to Thursday.
3.6.3. Analysis of peak hours was carried out against the 225 ATCs specifically commissioned for the SCTM highway model. Table 2 shows which hour within the AM peak period (07:00-10:00) at each ATC site shows the highest average flow.
3.6.4. Table 2 shows the peak hour during the AM peak period is 08:00-09:00 for Cars and LGVs which make up the bulk of the road traffic modelled.

Table 2 - AM Peak Hour Analysis

| TIME PERIOD <br> STARTING | CAR | LGV | HGV |
| :--- | :--- | :--- | :--- |
| $07: 00$ | 28 | 67 | 16 |
| $08: 00$ | 189 | 141 | 83 |
| $09: 00$ | 8 | 17 | 126 |
| Total | 225 | 225 | 225 |

3.6.5. Table 3 compares the average traffic flow by hour in the inter peak period (10:00-16:00), counting which hour across the ATC sites shows the highest flow. The analysis shows there is a clear peak between 15:00 and 16:00 for both Cars and LGVs. For HGVs there is not a definitive peak hour across the inter peak period.

Table 3 - Inter Peak Hour Analysis

| TIME PERIOD <br> STARTING | CAR | LGV | HGV |
| :--- | :--- | :--- | :--- |
| $10: 00$ | 2 | 5 | 53 |
| $11: 00$ | 1 | 5 | 48 |
| $12: 00$ | 5 | 1 | 15 |
| $13: 00$ | 9 | 4 | 18 |
| $14: 00$ | 6 | 10 | 43 |
| $15: 00$ | 202 | 200 | 48 |
| Total | 225 | 225 | 225 |

3.6.6. Further analysis was carried out by determining the standard deviation based on the average flow for each inter peak hour. Upper and lower limits were calculated based on one and two standard deviations, with this value added and subtracted respectively from the overall inter peak period average hourly flow. It was found that across all 225 ATC sites, the average hourly flow between 10:00 and 15:00 (therefore excluding the 15:00 to 16:00) was within one standard deviations of the mean for the inter peak period in all cases in both directions. This analysis shows that despite the 15:00 to 16:00 hour clearly being the peak for Cars and LGVs,
it is not significantly greater than the other inter peak hours. Therefore it is appropriate to use an average inter peak hour for the SCTM highway model.
3.6.7. Table 4 shows which hour within the PM peak period (16:00-19:00) has the highest flow at each ATC site. This comparison shows for cars the peak hour is between 17:00 and 18:00. For LGVs and HGVs, 16:00 to 17:00 is the peak hour.

Table 4 - PM Peak Hour Analysis

| TIME PERIOD <br> STARTING | CAR | LGV | HGV |
| :--- | :--- | :--- | :--- |
| $16: 00$ | 23 | 143 | 218 |
| $17: 00$ | 200 | 81 | 7 |
| $18: 00$ | 2 | 1 | 0 |
| Total | 225 | 225 | 225 |

3.6.8. Figure 10 show the variation in the total car traffic across all 225 ATC sites by hour for an average weekday (Monday to Thursday). This further highlights the appropriateness of modelling 08:00-09:00 as the AM peak hour and 17:00-18:00 as the PM peak hour.


Figure 10 - Car Traffic Across All ATC Sites by Time of Day
3.6.9. It was determined that "pre-load" assignments representing shoulder ("pre") peaks prior to the time periods modelled were not required in the highway assignment model. Delay leftover from time periods prior to the modelled time periods can have an impact on congestion and routing in the main modelled time periods if it reaches a significant level. Using only the modelled time periods without a pre-load assumes there is no congestion/delay at the start of assignment. Demand for the shoulder peak time periods was determined by deriving factors from the 225 ATCs across Suffolk to convert from the peak hour assignments to the pre peak hours, shown in Table 5.

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Table 5 - Shoulder Peak ATC Conversion Factors

| Modelled Time <br> Period | Shoulder (Pre) Peak <br> Time Period | ATC factor |
| :---: | :---: | :---: |
| $08: 00-09: 00$ | $07: 00-08: 00$ | 0.826 |
| $17: 00-18: 00$ | $16: 00-17: 00$ | 0.931 |

3.6.10. The pre peak matrices where then assigned in the highway model with the queued flow at the end of the modelled time period determined. Appendix $B$ contains plots of queued flow at the end of the modelled time period at the following extents:

- County wide
- Ipswich
- Lowestoft
- Suffolk Energy Gateway
3.6.11. The plots in Appendix $B$ show the level of queued flow at the end of the shoulder peak assignments is not significant. There is no queued flow at the end of the time period in the area of detailed modelling for the Suffolk Energy Gateway scheme. Within Lowestoft, there is a small amount of queued traffic (18 pcus) left over on Normanston Drive during the PM pre-peak hour. In Ipswich there are various locations which highlight leftover queues, the highest value reaching 50 pcus, County wide there are other locations such as Sudbury and Bury St. Edmunds which have leftover queues of between $35-50$ pcus.


### 3.7 USER CLASSES

3.7.1. The following user classes are modelled within the SCTM:

- UC1: Car - Home Based Work (Inbound)
- UC2: Car - Home Based Work (Outbound)
- UC3: Car - Home Based Employers Business (Inbound)
- UC4: Car - Home Based Employers Business (Outbound)
- UC5: Car - Non Home Based Employers Business
- UC6: Car - Home Based Other (Inbound)
- UC7: Car - Home Based Other (Outbound)
- UC8: Car - Non Home Based Other
- UC9: LGV
- UC10:HGV
3.7.2. The car user classes have changed from those specified in the D1 MSR (February 2016) which specified three car user classes: Commuting, Employers Business and Other. This change was made during the matrix building process to aid the conversion of highway assignment matrices in Origin-Destination format into Production-Attraction matrices in the SCTM Demand Model. The SCTM Demand Model needs to be able to distinguish which part of a trip is home-based, inbound; meaning an individual is heading towards their place of residence, and outbound; an individual is leaving their home at the start of the trip. This directionality of trips is available in the Mobile Network Data (MND) which was used to build the matrices and therefore this information was utilised rather than the SCTM Demand Model having to infer directionality of home-based trips artificially from user classes which combine the inbound and outbound direction of home-based trips.


### 3.8 ASSIGNMENT METHODOLOGY

3.8.1. Model assignment of trips to the highway network was undertaken using a standard approach based on a 'Wardrop User Equilibrium', which seeks to minimise travel costs for all vehicles in the network. The Wardrop User Equilibrium is based on the following proposition:
3.8.2. "Traffic arranges itself on congested networks such that the cost of travel on all routes used between each origin-destination pair is equal to the minimum cost of travel and unused routes have equal or greater costs."
3.8.3. The Wardrop User Equilibrium as implemented in SATURN is based on the 'Frank-Wolfe Algorithm', which employs an iterative process. This process is based on successive 'All or Nothing' iterations, which are combined to minimise an 'Objective Function'. The travel costs are recalculated after each iteration and
compared to those from the previous iteration. The process is terminated once successive iteration costs have not changed significantly. This process enables multi-routeing between any origin-destination pair.

### 3.9 GENERALISED COST FORMULATIONS AND PARAMETER VALUES

3.9.1. Generalised cost is defined in keeping with the guidance in section 2.8 of TAG Unit M3.1 (January 2014), and is as follows:

$$
\text { Generalised cost }=\text { Time }+\left(\frac{\text { Vehicle operating cost }}{\text { Value of time }}\right) \text { Distance }
$$

3.9.2. Value of time is calculated in pence per minute (PPM) and vehicle operating cost is calculated in pence per kilometre (PPK). The adopted parameters were calculated from the TAG data book (July 2017). The value of time (PPM) for the HGVs was doubled from the value provided in the TAG data book. This is in line with TAG Unit A1.3 which advises for HGV that the driver's time does not take account of the influence of owners on the routing of these vehicles.
3.9.3. The parameters adopted for a 2016 base year are shown in Table 6. For the HGV class, local ATC data was used to determine the split of vehicles which could be classified as OGV1 and OGV2 by peak hour. This split was used to calculate average generalised cost parameters for HGVs. Average simulation network speeds were also used to derive the generalised cost parameters.

Table 6-2016 Generalised Cost Parameters

| User Class | AM |  | IP |  | PM |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | PPM | PPK | PPM | PPK | PPM | PPK |
| Car Home <br> Based Work | 20.18 | 5.79 | 20.51 | 5.74 | 20.25 | 5.80 |
| Car Employers <br> Business | 30.10 | 12.15 | 30.84 | 11.97 | 30.53 | 12.19 |
| Car Other | 13.92 | 5.79 | 14.83 | 5.74 | 14.58 | 5.80 |
| LGV | 21.27 | 13.31 | 21.27 | 13.35 | 21.27 | 13.30 |
| HGV | 43.19 | 45.72 | 43.19 | 45.88 | 43.19 | 49.15 |

### 3.10 RELATIONSHIP WITH DEMAND MODELS AND PUBLIC TRANSPORT ASSIGNMENT MODELS

3.10.1. The SCTM Public Transport assignment model utilises the same MND provided by Telefonica as the basis for the matrices. As discussed in Section 7 of this report, movements designated as "Road" in the MND are separated into Cars / LGVs and Bus movements, with the latter matrix then used in the public transport model.
3.10.2. The SCTM Demand Model will utilise time and distance skim matrices from the SCTM Highway Model, as well as skims from the public transport model in order to determine costs and the propensity for modal shift between different motorised modes. The SCTM Demand Model will be capable to taking into account trips which involve car usage at the start of a journey to then access the rail network and therefore create a composite cost for full park and ride trips, and therefore the potential for transport users to switch between modes taking into account congestion will occur on the highway network in the future

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## 4 CALIBRATION AND VALIDATION DATA

### 4.1 INTRODUCTION

4.1.1. This section of the report details the sources of the traffic data which was used for traffic flow and journey time calibration and validation. It also provides details of the screenlines which have been used to assess the ability of the SCTM highway model to match to observed data across several sites representing key strategic movements within the county.

### 4.2 COMMISSIONED SURVEY DATA - 2016 DATA COLLECTION

4.2.1. WSP commissioned Intelligent Data to carry out traffic surveys across Suffolk during April 2016. These surveys were specifically commissioned for the purposes of building, validating and calibrating the county model. The survey data provides a suitable coverage of data across the county outside of Lowestoft and Ipswich. Section 5 of the D2 Traffic Data Collection Report (December 2017) provides greater detail on this data and how it has been processed. Table 7 summarises the data which was collected across the county.

Table 7-Commissioned Survey Data

| Survey Type | Number of Surveys | Survey Period | Time |
| :--- | :--- | :--- | :--- |
| Automatic Traffic Counts (ATC) - Links | 227 | 7th to 25th April, 2016 | All day |
| Manual Classified Turning Counts (MCTC) | 34 | 12th April, 2016 | All day |
| Classified Turning Count using ANPR | 1 | 12th April, 2016 | $07: 00-$ <br> $19: 00$ |

4.2.2. Figure 11 shows the coverage of the traffic surveys specifically commissioned for the SCTM.


Figure 11 - Commissioned Traffic Survey Locations
4.2.3. Following consolidation of the commissioned survey data with existing data detailed in section 5.3, a total of 211 ATCs and 31 MCCs were used for validation and calibration of the SCTM highway model. This is due to duplication between data sources and issues affecting the data collection along the A14 around Ipswich between Junctions 57 and 58.
4.2.4. During the data collection period there was a road closure in Ipswich affecting southbound traffic on Nacton Road in Ipswich and subsequently the MCC at Junction 57. It is understood this road closure lead to re-routing of traffic on Felixstowe Road, therefore affecting the MCC carried out at Junction 58. For the Nacton Road Junction 57, the 2015 surveys carried out for the Upper Orwell Crossing Outline Business Case were therefore utilised for the calibration of the SCTM.
4.2.5. A new survey was carried out at the A14 Junction 58 on 13th October 2016 and has been used for calibration of the SCTM at this location. This new count demonstrated that the impact of the closure on the A14 was limited (apart from junction 57) as the impact on the J 58 was relatively small.
4.2.6. $\quad$ To support the modelling required for Lake Lothing Third Crossing, 6 Manual Classified Turning Counts were undertaken on the $14^{\text {th }}$ July 2016, the locations of which are described in Table 8

Table 8 - 2016 Lowestoft MCTCs

| Ref | Site Location |
| :--- | :--- |
| 1 | A1117 Bridge Road |
| 3 | Denmark Road / Peto Way - Barnards Way |
| 4 | Denmark Road / Rotterdam Road - Rotterdam Road |
| 5 | A12 Pier Terrace |
| 6 | A12 Horn Hill / A12 Tom Crips Way / Waveney Drive |
| 7 | B1531 Waveney Drive / Waveney Crescent - Waveney Drive |

### 4.3 COMMISSIONED SURVEY DATA - 2017 DATA COLLECTION

4.3.1. Additional traffic data was collected in 2017 in the following locations to support the following scheme appraisals:

- Ipswich - The Upper Orwell Crossing, Development Consent Order modelling
- East Suffolk - Suffolk Energy Gateway, Outline Business Case (OBC) modelling
- Lowestoft - Lake Lothing Third Crossing, Development Consent Order modelling


## IPSWICH

4.3.2. Table 9 describes the locations of the 2017 traffic survey locations undertaken to support the appraisal of the Upper Orwell Crossings. Processing of this data is discussed in Section 8 of the D2 Traffic Data Collection Report (December 2017).

Table 9 - Ipswich 2017 Traffic Surveys

| Survey Type | Total number of sites | Date undertaken |
| :---: | :---: | :---: |
| Automatic Traffic Counts <br> (ATCs) | 138 | Main survey period: $10^{\text {th }}$ May <br> 2017 to $25^{\text {th }}$ May 2017 |
| 1 day Manual Classified <br> Turning Counts (MCTCs) | 4 | $23^{\text {rd }}$ May 2017 |
| 7 day Manual Classified <br> Turning Counts (MCTCs) | 1 | $5^{\text {th }}$ to $12^{\text {th }}$ December 2016 |

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## EAST SUFFOLK, SUFFOLK ENERGY GATEWAY

4.3.3. Table 10 describes the locations of the 2017 traffic survey locations undertaken to support the appraisal of the Suffolk Energy Gateways scheme. Processing of this data is discussed in Section 7 of the D2 Traffic Data Collection Report (December 2017).

Table 10 - Suffolk Energy Gateway 2017 Traffic Surveys

| Survey Type | Total number of sites | Date undertaken |
| :---: | :---: | :---: |
| Automatic Traffic Counts <br> (ATCs) | 36 | Main survey period: $9^{\text {th }}$ May to <br> $22^{\text {nd }}$ May 2017 / June 2017 |
| Manual Classified Turning <br> Count | 3 | $18^{\text {th }}$ May 2017 0700-1900 |

## LOWESTOFT

4.3.4. An additional MCTC was undertaken at Waveney Drive / Riverside Road / Durban Road on $5^{\text {th }}$ July 2017. This is discussed in Section 6 of the D2 Traffic Data Collection Report (December 2017).

### 4.4 EXISTING SURVEY DATA

4.4.1. Existing ATC, MCC, ANPR and TRADS data was also obtained from other various sources as detailed in Table 11 below. Section 4 of the D2 Traffic Data Collection Report (December 2017) discusses in detail the sources and processing of the traffic data readily available from external sources which has not been specifically commissioned for the SCTM highway assignment model. In summary the additional sources of traffic survey data in Suffolk were as follows:

- 2015 traffic data collection to support Outline Business Case (OBC) submissions for the following schemes:
- The Upper Orwell Crossings
- Lake Lothing Third Crossing
- Suffolk County Council permanent ATC sites
- Highways England (HE) data from TRADS / WebTRIS
4.4.2. The ATC data provided by SCC was volumetric only, requiring the data to be split into the Car, LGV and HGV user classes used within the SCTM. Vehicle class factors were therefore derived from the ATC data commissioned for the SCTM and applied against the overall traffic volumes in the SCC ATC data,

Table 11 - External Survey Data

| Survey Type | Source | Number of <br> Surveys | Survey Period | Time |
| :--- | :--- | :--- | :--- | :--- |
| Automatic Traffic Counts <br> (ATC) - Links | Suffolk County Council | 96 | $2015 \& 2016$ | All day |
| Manual Classified Counts <br> (MCC) | 2015 data collection from <br> ITM \& LTM | 61 | July 2015 \& Oct 2016 | All day |
| Classified Turning Count <br> using ANPR | 2015 data collection for <br> LTM | 3 | July 2015 | 07:00- <br> $19: 00$ |
| TRADS | HATRIS / Data.gov.uk / <br> WebTRIS | 82 | April and May 2016, <br> March and April 2015 | All day |

Figure 12 shows the locations of the TRADS counts which have been used for model calibration.


Figure 12 -TRADS Counts
4.4.3. Figure 13 shows the locations of the SCC ATC counts which have been used for model calibration and validation


Figure 13 - SCC ATC Counts
4.4.4. Figure 14 shows the traffic surveys originally commissioned for the 2015 update of the LTM which were used for calibration and validation of the SCTM.


Figure 14 - Lowestoft Traffic Survey Data
4.4.5. Figure 15 details the location of the 2015 traffic surveys within Ipswich utilised for the Upper Orwell Crossing OBC. The majority of this data has been superseded by the 2017 traffic collection undertaken in Ipswich. However, 2015 data has been retained in some instances in order to provide suitable coverage of data and complete screenlines.


Figure 15 - Ipswich Traffic Survey Data
4.4.6. In combination with the commissioned surveys it is considered there is extensive coverage of traffic data which will provide a suitable basis for development of Suffolk County Transport Model (SCTM).

### 4.5 COUNT DATA CORRECTION TO 2016 BASE YEAR

4.5.1. Given the majority of the traffic data used within the SCTM highway assignment model was collected in 2016, this year is specified as the base year for the model.
4.5.2. As outlined above data from 2015 and 2017 has been used within the model. It has therefore been necessary to adjust this data in line with 2016 values. TEMPro factors at a district level have been used to factor the car trips, whilst NRTF factors have been used to adjust LGV and HGV observed values. For cars the conversion factors are time period specific, whilst for LGVs and HGVs the conversion factor for all peaks is the same. These factors are detailed in Section 3 of the D2 Traffic Data Collection Report (December 2017)

### 4.6 CALIBRATION AND VALIDATION COUNT DATA

4.6.1. Counts were split into calibration counts which were used for matrix estimation and validation counts which are independent of the matrix estimation process. The balance in terms of calibration and validation for the traffic counts is detailed in Table 12. It was ensured any counts used for matrix estimation or validation were designated in the same way in both directions.

Table 12 - Split of Counts in Calibration and Validation
Traffic count data type Number of link counts

| Calibration | 1224 |
| :---: | :---: |
| Validation | 254 |
| Total | $\mathbf{1 4 7 8}$ |

4.6.2. Figure 16 shows the coverage of traffic count data used for calibration and validation at a county wide level.


Figure 16 - County Wide Count Data in Calibration and Validation
4.6.3. Figure 17 shows the coverage of traffic count data used for calibration and validation within Ipswich.


Figure 17 - Ipswich Count Data in Calibration and Validation

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4.6.4. Figure 18 shows the coverage of traffic count data used for calibration and validation within Lowestoft.


Figure 18 - Lowestoft Count Data in Calibration and Validation
4.6.5. Figure 19 shows the coverage of traffic count data used for calibration and validation for the area of detailed modelling relevant to the Suffolk Energy Gateway appraisal.


Figure 19 - Suffolk Energy Gateway Count Data in Calibration and Validation

### 4.7 COUNTY WIDE CALIBRATION SCREENLINES

4.7.1. Calibration screenlines were defined at a county wide level linking series of counts corresponding to similar strategic movements within the county. The calibration screenlines were run through the matrix estimation process.
4.7.2. Figure 20 shows the calibration screenlines which have been used within Suffolk outside of the local study areas relevant to scheme appraisal. 19 screenlines have been specified at a county level.

Key Calibration Screenline
Figure 20 - Calibration Screenlines
4.7.3. Table 13 describes the calibration screenlines and the directional split used for each screenline for county wide model calibration outside of the local validation carried out for scheme appraisal.

Table 13 - Description of Calibration Screenlines

| ID | Description | Direction |
| :--- | :--- | :--- |
| 1 | Newmarket | Inbound/ Outbound |
| 2 | Haverhill | Inbound/ Outbound |
| 3 | Sudbury | Inbound/ Outbound |
| 4 | Felixstowe | Inbound/ Outbound |
| 5 | Stowmarket | Inbound/ Outbound |
| 6 | Bury St. Edmunds | Inbound/ Outbound |
| 7 | Beccles | Inbound/ Outbound |
| 8 | South of Bury St. Edmunds | Northbound / Southbound |
| 9 | Waveney | Northbound / Southbound |
| 10 | East of Bury St. Edmunds | Eastbound / Westbound |
| 11 | Mid Suffolk / Suffolk Coastal | Northbound / Southbound |
| 12 | Babergh / Mid Suffolk | Eastbound / Westbound |
| 13 | Forest Heath | Northbound/ Southbound |
| 14 | North of Bury St. Edmunds | Northbound/ Southbound |


| 15 | South Babergh | Northbound/ Southbound |
| :--- | :--- | :--- |
| 16 | Ipswich (Inner) | Inbound/ Outbound |
| 17 | Ipswich (Outer) | Inbound/ Outbound |
| 18 | North Lowestoft | Inbound / Outbound |
| 19 | South Lowestoft | Inbound / Outbound |

## LOCAL CALIBRATION AND VALIDATION SCREENLINES

4.7.4. Detailed calibration and validation screenlines were specified for the local validation of the following locations:

- Ipswich
- Lowestoft
- East Suffolk; Suffolk Energy Gateway
4.7.5. Figure 21 shows the location of the validation and calibration screenlines within Ipswich


Figure 21 - Ipswich Screenlines
4.7.6. Table 14 describes the validation and calibration screenlines specific to lpswich, including the Inner and Outer county wide screenlines.

Table 14 - Ipswich Screenlines

| ID | Description | Direction | Type |
| ---: | :--- | :--- | :--- |
| 1 | East / West Screenline 1 (West) | East / West | Validation |
| 2 | East / West Screenline 2 (North <br> West) | East / West | Calibration |
| 3 | East / West Screenline 3 (South <br> West) | East / West | Calibration |
| 4 | East / West Screenline 4 (East) | East / West | Calibration |
| 5 | East / West Screenline 5 (Far East) | East / West | Validation |
| 6 | East / West Screenline 6 (Central) | East / West | Validation |
| 7 | North / South Screenline 1 (South) | North / South | Validation |
| 8 | North / South Screenline 2 (North) | North / South | Calibration |
| 16 | Ipswich (Inner) | Inbound/ <br> Outbound | Calibration |
| 17 | Ipswich (Outer) | Inbound/ <br> Outbound | Calibration |

4.7.7. Figure 22 details the location of the local validation and calibrations screenlines within Lowestoft


Figure 22 - Lowestoft Screenlines
4.7.8. Table 15 describes the locations of the Lowestoft screenlines. The performance of the local Lowestoft screenlines is outlined in the local Lowestoft SCTM LMVR (November 2017).

Table 15 - Lowestoft Screenlines

| ID | Description | Direction | Type |
| :---: | :--- | :--- | :--- |
| 1 | Lowestoft Screenline 1 | North / South | Calibration |
| 2 | Lowestoft Screenline 2 | North / South | Calibration |
| 3 | Lowestoft Screenline 3 | East / West | Validation |
| 4 | Lowestoft Screenline 4 | North / South | Validation |
| 5 | Lowestoft Screenline 5 | North / South | Validation |
| 18 | North Lowestoft | Inbound / <br> Outbound <br> Inbound / <br> Outbound | Calibration |
| 19 | South Lowestoft | Calibration |  |

## IS|)

4.7.9. Figure 23 shows the location of the validation and calibration screenlines within the area of detailed modelling for the Suffolk Energy Gateway scheme.


Figure 23 - Suffolk Energy Gateway Screenlines
4.7.10. Table 16 describes the locations of the screenlines for the Suffolk Energy Gateway scheme. The performance of the screenlines in the area of detailed modelling for the Suffolk Energy Gateway scheme is discussed in the local SEGWay LMVR (December 2017).

Table 16 - Suffolk Energy Gateway screenlines

| Screenline ID | Type | Description |
| :---: | :---: | :---: |
| 1 | Calibration | Leiston |
| 2 | Calibration | North / West of A12 |
| 3 | Calibration | South / East of A12 |
| 4 | Calibration | Saxmundham |
| 5 | Calibration | Framlingham |
| 6 | Calibration | Woodbridge |
| 7 | Calibration | North / South screenline |
| 8 | Calibration | East / West screenline |
| 9 | Validation | NW / SW screenline |
| 10 | Validation | North / South screenline |

### 4.8 A14 CORRIDOR TRAFFIC COUNT ADJUSTMENTS

4.8.1. Due to the density of the count data available along the A14 corridor around Ipswich between Junction 52 at Claydon and Junction 58 (A12 / Felixstowe Road) a systematic check was carried out to ensure the main carriageway and slip road flows balanced along each junction in order to reduce any issues of inconsistent count data and the impacts this could have on matrix estimation. Where the data was available, TRADS traffic volumes were generally relied upon over MCTCs, however in instances where both TRADS data and MCTCs were available, the TRADS traffic volume was used but with the MCTC vehicle split.
4.8.2. Tables in Appendix $C$ provide details of the original survey data available along the specific sections of the A14, and the adjusted data used as calibration counts for matrix estimation. Instances where there was no change made to the original survey data have greyed out boxes in the 'Adjusted Survey Data' columns.

### 4.9 JOURNEY TIME SURVEYS FOR CALIBRATION AND VALIDATION

4.9.1. Trafficmaster GPS data was obtained from Suffolk County Council covering a period between September 2015 and August 2016. The data was filtered to only include data from the following neutral months:

- September 2015
- October 2015
- November 2015
- March 2016
- April 2016
- May 2016
4.9.2. The data was processed to provide an average weekday (Monday to Thursday) travel time by direction for each peak hour being modelled within the SCTM. Suffolk school holidays and bank holidays were excluded from the data used to derive the average travel times.
4.9.3. Travel time data was processed for a total of 78 routes across Suffolk in both directions. Following the guidance in WebTAG unit M1.2 it has been ensured the journey time routes were kept between 3km and 15 km .
4.9.4. Analysis of the number of observations available for the Trafficmaster GPS data was carried out and is detailed in the D2 SCTM Highway Data Collection Report (December 2017). This led to route 25 being excluded from the list of journey time routes due to an inadequate number of observations being available. Analysis was also carried out in terms of the average speed along links, this information was used to assist in the calibration and validation of the SCTM journey time performance.
4.9.5. The journey time routes used for model calibration and validation are shown in Figure 24


Figure 24 - Journey Time Routes
4.9.6. Appendix D-1 provides a description of the journey time routes used for calibration and validation.

## 5 NETWORK DEVELOPMENT

### 5.1 NETWORK DATA, CODING AND CHECKING

5.1.1. Network coverage was based on the hierarchy set out in section 3.1.
5.1.2. The network was verified through the use of ArcGIS and aerial photography. In particular, checks were carried out to verify:

- Node co-ordinates
- Link length check against measured GIS distance
- Speed/flow relationship
- Link type
- Link capacity
- One way/two way operation
- Number of (effective) lanes
- Length and position of flares
- Any observed turn delays/penalties
- Location of public transport routes
- Access points.
5.1.3. The network errors and warnings generated by SATURN were checked to ensure the model network is free of coding errors. A key check which was undertaken was to ensure road speeds and link lengths were consistent on both sides of the highway, these appear as "Warning 32" errors within SATURN and were systematically checked for across the model network.
5.1.4. Traffic loads onto the model network from zones in the form of centroid connectors. The centroid zone connectors in the SCTM were designed to realistically represent the way in which traffic joins the road network. In the Fully Modelled Area, where the zoning system is fine, specific access roads from residential and commercial areas was used as a basis for connecting zones to the network via centroid connectors.
5.1.5. Zones in the External Area, which have a large geographical coverage and significant demand associated with them, was generally be connected to major routes to enter the network.
5.1.6. Separate zones were specified for any interchanges between car and public transport, such as railway stations with significant parking or park and ride sites. This is to allow car and public transport legs of a journey to be separated and assigned using the correct assignment model.
5.1.7. Major car parks in the town centres were modelled as separate zones so that correct vehicular origins and destinations are modelled, but these were not capacity constrained.


### 5.2 JUNCTIONS

5.2.1. Each junction included in the area of detailed modelling network required several parameters as detailed below:

- Lane allocations
- Junction type
- Saturation flows at signal-controlled and priority-controlled junctions
- Signal times, stages and phases
- Saturation flows at roundabouts
- Gap times.


## LANE ALLOCATIONS

5.2.2. Lane allocations were checked using satellite and street level imagery to ensure the correct number of lanes and allowed turning movements (where lane markings and/or signage was apparent) were coded for the approaches at each junction in the model.

## JUNCTION TYPE

5.2.3. Checks were carried out for junction type using satellite imagery and street level data, with junctions split into the following types:

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- Priority controlled junctions
- Signal controlled junctions
- Roundabouts (no u-turns allowed)
- Roundabouts (u-turns allowed)
5.2.4. Only mini roundabouts were coded to not allow u-turns. For all other roundabouts coded as a single node, the junction type was coded as a roundabout which allows u-turns.
5.2.5. For roundabouts which had a mixture of priority controlled and signalised approaches, or where the junction was complex e.g. major 'A' road junction with slip roads, the junction was coded as an "exploded" roundabout. This means multiple nodes were used to code the junction in detail, with each approach separately modelled.


## SATURATION FLOWS AT SIGNAL CONTROLLED \& PRIORITY CONTROLLED JUNCTIONS

5.2.6. Default saturation flows were used for all junctions within the model. The default saturation flows per lane for priority junctions are:

- Major straight ahead movement (unopposed) - 1,980 pcu/hr
- Major left turn movement (unopposed) - $1,500 \mathrm{pcu} / \mathrm{hr}$
- Major right turn movement (opposed) - $745 \mathrm{pcu} / \mathrm{hr}$
- Minor left turn movement (opposed) - $700 \mathrm{pcu} / \mathrm{hr}$
- Minor right turn movement (opposed) - $600 \mathrm{pcu} / \mathrm{hr}$
- Minor straight ahead movement (opposed) - $600 \mathrm{pcu} / \mathrm{hr}$
5.2.7. Default saturation flows at signalised junctions are set to:
- Straight ahead movement - 1,980 pcu/hr
- Left or right turn movement - 1,740 pcu/hr
5.2.8. By default, SATURN assumes that opposing right turns at signalised junctions are "hooked" i.e. they interfere with each other. At larger junctions where there is sufficient space for traffic to turn right without being affected by the opposing right turn, it is possible to code these turns in the model so they do not interfere with each other. This was implemented at relevant junctions.


## SIGNAL TIMES, STAGES AND PHASES

5.2.9. Signal timings, staging and phases were extracted from controller specifications provided by Dynniq who manage the maintenance of numerous signalised junctions across Suffolk. The list of the signalised junctions for which controller IDs have been obtained are listed in Appendix E.
5.2.10. Rail level crossings were included in the model as signalised nodes, with a single stage. Generic timings were assumed at these locations, based on a total cycle time of 30 minutes ( 1800 seconds) as shown in Table 17

Table 17 - Level Crossing Signal Staging

| Peak Hour | Total Cycle Time <br> (Seconds) | Green Time <br> (Seconds) | Red Time (Seconds) |
| :--- | :--- | :--- | :--- |
| Level Crossing Signal <br> Staging | 1800 | 1600 | 200 |

5.2.11. Within Lowestoft, the impact of the tidal flow section and eastern Bascule Bridge was also coded within the model. The tidal flow section was handled by use of a SATURN " $\$$ Include" file which ensured two lanes eastbound / northbound in the AM peak; and two lanes westbound / southbound in the inter peak and PM peak on the A12 Belvedere Road.

## SATURATION FLOWS AT ROUNDABOUTS

5.2.12. Roundabouts require special consideration. Unlike with other junction types, each turn needs to be given the total saturation flow for the approach e.g. if a roundabout has a two-lane approach, with one lane to turn left and one to turn right, each turn should be coded with a saturation flow of 2,200 . Default saturation flows ( $\mathrm{pcu} / \mathrm{hr}$ ) adopted for roundabouts are given in Table 18. These values have been adopted to replicate typical ARCADY capacity estimates and have previously been utilised in a range of other models.

Table 18 - Roundabout Entry Capacity Saturation Flows

| Approach lanes | Number of entry lanes |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
|  | 1 | 2 | 3 | 4 |
| Single $(3.5 \mathrm{~m})$ | 1,130 | 1,670 | 2,030 |  |
| Single $(5.0 \mathrm{~m})$ | 1,510 | 1,940 | 2,250 | 2,450 |
| Dual 2 lane |  | 2,200 | 2,780 | 3,190 |
| Dual 3 lane |  |  | 3,330 | 3,940 |

5.2.13. For roundabouts coded as a single node, the overall circulatory saturation flow was set to be the same as the highest saturation flow on the approach arms of the roundabout.
5.2.14. For "exploded" roundabouts coded in detail with multiple nodes, the saturation flow for circulatory movements on the roundabout was assumed to be 1,600 pcus per hour per lane. The saturation flows for the give-way approaches were coded using values in Table 18. Large gyratory systems were also coded as a series of priority junctions for a better representation of journey times through the junction.

## GAP TIMES

5.2.15. Gap acceptance parameters in seconds applied to individual roundabouts are provided in Table 19.

Table 19-Roundabout Gap Acceptance Parameters (seconds)

| Approach lanes | Number of entry lanes |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
|  | 1 | 2 | 3 | 4 |
| Single $(3.5 \mathrm{~m})$ | 1.8 | 1.3 | 1.2 |  |
| Single $(5.0 \mathrm{~m})$ | 1.4 | 1.2 | 1.1 | 1.1 |
| Dual 2 lane |  | 1.1 | 1.0 | 0.9 |
| Dual 3 lane |  |  | 0.9 | 0.8 |

5.2.16. Global gap parameters were also defined as shown in Table 20 and were used in the absence of values being explicitly coded at junctions

Table 20 - Global Gap Acceptance Parameters

| SATURN Parameter | Junction Type | Gap Acceptance <br> (Seconds) |
| :--- | :--- | :--- |
| GAP | Priority / Signalised | 1.5 |
| GAPM | Merge | 1.0 |

5.2.17. During calibration, junction capacities and gap times were altered from the default values listed above where appropriate. This occurred in instances where the modelled flows were found to match well in comparison to the observed flows, however the level of delay present in the Trafficmaster GPS data was not being emulated.

### 5.3 LINKS

5.3.1. Each link included in the area of detailed modelling network required several parameters as detailed below:

- Distance
- Speed
- Speed flow curves
- Number of lanes
- Penalties/bans.


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## DISTANCE

5.3.2. Distances in both the simulation and buffer network take into account the actual alignment of modelled road. Distances were measured using GIS incorporated detailed mapping and satellite imagery. Distances were also applied to zone connectors in the buffer network to better represent the travel time into the simulation network from the external zones.

## SPEED

5.3.3. Within the urban area for links below 1 km the use of the model speed flow curves were deemed not to be necessary due to capacity restraints from the junctions at either end of the link.
5.3.4. These links were given fixed speeds based on their individual speed limit as obtained from imagery and GIS data. Fixed speeds were adjusted were generally adjusted below the speed limit to allow for the impact of relief, road curvature, reduced capacity due to parked vehicles or other obstructions and distance of link between junctions. Fixed speeds limits were kept consistent between all time periods modelled.
5.3.5. These speeds will reflect the free flow speed whilst the delay at junctions will reflect the conditions in busier periods. During the validation process link speeds were revised in certain instances in order to ensure travel time and modelled flow matched the observed journey times and traffic survey data.

## SPEED/FLOW CURVES

5.3.6. Highway capacity is restrained by junctions and by the speed-flow curves allocated to links in the study area. Speed flow curves are based on standard COBA 10 values and allocated to specific links based on assessment of the road speed, width and capacity. Speed-flow curves have generally only been used on rural or inter-urban links where the characteristics of the link itself, rather than junction capacity, have an impact on traffic speed. It has been necessary in some circumstances to use speed-flow curves in suburban areas to replicate the impacts of un-modelled minor junctions.
5.3.7. The speed flow curves that were used are shown in Table 21. Following a substantial increase in the network coverage in the highway model compared to what is outlined in the D1 SCTM Model Specification Report (February 2016) which was deemed to be required during model validation, an additional speed flow curve has been utilised for represent country lanes (ID 60) to represent additional routes of local significance. Speed flow curves were updated since the version of the SCTM reported in the D3 SCTM LMVR (February 2017) to better reflect COBA 10 speed-flow relationships s.

Table 21 - Model Speed Flow Curves

| Description | ID | Free flow <br> speed | Speed at <br> capacity | Capacity | Power <br> value |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Rural - D4M Motorway - 4 lanes | 1 | 116 | 45 | 9999 | 3.8 |
| Rural - D3M Motorway - 3 lanes | 2 | 116 | 45 | 7560 | 3.8 |
| Rural - D2M Motorway - 2 lanes | 3 | 112 | 45 | 4860 | 3.9 |
| Rural - D3AP All-purpose - 3 lanes | 4 | 109 | 45 | 6780 | 3.7 |
| Rural - D2AP All-purpose - 2 lanes | 5 | 105 | 45 | 4360 | 3.7 |
| Rural - S2 Good | 6 | 87 | 45 | 3280 | 2.2 |
| Rural - S1 Good | 7 | 87 | 45 | 1640 | 2.2 |
| Rural - S2 Average | 12 | 78 | 45 | 2760 | 2.1 |
| Rural - S1 Average | 13 | 78 | 45 | 1380 | 2.1 |
| Rural - S2 Poor | 14 | 67 | 45 | 2020 | 1.8 |
| Rural - S1 Poor | 15 | 67 | 45 | 1010 | 1.8 |


| Suburban - Dual(Slight development) | 16 | 78 | 35 | 3460 | 3.3 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Suburban - Single(Slight development) | 19 | 68 | 25 | 3460 | 3.7 |
| Suburban - Single(Slight development) | 20 | 68 | 25 | 1730 | 3.7 |
| Suburban - Dual(Typical development) | 21 | 61 | 25 | 2540 | 2.3 |
| Suburban - Dual(Typical development) | 22 | 61 | 25 | 1570 | 2.3 |
| Suburban - Dual(Heavy development) | 23 | 48 | 25 | 1000 | 1.6 |
| Suburban - Dual(Heavy development) | 24 | 48 | 25 | 500 | 1.6 |
| Urban - Non-central(Good) - 2 lanes | 25 | 54 | 25 | 1960 | 1.7 |
| Urban - Non-central(Good) - 1 lane | 26 | 54 | 25 | 980 | 1.7 |
| Urban - Non-central(Typical) - 2 lanes | 27 | 49 | 25 | 1560 | 1.6 |
| Urban - Non-central(Typical) - 1 lane | 28 | 49 | 25 | 780 | 1.6 |
| Urban - Non-central(Poor) - 2 lanes | 29 | 45 | 25 | 1300 | 1.5 |
| Urban - Non-central(Poor)-1 lane | 30 | 45 | 25 | 650 | 1.5 |
| Urban - Central(Good) - 2 lanes | 31 | 37 | 15 | 1480 | 1.8 |
| Urban - Central(Good) - 1 lane | 32 | 37 | 15 | 740 | 1.8 |
| Urban - Central(Typical) - 2 lanes | 33 | 34 | 15 | 1260 | 1.7 |
| Urban - Central(Typical) - 1 lane | 34 | 34 | 15 | 630 | 1.7 |
| Urban - Central(Poor) - 2 lanes | 35 | 29 | 15 | 900 | 1.6 |
| Urban - Central(Poor)-1 lane | 36 | 29 | 15 | 450 | 1.6 |
| Small Town - Light development - 2 lanes | 37 | 66 | 30 | 2600 | 3.0 |
| Small Town - Light development - 1 lane | 38 | 66 | 30 | 1300 | 3.0 |
| Small Town - Typical development - 2 lanes | 39 | 57 | 30 | 2000 | 3.4 |
| Small Town - Typical development - 1 lane | 40 | 57 | 30 | 1000 | 3.4 |
| Small Town - Heavy development - 2 lanes | 41 | 47 | 30 | 1760 | 2.5 |
| Small Town - Heavy development - 1 lane | 42 | 47 | 30 | 880 | 2.5 |
| Suburban - Single(Slight development) | 43 | 78 | 35 | 1730 | 3.7 |
| Centroid Connector - Internal | 50 | 87 | 87 | 9999 | 3.3 |
| Country Lane | 60 | 50 | 21 | 1200 | 2.15 |

## NUMBER OF LANES

5.3.8. Checks were made to ensure the correct number of lanes were allocated to links in the model, It was ensured the coding of the number of lanes for a link matched the speed flow curve for instances where these capacity restraints were applied.

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## PENALTIES / BANS

5.3.9. During auditing and building of the network, instances where there were restrictions in terms of the vehicle types allowed along links were taken into account. Height and weight restrictions on roads were taken into account by banning the HGV user class in the matrix from using these links.

## 6 TRIP MATRIX DEVELOPMENT

### 6.1 INTRODUCTION

6.1.1. The trip matrix has been built directly from a combination of mobile network data and 2011 Census journey to work data and then factored using a number of other data sources to generate the journey purposes and vehicle classes required for the modelling work.
6.1.2. The matrices require the use of both mobile phone network data and a synthetic matrix generated from 2011 Census journey to work data to form the initial matrix for the model. This uses a synthetic trip matrix for short distance trips in conjunction with longer-range mobile network data trips. This is primarily to overcome a shortfall in shorter distance trips that the mobile network data has inherent to the data collection as a result of the size of mobile network 'cells' / tower ranges.
6.1.3. The mobile network data has been processed from a dataset provided by Telefonica into the format that is required for the modelling work; the process splits out vehicle types required and purposes required from the original data. The synthetic matrix has been generated using the Census data for one journey purpose, with the other journey purposes built up from this matrix using factors from the National Travel Survey.
6.1.4. Details on the processing for both the mobile network data and synthetic data are provided later in this section of this report. In summary, the following process has been undertaken to reach the prior matrices for the highway and public transport models:

- Create matrices from the mobile network data:
- Aggregate the data from an LSOA level to an MSOA level;
- Split out bus trips from the 'road' data into a bus matrix;
- Convert remaining 'road' data from person trips to vehicle trips;
- Split out LGV trips from the vehicle-based 'road' data into an LGV matrix;
- Split out additional purposes not included in the mobile network data;
- Disaggregate from MSOA level to model zone level;
- Create Synthetic matrices from Census journey to work data;
- Scale each of the matrices to the base year for the model;
- Combine the mobile network data matrices and synthetic matrices based on a distance threshold; and
- Redistribute rail trips into different legs due to trips otherwise representing full origin-destination trips as a pure-rail mode, while trips actually represent access legs as well as the rail trip(s).
6.1.5. Matrix estimation has then been used to determine final matrices for the highway and public transport models. These matrices have been used in the demand model, although some additional work has been carried out to split out some of the particular trip elements required for that model and is set out within the demand model LMVR.


### 6.2 WORKFLOW

6.2.1. Figure 25 contains a simplified overview of the process carried out to generate the initial matrices. Each of the sections in this chapter then describes the elements of the process.

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Figure 25 - Initial Matrix Development Process Overview

### 6.3 MATRIX LEVELS

6.3.1. The matrix build process has used a number of matrix 'levels' to make up the matrix. Each level represents a trip purpose (some per direction) or vehicle type.

## HIGHWAY MATRIX LEVELS

6.3.2. There are 10 matrix levels in the highway matrix, where each level of the matrix is normally referred to as a user class for SATURN models.

1. [HBW IB]: Car home-based work (inbound);
2. [HBW OB]: Car home-based work (outbound);
3. [HBEmp IB]: Car home-based employers business (inbound);
4. [HBEmp OB]: Car home-based employers business (outbound);
5. [NHBEmp]: Car non-home-based employers business (both directions);
6. [HBO IB]: Car home-based other (inbound);
7. [HBO OB]: Car home-based other (outbound);
8. [NHBO]: Car non-home-based other (both directions);
9. Light Goods Vehicle (LGV) trips; and
10. Heavy Goods Vehicle (HGV) trips.

## PUBLIC TRANSPORT MATRIX LEVELS

6.3.3. There are 8 matrix levels in the public transport matrix, which represent public transport users (rail and bus modes of transport) for each of the journey purposes listed as matrix levels 1-8 in the highway matrix levels above. These matrices have then been subdivided into two further matrices each, where one matrix represents the 'Car Available' (CA) travellers, and one matrix represents the 'No Car Available' (NCA) travellers. This split has been carried out to assist the demand model mode assignment.
6.3.4. The 'Car Available' matrix has been further subdivided into two modes of travel; those who travel by car and those who do not. Those who do not travel by car are full origin-destination trips, as their access legs to the rail station is assumed to be using sustainable modes, while those who do use car have been assigned directly as rail station to destination (with the access leg moved into the highway matrix). This process is detailed later in this section.

### 6.4 DATA SOURCES

6.4.1. The matrix development process has been informed by a number of data sources. Each data source has been carefully considered to make the matrix development process robust as far as possible. The data sources used include the following:

- Mobile network data (MND) provided by Telefonica for April 2015;
- National Travel Survey (NTS) 2015 for the East of England;
- National Trip End Model (NTEM) 7.2;
- Census 2011 data including the following elements:
- Proportion of bus users compared to all road users from Journey to Work data;
- Journey to Work data for car / bus / rail users;
- Adults and Employed Persons numbers (Census Output Area basis)
- Workplace population (Workplace Zones basis); and
- Car ownership data.
- Values for vehicle occupancy from the WebTAG databook (July 2017 edition); and
- Land area per zone.
6.4.2. The data sources used have been detailed further throughout this section.


### 6.5 MOBILE NETWORK DATA MATRIX PROCESSING

6.5.1. A simplified overview of the Mobile Network Data processing is provided as Figure 26. Figure 27 is provided to expand on the LGV process, as this process is significantly more involved than is able to be shown in Figure 26.

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Figure 26 - Mobile Network Data Processing Overview


Figure 27 - LGV Split Process Overview
6.5.2. Telefonica were commissioned to provide mobile network data covering Suffolk and surrounding areas. The data collection period was set as April 13 to May 21 2015, for Mondays to Thursdays, excluding bank holidays and school holidays, resulting in 23 days of data. This complies with WebTAG unit M1.2 which defines the neutral periods of time for data collection throughout the year.
6.5.3. The mobile network data covers trips that start or end within the defined cordon area given to Telefonica. For the study area this covers all trips that might be expected to be made within the study area, as it is unlikely that there would be any trips made that would pass through the primary area of study without having an origin or destination within the area. MND was provided at an LSOA level within Suffolk. This data has been aggregated to MSOA level as the data provided by Telefonica indicated a lower level of accuracy at LSOA level.
6.5.4. The mobile network data has been provided for a full 24 hour period, split into the following time periods by Telefonica:

- Early Off Peak (00:00 to 07:00);
- AM Peak Period (07:00 to 10:00);
- AM Peak Hour (08:00 to 09:00);
- Inter Peak Period (10:00 to 16:00);
- PM Peak Period (16:00 to 19:00);
- PM Peak Hour (17:00 to 18:00); and
- Late Off Peak (19:00 to 00:00).
6.5.5. The AM Peak Hour (APH), Inter Peak Period (IP), and PM Peak Hour (PPH) have been used for the generation of the transport model matrices, with the IP factored by $1 / 6$ th to represent an average hour of this period for modelling purposes.
6.5.6. The mobile network data that was provided by Telefonica was provided as three matrices:
- 'Road' person matrix, representing all highway person trips other than HGV trips;
- 'HGV' person matrix, representing HGV person trips; and
- 'Rail' person matrix, representing all rail person trips.
6.5.7. The mobile network data trips were provided with road and rail trips split in to 5 purposes:
- Home-based work (inbound);
- Home-based work (outbound);
- Home-based other (inbound);
- Home-based other (outbound); and
- Non-home-based other (both directions).
6.5.8. Appendix F-1 contains the mobile network data technical note that Telefonica provided with the data, explaining how this trip data was generated.
6.5.9. This data has been put through a number of processes to split out the trips into the trip types required for the different transport models involved in this work.
6.5.10. A detailed verification of the mobile network data has been carried out to assess fitness for purpose for the transport modelling work. Initial checks of the data had been carried out by Telefonica; however these have been regarded as high level checks with more detailed analysis required. The verification has been carried out through comparison to the National Travel Survey, National Trip-End Model version 6.2, and 2011 Census Journey to Work data at Output area to Work Place Zone level.
6.5.11. Appendix F-2 contains a report which details the extensive verification of the MND which has been carried out by WSP.


## INITIAL ROAD PERSON MATRIX PROCESSING

6.5.12. The road person matrix has been processed through the following major steps to get relevant matrices for this modelling work in terms of car vehicle trips, LGV vehicle trips, and person bus trips:

- Split bus person trips into a separate matrix;
- Convert the remaining person trips into vehicle trips; and
- Split LGV trips into a separate matrix.


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6.5.13. Bus trips were split off from the provided road matrices through factors calculated from National Travel Survey (NTS) data combined with Census Journey to Work data. The Census Journey to Work data was used to work out the proportion of bus users per zone-to-zone movement for outbound home-based work trips, with the NTS data then used to determine proportion matrices for the other purposes.
6.5.14. The conversion of trips from person trips to vehicle trips has been carried out using the WebTAG databook July 2017 edition, which contains vehicle occupancy values for the year 2000 along with adjustments per year. Vehicle occupancies have been applied on both a purpose and time period basis, calculated from the relevant WebTAG values, with an allowance for a proportion of LGVs.
6.5.15. The LGV split has been calculated using:

- MCC Data to work out the proportion of LGVs compared to Cars \& LGVs as a whole;
- Trafficmaster OD Data to work out a trip length distribution for the LGV trips; and
- NTEM Purpose Split information for Car Drivers, which has been used to control the resulting car driver proportions once LGVs are removed.
6.5.16. The total number of LGVs based upon the MCC data are then proportioned across purposes to give the total number of LGVs being split out of each purpose, then this is split into trip length bands and removed from those elements of the matrix to give an LGV matrix of the correct size.
6.5.17. The car and LGV trips are required in passenger car units (PCUs) for the highway model rather than vehicles, but it has been assumed for this work that both car and LGV vehicles are equivalent to a single PCU and therefore no change has been made to these matrices to account for this.
6.5.18. Table 22 shows the matrix changes as these processes are applied.

Table 22 - MND Initial Road Matrix Totals

| Time Period | Purpose | Raw MND | Post-Bus | Vehicle Occupancy | Post-LGV |
| :---: | :---: | :---: | :---: | :---: | :---: |
| AM | HBW Inbound | 267 | 254 | 216 | 203 |
|  | HBW Outbound | 228,023 | 213,268 | 181,721 | 170,268 |
|  | HBO Inbound | 22,586 | 21,218 | 14,963 | 14,041 |
|  | HBO Outbound | 190,562 | 178,704 | 126,025 | 117,775 |
|  | NHB | 97,242 | 92,887 | 68,602 | 43,377 |
| IP | HBW Inbound | 24,848 | 23,033 | 20,029 | 20,029 |
|  | HBW Outbound | 22,863 | 20,813 | 18,098 | 18,098 |
|  | HBO Inbound | 108,470 | 100,875 | 65,080 | 65,080 |
|  | HBO Outbound | 117,359 | 108,191 | 69,801 | 69,801 |
|  | NHB | 128,423 | 121,792 | 86,426 | 50,407 |
| PM | HBW Inbound | 210,456 | 197,187 | 169,784 | 165,984 |
|  | HBW Outbound | 1,322 | 1,216 | 1,047 | 1,025 |
|  | HBO Inbound | 209,104 | 194,217 | 131,673 | 128,821 |
|  | HBO Outbound | 72,999 | 67,687 | 45,889 | 44,919 |
|  | NHB | 124,036 | 117,744 | 85,014 | 54,438 |

6.5.19. The total matrix changes are summarised in Table 23. These percentages are shown as the total matrix change, however the factors are different per purpose within the matrix.

Table 23 - Changes in Matrix Proportions as a Result of Processes

| Time Period | Proportion of Bus <br> Trips | Vehicle Occupancy <br> Reduction | Proportion of LGV <br> Trips |
| :--- | :---: | :---: | :---: |
| AM | $6.0 \%$ | $22.7 \%$ | $11.7 \%$ |
| IP | $6.8 \%$ | $30.8 \%$ | $13.9 \%$ |
| PM | $6.5 \%$ | $25.0 \%$ | $8.8 \%$ |

## INITIAL HGV PERSON MATRIX PROCESSING

6.5.20. The highway matrices are required as passenger car units (PCUs), with HGV trips being factored by 2.3 to provide the relevant PCU equivalent. HGVs have been assumed to be one person per vehicle so no occupancy factor has been applied.

## INITIAL RAIL PERSON MATRIX PROCESSING

6.5.21. The rail matrices are required as person trips and therefore no processing has been carried out on the rail matrices at this stage.

## PURPOSE SPLITTING

6.5.22. All of the matrices from the mobile network data have had a further split applied to the 'other' trips (purposes 6, 7, and 8) to determine the employers business trips (purposes 3, 4, and 5). This split has been carried out using NTEM data, calculated individually for each of car, rail and bus trips. Car trips use data at an MSOA level, while rail and bus trips use data at a district level.
6.5.23. Table 24 shows the proportions applied to split out the Employers Business trips.

Table 24 - Employers Business Purpose Split Proportions

| Time Period | Original Purpose | Car / LGV / HGV |  | Public Transport |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Original <br> Purpose <br> Proportion | Employers Business Purpose Proportion | Original Purpose Proportion | Employers <br> Business <br> Purpose <br> Proportion |
| AM | HBO Inbound | 91.3\% | 8.7\% | 67.9\% | 32.1\% |
|  | HBO Outbound | 91.3\% | 8.7\% | 67.8\% | 32.2\% |
|  | NHB | 69.8\% | 30.2\% | 32.8\% | 67.2\% |
| IP | HBO Inbound | 96.5\% | 3.5\% | 87.3\% | 12.7\% |
|  | HBO Outbound | 96.6\% | 3.4\% | 87.3\% | 12.7\% |
|  | NHB | 78.8\% | 21.2\% | 61.5\% | 38.5\% |
| PM | HBO Inbound | 93.4\% | 6.6\% | 79.3\% | 20.7\% |
|  | HBO Outbound | 93.3\% | 6.7\% | 79.3\% | 20.7\% |
|  | NHB | 82.0\% | 18.0\% | 80.4\% | 19.6\% |

### 6.6 SYNTHETIC MATRIX CREATION

6.6.1. A simplified overview of the synthetic matrix generation process is provided as Figure 28.

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Figure 28 - Synthetic Matrix Creation Process
6.6.2. Synthetic matrices have been generated from Census Journey to Work 2011 data. This data has been used to form a synthetic matrix of trips that represents a 24 hour period for home-based work (outbound) trip purpose for car, rail, and bus trips. This base matrix has been used to create the remaining trip purposes. An initial 24 hour home-based work trips (inbound) matrix has been generated from the transpose of the outbound matrix. These matrices have been created for each of car, rail, and bus modes, as the census data does not cover LGV and HGV data. The inbound and outbound matrices have then been averaged to generate a nondirectional matrix for use when estimating the purposes that have no direction.
6.6.3. National Travel Survey data has then been used to generate an 'all-modes' factor for each journey purpose for each time period, as the survey data for the East of England has too low a sample rate to be able to get specific factors for each mode of travel in this manner. The resulting factored matrices have a distribution of trips that is the same as the journey to work data, but the total number of trips is representative of the purpose required.
6.6.4. The synthetic matrices have then been scaled to include education trips for relevant purposes (home-based work trips) as Census journey to work does not include education trip purposes.

### 6.7 MATRIX SCALING TO BASE YEAR

6.7.1. The mobile network dataset and synthetic matrices have a different year of origin than the base year of the transport models. Scaling factors have been calculated from the National Trip End Model 7.2 dataset, using the software TEMPro 7, to scale each of these matrices so that the data is more representative of the target base year. The factors calculated have been determined for car-driver (applied to Car, LGV, and HGV trips), rail users, and bus users.
6.7.2. The mobile network data matrix has had a scaling factor from 2015 to 2016 calculated and applied as the mobile network data has been taken from April 2015 data, with the synthetic matrix having a 2011 to 2016 scaling factor applied as it is based on 2011 Census data.
6.7.3. Scaling factors have been determined for each district within Suffolk and for the East of England region, with zones scaled depending on what area they fall within. Scaling factor application has been carried out using an average of the origin and destination scaling factors.
6.7.4. Table 25 shows how the final matrix totals have been adjusted based on the scaling factors applied. The proportions shown are the changes in total numbers of trips that result from the district-level scaling factors applied.

Table 25 - Proportional Difference When Applying Yearly Factors

| Time <br> Period | MND |  |  |  |  |  |  | Synthetic |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | :---: | :---: | :---: | :---: |
|  | Car | LGV | HGV | Rail | Bus | Car | Rail | Bus |  |  |  |  |
| AM | $99.2 \%$ | $102.9 \%$ | $100.6 \%$ | $99.4 \%$ | $99.7 \%$ | $96.3 \%$ | $96.5 \%$ | $98.5 \%$ |  |  |  |  |
| IP | $99.9 \%$ | $102.9 \%$ | $100.6 \%$ | $104.7 \%$ | $99.9 \%$ | $99.4 \%$ | $99.8 \%$ | $99.9 \%$ |  |  |  |  |
| PM | $99.4 \%$ | $102.9 \%$ | $100.6 \%$ | $163.4 \%$ | $99.6 \%$ | $97.1 \%$ | $97.7 \%$ | $98.3 \%$ |  |  |  |  |

### 6.8 ZONE SYSTEMS AND CORRESPONDENCE

6.8.1. The transport models have a zone system of 883 zones (numbered up to 906, with a gap between 854 and 878). All of the transport model elements (the highway model, public transport model, and demand model) use the same zone system for ease of data transfer.
6.8.2. The initial zone system used by the mobile network data and used for generating the synthetic matrix data is at a Lower Super Output Area level within Suffolk, with external areas using MSOA, District, or regional areas to represent the zones, with 755 zones in total. The mismatch in the number of zones between this initial system and the transport model system means that a correspondence process has been carried out in order to assign each of the initial zones into the transport model zones within it.
6.8.3. The correspondence process has been applied to the matrices using a number of datasets. These datasets are:

- Number of Adults from Census 2011;
- Number of Employed People from Census 2011;
- Workplace Population from Census 2011;
- Land area per zone information from GIS.
6.8.4. The Census 2011 data has been proportioned out to the relevant zones that intersect the Census features. In some cases, the Census data has been manually adjusted after this intersect process to ensure that populations are in the correct places e.g. the workplace population associated with Ipswich Hospital would have been assigned to the transport network in the wrong place if it had been left in the original assignment position from the intersect, so this workplace population has been moved to an adjacent zone to correct this.
6.8.5. Each trip purpose has been disaggregated using a different combination of datasets, applying the disaggregation as set out in Table 26.

Table 26 - Matrix Disaggregation Datasets by Trip Purpose

| Matrix <br> Level | Purpose | Origin | Destination |
| :--- | :--- | :--- | :--- |
| 1 | Home-based work (inbound) | Workplace Pop. | Employed People |
| 2 | Home-based work (outbound) | Employed People | Workplace Pop. |
| 3 | Home-based employers business <br> (inbound) | Workplace Pop. | Employed People |
| 4 | Home-based employers business <br> (outbound) | Employed People | Workplace Pop. |
| 5 | Non-home-based employers <br> business | Workplace Pop. | Workplace Pop. |
| 6 | Home-based other (inbound) | Workplace Pop | Adults |

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| 7 | Home-based other (outbound) | Adults | Workplace Pop |
| :--- | :--- | :--- | :--- |
| 8 | Non-home-based other | Workplace Pop | Workplace Pop |
| 9 | LGV | Workplace Pop. | Workplace Pop. |
| 10 | HGV | Workplace Pop. | Workplace Pop. |

6.8.6. The correspondence process determines how many trips would be expected to come out of any specific area within these initial zones based on a relevant dataset. For example, home-based work trips for the outbound direction should have origins of homes, and destinations of workplaces (which in this case include places of education). This enables an initial zone that contains say a large residential area, and a large employment area, but where the final zones have these two areas as separate zones, to be split in a way that means the final trip matrices better represent the individual purposes of trips compared to just splitting proportionally by area (or using some other dataset or combination of datasets proportionally) in all cases.

### 6.9 COMBINATION OF MATRICES

6.9.1. The mobile network data and synthetic matrices have been combined to create the final set of matrices. All of the trip pairs in the matrix have had distances determined. These distances have been used to carry out the following:

- Remove all trip pairs with a distance of less than two kilometres for the mobile network data matrices;
- Remove all trip pairs with a distance of equal to or more than two kilometres for the synthetic matrices;
- Combine the two sets of matrices which will now have no zone pairs that overlap.
6.9.2. These combined matrices have been created for each of car, rail, and bus trips. Table 27 shows the matrix totals for each of the matrices for each period. LGV and HGV trips have not been combined as no synthetic matrix has been created for them. The table shows that in general the resulting combined totals are between the MND and synthetic matrix totals. The trip totals exclude intrazonal trips (trips from a zone to itself) as these are not included in the transport modelling process. Trip totals are for the sum of all purpose levels.

Table 27 - Matrix Totals (vehicles) for Combination Process

| Vehicle Type | Matrix Type | AM | IP | PM |
| :--- | :--- | ---: | ---: | ---: |
| Car | MND | 343,036 | 223,116 | 392,837 |
|  | Synthetic | 472,530 | 527,654 | 628,577 |
|  | Combined | 412,096 | 298,514 | 458,034 |
| Bus | MND | 32,231 | 27,221 | 39,720 |
|  | Synthetic | 30,163 | 33,797 | 40,733 |
|  | Combined | 31,940 | 26,544 | 36,475 |
|  | MND | 29,834 | 13,551 | 30,218 |
|  | Synthetic | 32,932 | 37,623 | 45,109 |
|  | Combined | 29,899 | 14,243 | 30,579 |

6.9.3. Car availability has been applied to the combined matrices for bus and rail modes to split these into two separate sets of matrices that represent the 'car available' and 'no car available' matrices. The car availability data has come from Census household data on car ownership levels, looking at the percentage of households with zero cars versus total households, on a zone-zone basis.

### 6.10 RAIL REDISTRIBUTION

6.10.1. The rail trip matrices are generated from origin to destination data, where the primary trip is a rail trip but the origin and destination represent the actual origin and destination of the trip rather than the station to station movement. These trips are in actuality made up of (at least) three legs:

- An initial access leg for travel from the origin to the station (which could itself be made up of multiple parts, such as driving to a park and ride site before taking a bus to the station);
- A rail trip that goes from one station to another station (this may have multiple parts, changing at stations in between or even with other modes such as an underground or bus trip in the middle); and
- A final egress leg for travel from the station to the destination (again could be made up of multiple parts).
6.10.2. In essence, this means that the 'rail' trip is only part of the total trip in the matrix. The following assumptions have been made in order to turn this rail trip in to what is modelled in the transport models:
- The initial leg of the trip is made by car if more than two kilometres in length or sustainable modes if less than two kilometres in length;
- The weighting of which station a traveller will use to determine the destination of the initial leg is based on a gravity model depending on their origin zone;
- Zones external to Suffolk are large enough and provide enough choice of station that travellers from within one of these zones do not need to travel in to another zone in order to get to a station, and therefore their trips are representative of the rail leg entirely;
- The rail trip is the dominant element of the trip with intermediate trips being insignificant; and
- The final leg is a minor element of the trip, and most likely to be made by a sustainable mode and be of a relatively short distance from the end station and therefore negligible in terms of the modelling work being carried out.
6.10.3. These assumptions result in a process that splits the rail trips into two forms; those that access by car and carry out a 'park and ride' trip, and those that do not access by car. The park and ride trips are split up, so that the car elements can be put into the highway matrices and the rail elements can be put into the public transport matrices as a separate type of trip, while those that do not access by car are left as full origindestination trips for the public transport matrices. The non-car-available trips are assumed to be entirely noncar access trips (due to not having a car available), while the car-available trips have gone through a process to get a 'park and ride' public transport matrix and a number of trips to add to the highway matrix.
6.10.4. The full redistribution process is summarised in Figure 29. The non-car-available (NCA) matrices are not shown, but the process is simple for these matrices as the NCA rail and bus matrices are added directly together to form a NCA public transport set of matrices


Figure 29 - Rail Redistribution Process

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6.10.5. The gravity model that has been used to work out the split per origin zone for park and ride type trips uses travel time from the origin zone to the station, station parking availability, and rail service frequency to determine a ranking of the 5 most popular stations. The trips from that origin zone have then been split based on the weighting for these stations.
6.10.6. The resulting car matrix that has come out of the initial trip leg has been added to the highway car matrices. Table 28 shows the additional car trips added to the highway matrices.

Table 28 - Additional Car Trips as a Result of the Rail Redistribution Process

|  | AM | IP | PM |
| :--- | :--- | :--- | :--- |
| Total | 1,620 | 637 | 941 |

## 7 ROUTE CHOICE CALIBRATION AND VALIDATION

### 7.1 ROUTE CHOICE CALIBRATION

7.1.1. The generalised costs have an effect on the route choice made by different user class and trip purposes.
7.1.2. Generalised costs were calculated using values of time, GDP growth rates, purpose splits and vehicle operating costs recommended by the DfT for use in economic appraisal of transport projects in England. These values are present in the July 2016 TAG data book and follow the guidance within the latest version of TAG Unit A1.3 (November 2016). The values calculated for the base year model in terms of Pence per Minute (PPM) and Price per Kilometre (PPK) are shown in Table 6.

### 7.2 ROUTE CHOICE VALIDATION

7.2.1. The routes chosen to validate the route choice were based on the criteria set out in TAG Unit M3.1 (January 2014) and have the following attributes:

- Relate to significant number of trips
- Are of significant length or cost
- Pass through areas of interest
- Include both directions of travel
- Link different compass areas
- Coincide with journey time routes as appropriate
- Study areas for specific scheme appraisals
7.2.2. Routes were plotted for all user classes. Guidance presented in section 7.3 of TAG Unit M3.1 (January 2014), with the number of OD pairs determines as follows:
7.2.3. Number of OD pairs $=(\text { number of zones })^{\wedge} 0.25 \times$ number of user classes
7.2.4. Based on the assignment model zoning system, this equates to 54 routes. The routes that were chosen in the D1 Model Specification Report (February 2016) are described in Table 29 with additional routes related to local study areas for specific scheme appraisals added. These routes were used to validate route choice within the model.

Table 29 - County-Wide Origin-Destination Route Checks

| Route | Origin <br> Zone | Origin Description | Destination <br> Zone | Destination Description | Type |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | 391 | Felixstowe | 412 | Lowestoft | County wide |
| 2 | 238 | Stowmarket | 455 | Beccles | County wide |
| 3 | 287 | Bury St. Edmunds | 391 | Felixstowe | County wide |
| 4 | 316 | Haverhill | 412 | Lowestoft | County wide |
| 5 | 455 | Beccles | 95 | Newmarket | County wide |
| 6 | 146 | lpswich | 33 | Sudbury | County wide |
| 7 | 412 | Lowestoft | 238 | Stowmarket | County wide |
| 8 | 33 | Sudbury | 95 | Newmarket | County wide |
| 9 | 391 | Felixstowe | 95 | Newmarket | County wide |
| 10 | 238 | Stowmarket | 391 | Felixstowe | County wide |
| 11 | 316 | Haverhill | 238 | Stowmarket | County wide |
| 12 | 412 | Lowestoft | 287 | Bury St. Edmunds | County wide |

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| 13 | 287 | Bury St. Edmunds | 316 | Haverhill | County wide |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 14 | 95 | Newmarket | 455 | Beccles | County wide |
| 15 | 33 | Sudbury | 146 | Ipswich | County wide |
| 16 | 287 | Bury St. Edmunds | 412 | Lowestoft | County wide |
| 17 | 455 | Beccles | 33 | Sudbury | County wide |
| 18 | 146 | Ipswich | 412 | Lowestoft | County wide |
| 19 | 412 | Lowestoft | 146 | Ipswich | County wide |
| 20 | 287 | Bury St. Edmunds | 412 | Lowestoft | County wide |
| 21 | 412 | Lowestoft | 287 | Bury St. Edmunds | County wide |
| 22 | 146 | Ipswich | 287 | Bury St. Edmunds | County wide |
| 23 | 391 | Felixstowe | 412 | Lowestoft | County wide |
| 24 | 455 | Beccles | 238 | Stowmarket | County wide |
| 25 | 332 | A12 Near Yoxford | 259 | Woodbridge Road Near | Suffolk Energy |
| 26 | 359 | Woodbridge Road | 332 | A12 Near Yoxford | Suffolk Energy |
| 27 | 348 | Yarmouth road near | 897 | B119 Near Saxmundham | Suffolk Energy |
| 28 | 897 | B119 Near | 348 | Yarmouth road near | Suffolk Energy |
| 29 | 902 | 167 Carr Avenue | 895 | 7 Woodbridge Road near | Suffolk Energy |
| 30 | 895 | 7 Woodbridge Road | 902 | 167 Carr Avenue near | Suffolk Energy |
| 31 | 896 | A12 near | 356 | School Lane, Bromeswell | Suffolk Energy |
| 32 | 356 | School Lane, | 896 | A12 near Saxmundham | Suffolk Energy |
| 33 | 362 | A12, Woodbridge, | 572 | Dunwich Rd, | Suffolk Energy |
| 34 | 572 | Dunwich Rd, | 362 | A12, Woodbridge, Melton | Suffolk Energy |
| 35 | 408 | Stirrups Lane | 465 | A12 Near the Hollies | Lowestoft |
| 36 | 465 | A12 Near the Hollies | 408 | Stirrups Lane | Lowestoft |
| 37 | 84 | 395 Whapload Road | 782 | 40-50 Rectory Road | Lowestoft |
| 38 | 782 | 40-50 Rectory Road | 84 | 395 Whapload Road | Lowestoft |
| 39 | 409 | 5 Lowry Way | 584 | 51-55 Borrow Road | Lowestoft |
| 40 | 584 | 51-55 Borrow Road | 409 | 5 Lowry Way | Lowestoft |
| 41 | 409 | 5 Lowry Way | 589 | 138 Waveney Drive | Lowestoft |
| 42 | 589 | 138 Waveney Drive | 409 | 5 Lowry Way | Lowestoft |
| 43 | 767 | 5 Lulworth Park | 434 | 17 Smith's Walk | Lowestoft |
| 44 | 434 | 17 Smith's Walk | 767 | 5 Lulworth Park | Lowestoft |
| 45 | 586 | 2 The Lease | 781 | Hadenham Road (Near | Lowestoft |


| 46 | 781 | Hadenham Road | 586 | 2 The Lease | Lowestoft |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 47 | 673 | Martlesham | 482 | Copdock Interchange | Ipswich |
| 48 | 482 | Copdock Interchange | 673 | Martlesham | Ipswich |
| 49 | 253 | Claydon | 676 | Brightwell | Ipswich |
| 50 | 676 | Brightwell | 253 | Claydon | Ipswich |
| 51 | 114 | Castle Hill | 185 | Chantry | Ipswich |
| 52 | 185 | Chantry | 114 | Castle Hill | Ipswich |
| 53 | 735 | Hadleigh Road | 194 | Nacton Road | Ipswich |
| 54 | 194 | Nacton Road | 735 | Hadleigh Road Industrial | Ipswich |

7.2.5. The results of the route choice validation can be in seen in Appendix G.
7.2.6. The routes generated were based on Origin-Destination forests as well as showing the lowest cost route, also show other potential routes which are considered by SATURN during assignment.
7.2.7. The figures in Appendix $G$ show the routing between the O-D pairs is consistent across all three peaks and logical, following expecting routes through the simulation network

## 8 TRIP MATRIX CALIBRATION AND VALIDATION

### 8.1 TRIP MATRIX VALIDATION

8.1.1. The initial prior matrix was created as described in Section 7 of this report. The initial prior matrix was assigned within the model and the screenline performance analysed.
8.1.2. The observed data was split into calibration and validation counts, the validation counts were not used in any matrix adjustment or matrix estimation.
8.1.3. Section 3.2 of TAG Unit M3.1 (January 2014) stipulates modelled flows across screenlines for each vehicle type should be within $5 \%$ of observed flows. TAG recommends that this should apply to "all, or nearly all" screenlines.
8.1.4. It is considered a GEH across the screenline of 4.0 or below is acceptable and has been considered in this report when looking at screenline performance. This approach is well aligned with previous versions of WebTAG.
8.1.5. This section provides a discussion of the performance of two different assignments of the SCTM highway model:

- Prior matrix assignment: assignment of matrix which has been unaffected by matrix scaling or matrix estimation
- Post matrix-estimation assignment: the final assignment which is based on the prior matrix being run through matrix estimation
8.1.6. The performance of these two assignments will be discussed in terms of screenline performance.


### 8.2 PRIOR MATRIX ASSIGNMENT

8.2.1. Table 30 to Table 32 show the performance of the prior assignment in terms of the county screenlines. These tables show that the prior matrix has a significantly higher level of traffic compared to the observed traffic across all screenlines. This is because the main data source for the prior matrix is the MND which provides total demand within the study area, whereas the SCTM network represents a simplification of the road network, only including key strategic routes outside of the main towns. This means the total traffic demand where it is represented by zone to zone movements is forced to use the major routes in the model network whereas in reality traffic uses local routes which it would not be feasible to include within a strategic county model.
8.2.2. Table 30 show the performance of the prior matrix against the calibration screenlines in the AM peak

Table 30 - Prior Matrix: Calibration Screenlines - AM peak

| ID | Description | Dir | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Newmarket | IB | 2380 | 2631 | $10.6 \%$ | OB | 2671 | 3005 | $12.5 \%$ |
| 2 | Haverhill | IB | 1538 | 1409 | $-8.4 \%$ | OB | 1747 | 2003 | $14.7 \%$ |
| 3 | Sudbury | IB | 3162 | 3521 | $11.4 \%$ | OB | 2730 | 3673 | $34.5 \%$ |
| 4 | Felixstowe | IB | 1552 | 1801 | $16.1 \%$ | OB | 1559 | 1576 | $1.1 \%$ |
| 5 | Stowmarket | IB | 2149 | 2409 | $12.1 \%$ | OB | 1733 | 2210 | $27.5 \%$ |
| 6 | Bury St. Edmunds | IB | 4406 | 6340 | $43.9 \%$ | OB | 4335 | 4011 | $-7.5 \%$ |
| 7 | Beccles | IB | 1144 | 1130 | $-1.3 \%$ | OB | 1037 | 1102 | $6.3 \%$ |
| 8 | South of Bury St. Edmunds | NB | 811 | 1357 | $67.3 \%$ | OB | 1315 | 2321 | $76.5 \%$ |
| 9 | Waveney | NB | 1268 | 1776 | $40.1 \%$ | SB | 1069 | 2143 | $100.3 \%$ |
| 10 | East of Bury St. Edmunds | EB | 1244 | 1584 | $27.4 \%$ | WB | 690 | 1101 | $59.5 \%$ |
| 11 | Mid Suffolk / Suffolk Coastal | NB | 879 | 1550 | $76.3 \%$ | SB | 1046 | 1682 | $60.8 \%$ |


| 12 | Babergh / Mid Suffolk | EB | 961 | 1827 | $90.1 \%$ | WB | 941 | 1555 | $65.2 \%$ |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 13 | Forest Heath | NB | 1467 | 2492 | $69.9 \%$ | SB | 2380 | 3476 | $46.1 \%$ |
| 14 | North of Bury St. Edmunds | NB | 1251 | 1942 | $55.2 \%$ | SB | 1134 | 1914 | $68.8 \%$ |
| 15 | South Babergh | NB | 818 | 952 | $16.4 \%$ | SB | 980 | 1422 | $45.1 \%$ |
| 16 | Ipswich (Inner) | IB | 10385 | 12902 | $24.2 \%$ | OB | 8923 | 8980 | $0.6 \%$ |
| 17 | Ipswich (Outer) | IB | 10796 | 15426 | $42.9 \%$ | OB | 10548 | 12925 | $22.5 \%$ |
| 18 | North Lowestoft | IB | 1194 | 1624 | $36.0 \%$ | OB | 1454 | 2226 | $53.1 \%$ |
| 19 | South Lowestoft | IB | 1747 | 2289 | $31.1 \%$ | OB | 1741 | 2131 | $22.4 \%$ |

8.2.3. Table 30 shows the following screenlines have modelled flow within $5 \%$ of observed flow in the AM peak prior assignment:

- Felixstowe Inbound
- Beccles Inbound
- Ipswich Inner Outbound
8.2.4. Table 31 details the performance of the county wide screenlines for the inter peak prior assignment.

Table 31 - Prior Matrix: Calibration Screenlines - Inter peak

| ID | Description | Dir | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Newmarket | IB | 1818 | 1631 | $-10.3 \%$ | OB | 1873 | 1518 | $-18.9 \%$ |
| 2 | Haverhill | IB | 1289 | 1066 | $-17.3 \%$ | OB | 1249 | 1083 | $-13.3 \%$ |
| 3 | Sudbury | IB | 2345 | 2453 | $4.6 \%$ | OB | 2565 | 2576 | $0.4 \%$ |
| 4 | Felixstowe | IB | 1152 | 1021 | $-11.4 \%$ | OB | 1143 | 1073 | $-6.1 \%$ |
| 5 | Stowmarket | IB | 1465 | 1370 | $-6.5 \%$ | OB | 1387 | 1154 | $-16.8 \%$ |
| 6 | Bury St. Edmunds | IB | 2907 | 3149 | $8.3 \%$ | OB | 2945 | 3202 | $8.7 \%$ |
| 7 | Beccles | IB | 1050 | 752 | $-28.4 \%$ | OB | 1087 | 726 | $-33.2 \%$ |
| 8 | South of Bury St. Edmunds | NB | 645 | 1124 | $74.1 \%$ | OB | 659 | 1273 | $93.1 \%$ |
| 9 | Waveney | NB | 961 | 1307 | $35.9 \%$ | SB | 957 | 1259 | $31.6 \%$ |
| 10 | East of Bury St. Edmunds | EB | 663 | 783 | $18.1 \%$ | WB | 646 | 771 | $19.3 \%$ |
| 11 | Mid Suffolk / Suffolk Coastal | NB | 604 | 1036 | $71.4 \%$ | SB | 618 | 1018 | $64.6 \%$ |
| 12 | Babergh / Mid Suffolk | EB | 642 | 1017 | $58.4 \%$ | WB | 645 | 927 | $43.6 \%$ |
| 13 | Forest Heath | NB | 1569 | 1991 | $26.9 \%$ | SB | 1751 | 1978 | $13.0 \%$ |
| 14 | North of Bury St. Edmunds | NB | 789 | 1073 | $36.0 \%$ | SB | 828 | 1191 | $43.8 \%$ |
| 15 | South Babergh | NB | 574 | 748 | $30.4 \%$ | SB | 561 | 740 | $31.8 \%$ |
| 16 | Ipswich (Inner) | IB | 7895 | 7911 | $0.2 \%$ | OB | 7993 | 7612 | $-4.8 \%$ |
| 17 | Ipswich (Outer) | IB | 6823 | 9135 | $33.9 \%$ | OB | 7672 | 9038 | $17.8 \%$ |
| 18 | North Lowestoft | IB | 1052 | 1148 | $9.2 \%$ | OB | 1027 | 1132 | $10.2 \%$ |

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8.2.5. Table 31 shows the following screenlines show modelled flow within $5 \%$ of observed flow in the inter peak prior assignment:

- Sudbury Inbound \& Outbound
- Ipswich Inner Inbound \& Outbound
8.2.6. Table 32 details the performance of the county wide screenlines in the PM peak prior assignment.

Table 32 - Prior Matrix: Calibration Screenlines - PM peak

| ID | Description | Dir | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Newmarket | IB | 2941 | 2969 | $0.9 \%$ | OB | 2872 | 2813 | $-2.1 \%$ |
| 2 | Haverhill | IB | 2068 | 2164 | $4.6 \%$ | OB | 1712 | 1580 | $-7.7 \%$ |
| 3 | Sudbury | IB | 3091 | 3537 | $14.4 \%$ | OB | 3586 | 4121 | $14.9 \%$ |
| 4 | Felixstowe | IB | 1517 | 1735 | $14.4 \%$ | OB | 1512 | 1818 | $20.2 \%$ |
| 5 | Stowmarket | IB | 2036 | 2820 | $38.5 \%$ | OB | 2075 | 2271 | $9.4 \%$ |
| 6 | Bury St. Edmunds | IB | 4599 | 4437 | $-3.5 \%$ | OB | 4348 | 6617 | $52.2 \%$ |
| 7 | Beccles | IB | 1323 | 1389 | $5.0 \%$ | OB | 1286 | 1133 | $-11.9 \%$ |
| 8 | South of Bury St. Edmunds | NB | 1211 | 1986 | $64.0 \%$ | OB | 903 | 1857 | $105.8 \%$ |
| 9 | Waveney | NB | 1109 | 2038 | $83.8 \%$ | SB | 1378 | 2004 | $45.5 \%$ |
| 10 | East of Bury St. Edmunds | EB | 827 | 1128 | $36.4 \%$ | WB | 1286 | 1763 | $37.1 \%$ |
| 11 | Mid Suffolk / Suffolk Coastal | NB | 971 | 1492 | $53.7 \%$ | SB | 872 | 1573 | $80.3 \%$ |
| 12 | Babergh / Mid Suffolk | EB | 982 | 1828 | $86.1 \%$ | WB | 1052 | 1701 | $61.7 \%$ |
| 13 | Forest Heath | NB | 2534 | 3845 | $51.7 \%$ | SB | 2109 | 2251 | $6.7 \%$ |
| 14 | North of Bury St. Edmunds | NB | 1194 | 2029 | $69.9 \%$ | SB | 1314 | 1925 | $46.6 \%$ |
| 15 | South Babergh | NB | 1009 | 1443 | $43.0 \%$ | SB | 787 | 1075 | $36.6 \%$ |
| 16 | Ipswich (Inner) | IB | 9633 | 10365 | $7.6 \%$ | OB | 9900 | 12439 | $25.6 \%$ |
| 17 | Ipswich (Outer) | IB | 9953 | 14673 | $47.4 \%$ | OB | 12100 | 14888 | $23.0 \%$ |
| 18 | North Lowestoft | IB | 1851 | 2251 | $21.6 \%$ | OB | 1211 | 1661 | $37.2 \%$ |

8.2.7. Table 32 shows the following screenlines show modelled flow within $5 \%$ of observed flow in the inter peak prior assignment:

- Haverhill Inbound
- Bury St. Edmunds Inbound
- Beccles Inbound
8.2.8. In summary, the results of the screenlines suggest the prior matrix over-estimates the modelled traffic compared the observed flows for key strategic movements covered by the screenlines. This is again deemed to be due to the issue of the MND representing total travel demand being assigned to a simplified highway network.


### 8.3 PRIOR MATRIX SCALING

8.3.1. In order to improve the performance of the model validation and calibration in Lowestoft, scaling was undertaken in instances of observed data being available for zone connector links related to land uses such as retail parks and supermarkets where there was a single point of access. These adjustments lead to a net increase in the size of the final matrix of $0.1 \%$ in the AM peak, $0.1 \%$ in the inter peal and $0.2 \%$ in the PM peak.
8.3.2. The final assignments detailed in this report are consistent with those reported in the local Lowestoft SCTM LMVR (November 2017). The assignment differs from that reported in the local SEGWay SCTM LMVR (December 2017) due to the Lowestoft prior matrix adjustments. These differences did not lead to significant changes in flow in the area of detailed modelling for the Suffolk Energy Gateway scheme appraisal.

### 8.4 POST MATRIX-ESTIMATION MATRIX ASSIGNMENT

8.4.1. Following analysis of the screenline performance of the prior matrix assignment, matrix estimation was used to provide a final adjustment to the matrix. The post matrix-estimation matrix forms the final assignment of the SCTM highway model.
8.4.2. Table 33 compares the scaled prior matrix totals to the post matrix-estimation totals.

Table 33 - Scaled Prior and Post ME Matrix Totals

| User Class | AM Peak Hour <br> $(0800-0900)$ |  | Inter Peak Avg Hour <br> $(1000-1600)$ |  | PM Peak Hour <br> $(1700-1800)$ |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Adj Prior | Post ME | Adj Prior | Post ME | Adj Prior | Post ME |
| UC1 - Car HBW IB | 3046 | 3719 | 7557 | 7899 | 61232 | 53969 |
| UC2 - Car HBW OB | 64460 | 56288 | 6322 | 6562 | 1315 | 1485 |
| UC3 - Car HEB IB | 302 | 316 | 956 | 1036 | 4041 | 4031 |
| UC4 - Car HEB OB | 4433 | 4207 | 853 | 892 | 958 | 967 |
| UC5 - Car NHEB | 7588 | 8186 | 6209 | 6846 | 7165 | 7662 |
| UC6 - Car HBO IB | 4928 | 5510 | 26251 | 28444 | 40921 | 37366 |
| UC7 - Car HBO OB | 37957 | 34885 | 26809 | 28389 | 16164 | 16567 |
| UC8 - Car NHBO | 6298 | 6037 | 13800 | 15004 | 12191 | 11579 |
| UC9 - LGV | 14806 | 12629 | 11563 | 10611 | 11689 | 10292 |
| UC10 - HGV | 4458 | 8810 | 4631 | 9070 | 4988 | 6085 |
| Total | 148275 | 140587 | 104951 | 114752 | 160665 | 150004 |

8.4.3. Table 33 shows that in the AM peak, matrix estimation leads to an decrease in the size of the matrix of $-5 \%$, in the inter-peak there is an increase of $9 \%$, whilst in the PM peak there is a decrease of $-7 \%$.

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## COUNTYWIDE CALIBRATION SCREENLINES

8.4.4. Table 34 to Table 36 details the calibration screenline performance for the post matrix estimation assignments in all peak hours modelled.

Table 34 - Post ME Matrix: Calibration Screenlines - AM peak

| ID | Description | Dir | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Newmarket - Inbound | IB | 2380 | 2364 | -0.7\% | OB | 2671 | 2664 | -0.3\% |
| 2 | Haverhill - Inbound | IB | 1538 | 1538 | 0.0\% | OB | 1747 | 1746 | 0.0\% |
| 3 | Sudbury - Inbound | IB | 3162 | 3213 | 1.6\% | OB | 2730 | 2834 | 3.8\% |
| 4 | Felixstowe - Inbound | IB | 1552 | 1546 | -0.3\% | OB | 1559 | 1570 | 0.7\% |
| 5 | Stowmarket - Inbound | IB | 2149 | 2126 | -1.1\% | OB | 1733 | 1732 | -0.1\% |
| 6 | Bury St. Edmunds Inbound | IB | 4406 | 4468 | 1.4\% | OB | 4335 | 4349 | 0.3\% |
| 7 | Beccles - Inbound | IB | 1144 | 1141 | -0.3\% | OB | 1037 | 1030 | -0.6\% |
| 8 | South of Bury St. Edmunds | NB | 811 | 868 | 7.0\% | OB | 1315 | 1316 | 0.1\% |
| 9 | Waveney | NB | 1268 | 1259 | -0.7\% | SB | 1069 | 1063 | -0.6\% |
| 10 | East of Bury St. Edmunds | EB | 1244 | 1245 | 0.1\% | WB | 690 | 696 | 0.9\% |
| 11 | Mid Suffolk / Suffolk Coastal | NB | 879 | 897 | 2.1\% | SB | 1046 | 1048 | 0.2\% |
| 12 | Babergh / Mid Suffolk Eastbound | EB | 961 | 960 | -0.1\% | WB | 941 | 945 | 0.4\% |
| 13 | Forest Heath Northbound | NB | 1467 | 1472 | 0.3\% | SB | 2380 | 2393 | 0.6\% |
| 14 | North of Bury St. Edmunds | NB | 1251 | 1258 | 0.6\% | SB | 1134 | 1130 | -0.3\% |
| 15 | South Babergh | NB | 818 | 815 | -0.4\% | SB | 980 | 982 | 0.2\% |
| 16 | Ipswich (Inner) | IB | 10385 | 10516 | 1.3\% | OB | 8923 | 9090 | 1.9\% |
| 17 | Ipswich (Outer) | IB | 10796 | 10852 | 0.5\% | OB | 10548 | 10561 | 0.1\% |
| 18 | North Lowestoft | IB | 1194 | 1191 | -0.3\% | OB | 1454 | 1452 | -0.1\% |
| 19 | South Lowestoft | IB | 1747 | 1626 | -6.9\% | OB | 1741 | 1541 | -11.5\% |

8.4.5. Table 34 shows 35 of the 38 screenlines ( $92 \%$ ) shows a flow difference between modelled and observed of less than 5\%.
8.4.6. Table 35 shows the performance of the county wide screenlines in the inter peak.

Table 35 - Post ME Matrix: Calibration Screenlines - Inter peak

| ID | Description | Dir | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Newmarket | IB | 1818 | 1819 | $0.1 \%$ | OB | 1873 | 1870 | $-0.2 \%$ |
| 2 | Haverhill | IB | 1289 | 1286 | $-0.2 \%$ | OB | 1249 | 1249 | $0.0 \%$ |
| 3 | Sudbury | IB | 2345 | 2367 | $0.9 \%$ | OB | 2565 | 2588 | $0.9 \%$ |
| 4 | Felixstowe | IB | 1152 | 1145 | $-0.6 \%$ | OB | 1143 | 1198 | $4.8 \%$ |
| 5 | Stowmarket | IB | 1465 | 1465 | $0.0 \%$ | OB | 1387 | 1386 | $-0.1 \%$ |
| 6 | Bury St. Edmunds | IB | 2907 | 2922 | $0.5 \%$ | OB | 2945 | 2956 | $0.4 \%$ |
| 7 | Beccles | IB | 1050 | 1043 | $-0.7 \%$ | OB | 1087 | 1086 | $-0.1 \%$ |
| 8 | South of Bury St. <br> Edmunds | NB | 645 | 705 | $9.2 \%$ | OB | 659 | 660 | $0.2 \%$ |
| 9 | Waveney | NB | 961 | 962 | $0.0 \%$ | SB | 957 | 957 | $0.0 \%$ |
| 10 | East of Bury St. <br> Edmunds | EB | 663 | 664 | $0.2 \%$ | WB | 646 | 648 | $0.3 \%$ |
| 11 | Mid Suffolk / Suffolk <br> Coastal | NB | 604 | 602 | $-0.4 \%$ | SB | 618 | 617 | $-0.2 \%$ |
| 12 | Babergh / Mid Suffolk <br> Eastbound | EB | 642 | 643 | $0.1 \%$ | WB | 645 | 647 | $0.2 \%$ |
| 13 | Forest Heath | NB | 1569 | 1565 | $-0.3 \%$ | SB | 1751 | 1753 | $0.1 \%$ |
| 14 | North of Bury St. <br> Edmunds | NB | 789 | 790 | $0.1 \%$ | SB | 828 | 829 | $0.2 \%$ |
| 15 | South Babergh | NB | 574 | 574 | $0.0 \%$ | SB | 561 | 561 | $0.0 \%$ |
| 16 | Ipswich (Inner) | IB | 7895 | 7907 | $0.1 \%$ | OB | 7993 | 7994 | $0.0 \%$ |
| 17 | Ipswich (Outer) | IB | 6823 | 6819 | $-0.1 \%$ | OB | 7672 | 7678 | $0.1 \%$ |
| 18 | North Lowestoft | IB | 1052 | 1030 | $-2.1 \%$ | OB | 1027 | 1009 | $-1.7 \%$ |
| 19 | South Lowestoft | IB | 1665 | 1587 | $-4.7 \%$ | OB | 1662 | 1595 | $-4.0 \%$ |

8.4.7. Table 35 shows 37 of the 38 screenlines (97\%) return a difference between modelled and observed flow of less than 5\%.

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8.4.8. Table 36 outlines the performance of the calibration screenlines in the PM peak.

Table 36 - Post ME Matrix: Calibration Screenlines - PM peak

| ID | Description | Dir | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Newmarket - Inbound | IB | 2941 | 2978 | $1.2 \%$ | OB | 2872 | 2911 | $1.3 \%$ |
| 2 | Haverhill - Inbound | IB | 2068 | 2071 | $0.1 \%$ | OB | 1712 | 1716 | $0.2 \%$ |
| 3 | Sudbury - Inbound | IB | 3091 | 3120 | $1.0 \%$ | OB | 3586 | 3662 | $2.1 \%$ |
| 4 | Felixstowe - Inbound | IB | 1517 | 1512 | $-0.3 \%$ | OB | 1512 | 1565 | $3.5 \%$ |
| 5 | Stowmarket - Inbound | IB | 2036 | 2031 | $-0.2 \%$ | OB | 2075 | 2065 | $-0.5 \%$ |
| 6 | Bury St. Edmunds - <br> Inbound | IB | 4599 | 4662 | $1.4 \%$ | OB | 4348 | 4357 | $0.2 \%$ |
| 7 | Beccles - Inbound | IB | 1323 | 1318 | $-0.3 \%$ | OB | 1286 | 1283 | $-0.3 \%$ |
| 8 | South of Bury St. <br> Edmunds | NB | 1211 | 1205 | $-0.5 \%$ | OB | 903 | 906 | $0.4 \%$ |
| 9 | Waveney | NB | 1109 | 1112 | $0.2 \%$ | SB | 1378 | 1371 | $-0.5 \%$ |
| 10 | East of Bury St. <br> Edmunds | EB | 827 | 832 | $0.6 \%$ | WB | 1286 | 1300 | $1.1 \%$ |
| 11 | Mid Suffolk / Suffolk <br> Coastal | NB | 971 | 974 | $0.3 \%$ | SB | 872 | 875 | $0.3 \%$ |
| 12 | Babergh / Mid Suffolk <br> - Eastbound | EB | 982 | 984 | $0.2 \%$ | WB | 1052 | 1056 | $0.4 \%$ |
| 13 | Forest Heath - <br> Northbound | NB | 2534 | 2540 | $0.2 \%$ | SB | 2109 | 2133 | $1.1 \%$ |
| 14 | North of Bury St. <br> Edmunds | NB | 1194 | 1199 | $0.5 \%$ | SB | 1314 | 1305 | $-0.6 \%$ |
| 15 | South Babergh | NB | 1009 | 1006 | $-0.3 \%$ | SB | 787 | 789 | $0.3 \%$ |
| 16 | Ipswich (Inner) | IB | 9633 | 9784 | $1.6 \%$ | OB | 9900 | 10034 | $1.4 \%$ |
| 17 | lpswich (Outer) | IB | 9953 | 9998 | $0.5 \%$ | OB | 12100 | 12126 | $0.2 \%$ |
| 18 | North Lowestoft | IB | 1851 | 1829 | $-1.2 \%$ | OB | 1211 | 1207 | $-0.3 \%$ |
| 19 | South Lowestoft | IB | 2021 | 1899 | $-6.1 \%$ | OB | 1725 | 1784 | $3.4 \%$ |
|  | Sos |  |  |  |  |  |  |  |  |

8.4.9. Table 36 shows 37 of the 38 calibration screenlines ( $97 \%$ ) achieve a difference between modelled and observed flow of less than $5 \%$.
8.4.10. Analysis of the screenline performance for the county wide screenlines shows a high level of performance in terms of the model being able to replicate observed flows, particularly for the calibration screenlines. The majority of screenlines show a flow difference within $5 \%$ between modelled and observed flow. This gives confidence the SCTM shows a close match between the observed and modelled flow for the key strategic movements within the county.
8.4.11. Appendix H provides details of GEH and flow performance of counts within the county wide screenlines.

## IPSWICH LOCAL SCREENLINES

8.4.12. Table 37 shows the performance of the screenlines within Ipswich.

Table 37 - Post ME Matrix: Ipswich Screenlines - AM peak

| ID | Description | Dir | Type | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | East / West Screenline 1 (western) | EB | Val | 4423 | 3574 | -19.2\% | WB | 4073 | 3396 | -16.6\% |
| 2 | East / West Screenline 2 (North West) | EB | Cal | 2578 | 2655 | 3.0\% | WB | 1862 | 1919 | 3.1\% |
| 3 | East / West Screenline 3 (South West) | EB | Cal | 2694 | 2707 | 0.5\% | WB | 1674 | 1704 | 1.8\% |
| 4 | East / West Screenline 4 (East) | EB | Cal | 3588 | 3643 | 1.5\% | WB | 3397 | 3418 | 0.6\% |
| 5 | East / West Screenline 5 (Far East) | EB | Val | 2062 | 1796 | -12.9\% | WB | 1206 | 880 | -27.1\% |
| 6 | East / West Screenline 6 (Central) | EB | Val | 2428 | 2814 | 15.9\% | WB | 3235 | 3302 | 2.1\% |
| 7 | North / south Screenline 1 (Northern) | NB | Cal | 1212 | 1215 | 0.2\% | SB | 2088 | 2044 | -2.1\% |
| 8 | North / south Screenline 2 (Southern) | NB | Val | 4297 | 4274 | -0.5\% | SB | 3368 | 3121 | -7.3\% |
| 16 | Ipswich (Inner) | IB | Cal | 10385 | 10516 | 1.3\% | OB | 8923 | 9090 | 1.9\% |
| 17 | Ipswich (Outer) | IB | Cal | 10796 | 10852 | 0.5\% | OB | 10548 | 10561 | 0.1\% |

8.4.13. Table 37 shows all calibration screenlines show differences between modelled and observed flow of less than $5 \%$. Validation screenlines are shown to generally show differences of greater than 5\%.
8.4.14. Table 38 details the performance of the local lpswich screenlines in the inter peak.

Table 38 - Post ME Matrix: Ipswich Screenlines - Inter peak

| ID | Description | Dir | Type | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | East / West Screenline 1 (western) | EB | Val | 3101 | 2826 | -8.9\% | WB | 3342 | 2929 | -12.4\% |
| 2 | East / West Screenline 2 (North West) | EB | Cal | 1745 | 1745 | 0.0\% | WB | 1743 | 1723 | -1.2\% |
| 3 | East / West Screenline 3 (South West) | EB | Cal | 1714 | 1718 | 0.2\% | WB | 1647 | 1660 | 0.8\% |
| 4 | East / West Screenline 4 (East) | EB | Cal | 2913 | 2916 | 0.1\% | WB | 3105 | 3093 | -0.4\% |
| 5 | East / West Screenline 5 (Far East) | EB | Val | 1156 | 921 | -20.3\% | WB | 1224 | 926 | -24.3\% |
| 6 | East / West Screenline 6 (Central) | EB | Val | 2334 | 2296 | -1.7\% | WB | 2486 | 2659 | 7.0\% |
| 7 | North / south Screenline 1 (Northern) | NB | Cal | 985 | 989 | 0.5\% | SB | 988 | 990 | 0.2\% |
| 8 | North / south Screenline 2 (Southern) | NB | Val | 2628 | 2714 | 3.3\% | SB | 2655 | 2685 | 1.1\% |
| 16 | Ipswich (Inner) | IB | Cal | 7895 | 7907 | 0.1\% | OB | 7993 | 7994 | 0.0\% |
| 17 | Ipswich (Outer) | IB | Cal | 6823 | 6819 | -0.1\% | OB | 7672 | 7678 | 0.1\% |

8.4.15. Table 38 shows all calibration screenlines show modelled flows within $5 \%$ of observed flows in the inter peak. Validation screenlines are shown to generally show differences of greater than $5 \%$.
8.4.16. Table 39 details the performance of the local lpswich screenlines in the PM peak.

Table 39 - Post ME Matrix: Ipswich Screenlines - PM peak

| ID | Description | Dir | Type | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | East / West Screenline 1 (western) | EB | Val | 4293 | 3891 | -9.4\% | WB | 4744 | 3875 | -18.3\% |
| 2 | East / West Screenline 2 (North West) | EB | Cal | 2205 | 2281 | 3.5\% | WB | 2320 | 2328 | 0.3\% |
| 3 | East / West Screenline 3 (South West) | EB | Cal | 2108 | 2096 | -0.6\% | WB | 2380 | 2448 | 2.9\% |
| 4 | East / West Screenline 4 (East) | EB | Cal | 3295 | 3364 | 2.1\% | WB | 4076 | 4089 | 0.3\% |
| 5 | East / West Screenline 5 (Far East) | EB | Val | 1374 | 976 | -29.0\% | WB | 2322 | 1801 | -22.5\% |
| 6 | East / West Screenline 6 (Central) | EB | Val | 2849 | 2940 | 3.2\% | WB | 2544 | 2757 | 8.4\% |
| 7 | North / south Screenline 1 (Northern) | NB | Cal | 1909 | 1910 | 0.1\% | SB | 1255 | 1255 | 0.0\% |
| 8 | North / south Screenline 2 (Southern) | NB | Val | 3456 | 3333 | -3.6\% | SB | 3885 | 4397 | 13.2\% |
| 16 | Ipswich (Inner) | IB | Cal | 9633 | 9784 | 1.6\% | OB | 9900 | 10034 | 1.4\% |
| 17 | Ipswich (Outer) | IB | Cal | 9953 | 9998 | 0.5\% | OB | 12100 | 12126 | 0.2\% |

8.4.17. Table 39 shows all calibration screenlines show modelled flows within $5 \%$ of observed flows in the PM peak. Validation screenlines are shown to generally show differences of greater than 5\%
8.4.18. In summary, the local screenlines show the calibration screenlines match well in terms of modelled flow compared to observed flow. The validation screenlines indicate further improvements are required to the model within Ipswich.

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## LOWESTOFT LOCAL SCREENLINES

8.4.19. Table 40 describes the performance of the local Lowestoft screenlines within the AM peak.

Table 40 - Post ME Matrix: Lowestoft Screenlines - AM peak

| ID | Description | Dir | Type | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Lowestoft <br> Screenline 1 | NB | Cal | 2207 | 2207 | $0.0 \%$ | SB | 1685 | 1693 | $0.5 \%$ |
| 2 | Lowestoft <br> Screenline 2 | NB | Cal | 2866 | 2872 | $0.2 \%$ | SB | 2095 | 2114 | $0.9 \%$ |
| 3 | Lowestoft <br> Screenline 3 | EB | Val | 1747 | 1626 | $-6.9 \%$ | WB | 1741 | 1541 | $-11.5 \%$ |
| 4 | Lowestoft <br> Screenline 4 | NB | Val | 2419 | 2308 | $-4.6 \%$ | SB | 1732 | 1883 | $8.7 \%$ |
| 5 | Lowestoft <br> Screenline 5 | NB | Val | 1359 | 1344 | $-1.1 \%$ | SB | 1024 | 1052 | $2.7 \%$ |
| 18 | North Lowestoft | IB | Cal | 1194 | 1191 | $-0.3 \%$ | OB | 1454 | 1452 | $-0.1 \%$ |
| 19 | South Lowestoft | IB | Cal | 1421 | 1452 | $2.2 \%$ | OB | 1330 | 1364 | $2.6 \%$ |

8.4.20. Table 40 shows all calibration screenlines show modelled flows within $5 \%$ of observed flows in the AM peak within Lowestoft.
8.4.21. Table 41 describes the performance of the local Lowestoft screenlines within the inter peak.

Table 41 - Post ME Matrix: Lowestoft Screenlines - Inter peak

| ID | Description | Dir | Type | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Lowestoft <br> Screenline 1 | NB | Cal | 1826 | 1828 | $0.1 \%$ | SB | 1873 | 1876 | $0.1 \%$ |
| 2 | Lowestoft <br> Screenline 2 | NB | Cal | 2473 | 2481 | $0.3 \%$ | SB | 2635 | 2628 | $-0.3 \%$ |
| 3 | Lowestoft <br> Screenline 3 | EB | Val | 1665 | 1587 | $-4.7 \%$ | WB | 1662 | 1595 | $-4.0 \%$ |
| 4 | Lowestoft | NB | Val | 1857 | 2036 | $9.6 \%$ | SB | 2013 | 2188 | $8.7 \%$ |
| 5 | Screenline 4 <br> Lowestoft <br> Screenline 5 | NB | Val | 940 | 936 | $-0.5 \%$ | SB | 905 | 936 | $3.4 \%$ |
| 18 | North Lowestoft | IB | Cal | 1052 | 1030 | $-2.1 \%$ | OB | 1027 | 1009 | $-1.7 \%$ |
| 19 | South Lowestoft | IB | Cal | 1326 | 1339 | $1.0 \%$ | OB | 1347 | 1361 | $1.0 \%$ |

8.4.22. Table 41 shows all calibration screenlines show modelled flows within $5 \%$ of observed flows in the inter peak within Lowestoft. Validation screenlines are shown to have differences of within $5 \%$ in the majority of cases.
8.4.23. Table 42 describes the performance of the local Lowestoft screenlines within the PM peak

Table 42 - Post ME Matrix: Lowestoft Screenlines - PM peak

| ID | Description | Dir | Type | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Lowestoft Screenline 1 | NB | Cal | 2145 | 2145 | 0.0\% | SB | 2284 | 2292 | 0.3\% |
| 2 | Lowestoft Screenline 2 | NB | Cal | 2471 | 2473 | 0.1\% | SB | 3219 | 3228 | 0.3\% |
| 3 | Lowestoft Screenline 3 | EB | Val | 2021 | 1899 | -6.1\% | WB | 1725 | 1784 | 3.4\% |
| 4 | Lowestoft Screenline 4 | NB | Val | 2041 | 2113 | 3.5\% | SB | 2528 | 3001 | 18.7\% |
| 5 | Lowestoft Screenline 5 | NB | Val | 1129 | 1116 | -1.2\% | SB | 1598 | 1699 | 6.3\% |
| 18 | North Lowestoft | IB | Cal | 1851 | 1829 | -1.2\% | OB | 1211 | 1207 | -0.3\% |
| 19 | South Lowestoft | IB | Cal | 1792 | 1801 | 0.5\% | OB | 1517 | 1536 | 1.2\% |

8.4.24. Table 42 shows all calibration screenlines show modelled flows within $5 \%$ of observed flows in the PM peak within Lowestoft.
8.4.25. In summary the screenlines within Lowestoft show the model is able to replicate key strategic movements within this locality.

## SUFFOLK ENERGY GATEWAY LOCAL SCREENLINES

8.4.26. The values in this report related to the local Suffolk Energy Gateway screenlines differ to those presented in the local LMVR for this scheme appraisal due to the adjustments undertaken to the prior matrix in Lowestoft. The modelled flows are based on the post matrix estimation based on this updated prior matrix.
8.4.27. Table 43 describes the performance of the local Suffolk Energy Gateway screenlines within the AM peak.

Table 43 - Post ME Matrix: Suffolk Energy Gateway Screenlines - AM peak

| ID | Description | Dir | Type | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Leiston | IB | Cal | 755 | 764 | $1.2 \%$ | OB | 912 | 910 | $-0.3 \%$ |
| 2 | North / West of A12 | NB | Cal | 588 | 584 | $-0.7 \%$ | SB | 693 | 688 | $-0.8 \%$ |
| 3 | South / East of A12 | NB | Cal | 1051 | 1078 | $2.6 \%$ | SB | 1010 | 1003 | $-0.7 \%$ |
| 4 | Saxmundham | IB | Cal | 1018 | 1021 | $0.3 \%$ | OB | 915 | 920 | $0.6 \%$ |
| 5 | Framlingham | IB | Cal | 941 | 934 | $-0.7 \%$ | OB | 678 | 678 | $0.0 \%$ |
| 6 | Woodbridge | IB | Cal | 2536 | 2535 | $0.0 \%$ | OB | 1987 | 1986 | $-0.1 \%$ |
| 7 | North / South <br> screenline | NB | Cal | 1276 | 1277 | $0.1 \%$ | SB | 1804 | 1806 | $0.1 \%$ |
| 8 | East / West <br> screenline | EB | Cal | 1069 | 1071 | $0.2 \%$ | WB | 1105 | 1107 | $0.2 \%$ |
| 9 | NE / SW screenline | NEB | Val | 815 | 898 | $10.3 \%$ | SWB | 1095 | 1074 | $-1.9 \%$ |
| 10 | North / South <br> screenline | EB | Val | 933 | 919 | $-1.6 \%$ | WB | 1072 | 1167 | $9.0 \%$ |

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8.4.28. Table 43 shows all of the calibration screenlines are within $5 \%$ in the $A M$ peak, whilst this is the case for half of the validation screenlines.
8.4.29. Table 44 describes the performance of the local Suffolk Energy Gateway screenlines within the inter peak.

Table 44 - Post ME Matrix: Suffolk Energy Gateway Screenlines - Inter peak

| ID | Description | Dir | Type | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Leiston | IB | Cal | 704 | 714 | $1.3 \%$ | SB | 703 | 704 | $0.1 \%$ |
| 2 | North / West of A12 | NB | Cal | 435 | 435 | $-0.1 \%$ | SB | 458 | 459 | $0.2 \%$ |
| 3 | South / East of A12 | NB | Cal | 872 | 873 | $0.1 \%$ | SB | 869 | 869 | $0.1 \%$ |
| 4 | Saxmundham | IB | Cal | 818 | 819 | $0.1 \%$ | SB | 828 | 832 | $0.5 \%$ |
| 5 | Framlingham | IB | Cal | 509 | 508 | $-0.2 \%$ | SB | 503 | 503 | $-0.1 \%$ |
| 6 | Woodbridge | IB | Cal | 1606 | 1608 | $0.1 \%$ | SB | 1636 | 1633 | $-0.2 \%$ |
| 7 | North / South <br> screenline | NB | Cal | 1125 | 1125 | $0.0 \%$ | SB | 1144 | 1146 | $0.2 \%$ |
| 8 | East / West <br> screenline | EB | Cal | 958 | 958 | $-0.1 \%$ | SB | 968 | 969 | $0.1 \%$ |
| 9 | NE / SW screenline | NEB | Val | 754 | 801 | $6.2 \%$ | SB | 761 | 792 | $4.1 \%$ |
| 10 | North / South <br> screenline | EB | Val | 967 | 915 | $-5.3 \%$ | SB | 922 | 901 | $-2.3 \%$ |

8.4.30. Table 44 shows all calibration screenlines in the inter peak return a very close match between modelled and observed flow. All validation screenlines show differences within $5 \%$ when comparing modelled and observed flow.
8.4.31. Table 45 describes the performance of the local Suffolk Energy Gateway screenlines within the PM peak.

Table 45 - Post ME Matrix: Suffolk Energy Gateway Screenlines - PM peak

| ID | Description | Dir | Type | Obs | Mod | Diff | Dir | Obs | Mod | Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Leiston | IB | Cal | 912 | 920 | $0.9 \%$ | SB | 920 | 920 | $0.0 \%$ |
| 2 | North / West of A12 | NB | Cal | 620 | 618 | $-0.4 \%$ | SB | 598 | 592 | $-1.0 \%$ |
| 3 | South / East of A12 | NB | Cal | 949 | 954 | $0.6 \%$ | SB | 1107 | 1103 | $-0.3 \%$ |
| 4 | Saxmundham | IB | Cal | 901 | 905 | $0.5 \%$ | SB | 1009 | 1013 | $0.4 \%$ |
| 5 | Framlingham | IB | Cal | 626 | 624 | $-0.2 \%$ | SB | 744 | 744 | $0.0 \%$ |
| 6 | Woodbridge | IB | Cal | 1902 | 1917 | $0.8 \%$ | SB | 2565 | 2568 | $0.1 \%$ |
| 7 | North / South <br> screenline | NB | Cal | 1637 | 1631 | $-0.4 \%$ | SB | 1449 | 1453 | $0.2 \%$ |
| 8 | East / West <br> screenline | EB | Cal | 1139 | 1136 | $-0.3 \%$ | SB | 1045 | 1048 | $0.3 \%$ |
| 9 | NE / SW screenline | NEB | Val | 1045 | 1083 | $3.5 \%$ | SB | 903 | 894 | $-1.1 \%$ |
| 10 | North / South <br> screenline | EB | Val | 1235 | 1211 | $-1.9 \%$ | SB | 892 | 834 | $-6.4 \%$ |

8.4.32. Table 45 shows all calibration screenlines in the PM peak show a very close match between modelled and observed flow. All validation screenlines show differences within or close to $5 \%$ when comparing modelled and observed flow.
8.4.33. In summary, the screenlines within the area of detailed modelling for the Suffolk Energy Gateway show key strategic movements are matched well between observed and modelled flows. This forms a suitable basis from which to undertake scheme appraisal.

## IMPACT OF MATRIX ESTIMATION

8.4.34. Figure 30 to Figure 32 shows graphs which compared the proportion of trips in the scaled prior assignment and post matrix-estimation assignment by trip length band in kilometres. This analysis provides a check that matrix estimation is not fundamentally changing the distribution of the prior matrix.


Figure 30-AM Peak Trip Length Distribution


Figure 31 - Inter peak Trip Length Distribution


Figure 32 - PM Peak Trip Length Distribution
8.4.35. The graphs across all three peaks show similar proportions between the prior matrix and final matrix in terms of proportions of trips longer than 5 km . Matrix estimation is shown to most significantly increase the proportion of trips below 5 km , by between $5-8 \%$ across all three peaks.
8.4.36. Table 46 to Table 48 provide details of the regression statistics which looks at the changes occurring in the matrix as a result of matrix estimation.

Table 46 - Regression Statistics AM Peak

| Measurement |  | Requirement | Value | Pass? |
| :---: | :---: | :---: | :---: | :---: |
| Cells | Slope | Within 0.98 and 1.02 | 0.99 | Yes |
|  | Intercept | Near 0 | -0.01 | Yes |
|  | R-Sq | $>0.95$ | 0.98 | Yes |
| Rows | Slope | Within 0.99 and 1.01 | 0.92 | No |
|  | Intercept | Near 0 | 3.90 | Yes |
|  | R-Sq | > 0.98 | 0.95 | No |
| Columns | Slope | Within 0.99 and 1.01 | 0.87 | No |
|  | Intercept | Near 0 | 10.0 | Yes |
|  | R-Sq | > 0.98 | 0.94 | No |
| Mean | Prior | Within 5\% | 26.95 | No |
|  | Post |  | 24.90 |  |
|  | Diff |  | 8\% |  |
| SD | Prior | Within 5\% | 95.95 | Yes |
|  | Post |  | 96.18 |  |
|  | Diff |  | 1\% |  |

8.4.37. Table 46 shows that in the AM peak, the changes to the matrix following matrix estimation pass the criteria in terms of cells. For all other criteria the model is either close to or meets criteria.

Table 47 - Regression Statistics Inter Peak

| Measurement |  | Requirement | Value | Pass? |
| :---: | :---: | :---: | :---: | :---: |
| Cells | Slope | Within 0.98 and 1.02 | 1 | Yes |
|  | Intercept | Near 0 | 0.007 | Yes |
|  | R-Sq | > 0.95 | 0.99 | Yes |
| Rows | Slope | Within 0.99 and 1.01 | 0.99 | Yes |
|  | Intercept | Near 0 | 9.21 | Yes |
|  | R-Sq | $>0.98$ | 0.95 | No |
| Columns | Slope | Within 0.99 and 1.01 | 0.99 | No |
|  | Intercept | Near 0 | 9.39 | Yes |
|  | R-Sq | > 0.98 | 0.95 | No |
| Mean | Prior | Within 5\% | 25.37 | No |
|  | Post |  | 23.45 |  |
|  | Diff |  | 7.6\% |  |
| SD | Prior | Within 5\% | 130.78 | Yes |
|  | Post |  | 124.78 |  |
|  | Diff |  | 4.6\% |  |

8.4.38. Table 47 shows that in the inter peak, the changes to the matrix following matrix estimation pass the criteria in terms of cells. For all other criteria the model is either close to or meets criteria.

Table 48 - Regression Statistics PM Peak

| Measurement |  | Requirement | Value | Pass? |
| :---: | :---: | :---: | :---: | :---: |
| Cells | Slope | Within 0.98 and 1.02 | 1.00 | Yes |
|  | Intercept | Near 0 | -0.01 | Yes |
|  | R-Sq | > 0.95 | 0.00 | Yes |
| Rows | Slope | Within 0.99 and 1.01 | 0.99 | No |
|  | Intercept | Near 0 | 0.90 | Yes |
|  | R-Sq | > 0.98 | 4.54 | No |
| Columns | Slope | Within 0.99 and 1.01 | 0.07 | No |
|  | Intercept | Near 0 | 0.94 | Yes |
|  | R-Sq | > 0.98 | 0.92 | No |
| Mean | Prior | Within 5\% | 26.53 | No |
|  | Post |  | 23.91 |  |
|  | Diff |  | 10\% |  |
| SD | Prior | Within 5\% | 91.73 | Yes |
|  | Post |  | 92.27 |  |
|  | Diff |  | 1\% |  |

8.4.39. Table 48 shows that in the PM peak, the changes to the matrix following matrix estimation pass the criteria in terms of cells. For all other criteria the model is either close to or meets criteria.
8.4.40. The results for all three peaks show that at an individual cell level the impacts of matrix estimation are all within the criteria stipulated within WebTAG.

## 9 ASSIGNMENT CALIBRATION AND VALIDATION

### 9.1 MODEL CONVERGENCE

9.1.1. Table 49 to Table 51 shows the convergence results against WebTAG criteria for each peak hour modelled.

Table 49-AM peak Convergence Results

| Iteration | Delta | \%Flow | \%Gap |
| :---: | :---: | :---: | :---: |
| 18 | 0.0179 | 98 | 0.014 |
| 19 | 0.0161 | 98.4 | 0.012 |
| 20 | 0.0147 | 98.7 | 0.011 |
| 21 | 0.0139 | 99 | 0.0093 |

Table 50 - Inter peak Convergence Results

| Iteration | Delta | \%Flow | \%Gap |
| :---: | :---: | :---: | :---: |
| 16 | 0.0062 | 98.4 | 0.023 |
| 17 | 0.0063 | 98.8 | 0.01 |
| 18 | 0.0064 | 98.7 | 0.0065 |
| 19 | 0.0044 | 98.4 | 0.0056 |

Table 51 - PM peak Convergence Results

| Iteration | Delta | \%Flow | \%Gap |
| :---: | :---: | :---: | :---: |
| 20 | 0.0182 | 98.1 | 0.034 |
| 21 | 0.0206 | 98.5 | 0.035 |
| 22 | 0.0263 | 98.2 | 0.034 |
| 23 | 0.0196 | 98.5 | 0.03 |

9.1.2. The model convergence results show the SCTM successfully converges to the WebTAG requirements in all three peaks.

### 9.2 ASSIGNMENT CALIBRATION

9.2.1. Assignment calibration involved steps to identify any issues that prevented an acceptable level of calibration of the network, route choice and trip matrix. This includes:

- Checking appropriateness of centroid connectors
- Production of forests to understand nature of competing routes between OD pairs
- Checking representation of queues on surveyed journey time routes.


### 9.3 ASSIGNMENT VALIDATION

9.3.1. Link flow validation and calibration results for the final post matrix estimation shown an improved situation compared to the scaled matrix. Section 10 details the final assignment and shows a close match between modelled and observed flows across an array of screenlines.

## VALIDATION AND CALIBRATION - INDIVIDUAL COUNT PERFORMANCE

9.3.2. The calibration and validation results for all user classes in the AM peak are shown in Table 52. This shows in terms of calibration counts, $82 \%$ achieve a GEH below 5 which is marginally outside the target specified in WebTAG.

Table 52 - AM Peak - All User Classes - Calibration and Validation results

| Criteria and Measure |  | Acceptability Guideline | Calibration |  |  | Validation |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total Counts | Meet Criteria | \% | Total Counts | Meet Criteria | \% |
| Flow Criteria |  |  |  |  |  |  |  |  |
| < 700 vph | $\pm 100 \mathrm{vph}$ |  | $\begin{aligned} & >85 \% \text { of } \\ & \text { links } \end{aligned}$ | 1022 | 884 | 86\% | 200 | 140 | 70\% |
| $\begin{aligned} & 700-2,700 \\ & \text { vph } \end{aligned}$ | $\pm 15 \%$ | $\begin{aligned} & >85 \% \text { of } \\ & \text { links } \end{aligned}$ | 195 | 173 | 89\% | 54 | 38 | 70\% |
| > 2,700 vph | $\pm 400$ vph | $\begin{aligned} & >85 \% \text { of } \\ & \text { links } \end{aligned}$ | 7 | 7 | 100\% | 0 | 0 | 0\% |
| GEH Criteria |  |  |  |  |  |  |  |  |
| GEH Statistic for individual links < 5 |  | $\begin{aligned} & >85 \% \text { of } \\ & \text { links } \end{aligned}$ | 1224 | 1011 | 83\% | 254 | 166 | 65\% |

9.3.3. Table 53 shows the breakdown of calibration and validation count performance by GEH band for the AM peak. This shows that when calibration and validation count performance is combined, $80 \%$ achieve a GEH of 5 or less which achieves the target outlined in WebTAG. This rises to $84 \%$ for a GEH below 6, and $89 \%$ for a GEH below 8 , this implies there are a number of counts falling marginally outside the WebTAG target for a GEH below 5 . For all flow criteria categories, the requirement of $85 \%$ or greater is achieved for calibration counts in the AM peak.

Table 53-AM Peak - All User Classes - Calibration and Validation results

| GEH Range | Calibration |  | Validation |  | Combined |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| GEH < 2 | 780 | $64 \%$ | 11 | $4 \%$ | 791 | $54 \%$ |
| GEH < 4 | 943 | $77 \%$ | 138 | $54 \%$ | 1081 | $73 \%$ |
| GEH < 6 | 1059 | $87 \%$ | 186 | $73 \%$ | 1245 | $84 \%$ |
| GEH < 8 | 1113 | $91 \%$ | 209 | $82 \%$ | 1322 | $89 \%$ |
| GEH < 10 | 1157 | $95 \%$ | 223 | $88 \%$ | 1380 | $93 \%$ |
| GEH <5 | 1011 | $83 \%$ | 166 | $65 \%$ | 1177 | $80 \%$ |

9.3.4. The calibration and validation results for all user classes in the Inter peak are shown in Table 54. This shows $86 \%$ of calibration counts achieve a GEH of 5 or less which is above the target of $85 \%$ stipulated in WebTAG. In combination with validation counts, $83 \%$ of counts have a GEH below 5 which is marginally outside the target outlined within WebTAG. For the " $<700 \mathrm{vph}$ " and " $700-2700 \mathrm{vph}$ " flow criteria categories, the requirement of $85 \%$ or greater is achieved for calibration counts in the inter peak.

Table 54 - Inter Peak - All User Classes - Calibration and Validation results

| Criteria and Measure |  | Acceptability Guideline | Calibration |  |  | Validation |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total Counts | Meet Criteria | \% | Total Counts | Meet Criteria | \% |
| Flow Criteria |  |  |  |  |  |  |  |  |
| < 700 vph | $\pm 100 \mathrm{vph}$ |  | $\begin{aligned} & >85 \% \text { of } \\ & \text { links } \end{aligned}$ | 1085 | 1007 | 93\% | 228 | 177 | 78\% |
| $\begin{aligned} & 700-2,700 \\ & \text { vph } \end{aligned}$ | $\pm 15 \%$ | $\begin{aligned} & >85 \% \text { of } \\ & \text { links } \end{aligned}$ | 139 | 129 | 93\% | 26 | 21 | 81\% |
| > 2,700 vph | $\pm 400 \mathrm{vph}$ | $\begin{aligned} & >85 \% \text { of } \\ & \text { links } \end{aligned}$ | 0 | 0 | 0\% | 0 | 0 | 0\% |
| GEH Criteria |  |  |  |  |  |  |  |  |
| GEH Statistic for individual links < 5 |  | $\begin{aligned} & >85 \% \text { of } \\ & \text { links } \end{aligned}$ | 1224 | 1053 | 86\% | 254 | 181 | 71\% |

9.3.5. Table 55 details the performance of the calibration and validation counts in the inter peak by GEH band. This analysis shows $86 \%$ of calibration counts are achieve a GEH below 5 . Relaxing the GEH criteria to 6 shows $88 \%$ of counts achieve a GEH below 6 , highlighting a number of counts have a GEH marginally outside WebTAG guidance.

Table 55 - Inter Peak - All User Classes - Calibration and Validation results

| GEH Range | Calibration |  | Validation |  | Combined |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| GEH $<2$ | 846 | $69 \%$ | 5 | $2 \%$ | 851 | $58 \%$ |
| GEH $<4$ | 999 | $82 \%$ | 164 | $65 \%$ | 1163 | $79 \%$ |
| GEH $<6$ | 1099 | $90 \%$ | 196 | $77 \%$ | 1295 | $88 \%$ |
| GEH $<8$ | 1159 | $95 \%$ | 220 | $87 \%$ | 1379 | $93 \%$ |
| GEH < 10 | 1195 | $98 \%$ | 234 | $92 \%$ | 1429 | $97 \%$ |
| GEH <5 | 1053 | $86 \%$ | 181 | $71 \%$ | 1234 | $83 \%$ |

9.3.6. The calibration and validation results for all user classes in the PM peak are shown in Table 56. This shows in terms of calibration counts, $82 \%$ achieve a GEH of 5 or less which is below the target outlined within WebTAG. Combined with calibration counts this values falls to $79 \%$ of counts with a GEH below 5 . However, for all flow criteria categories, the requirement of $85 \%$ or greater is achieved for calibration counts in the PM peak.

Table 56-PM Peak - All User Classes - Calibration and Validation results

| Criteria and Measure |  | Acceptability Guideline | Calibration |  |  | Validation |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total Counts | Meet Criteria | \% | Total Counts | Meet Criteria | \% |
| Flow Criteria |  |  |  |  |  |  |  |  |
| < 700 vph | $\pm 100 \mathrm{vph}$ |  | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 985 | 859 | 87\% | 190 | 125 | 66\% |
| $\begin{gathered} 700-2,700 \\ \text { vph } \end{gathered}$ | $\pm 15 \%$ | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 230 | 200 | 87\% | 64 | 45 | 70\% |
| > 2,700 vph | $\pm 400 \mathrm{vph}$ | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 9 | 9 | 100\% | 0 | 0 | 0\% |
| GEH Criteria |  |  |  |  |  |  |  |  |
| GEH Statistic for individual links < 5 |  | $\begin{gathered} >85 \% \text { of } \\ \text { links } \end{gathered}$ | 1224 | 1007 | 82\% | 254 | 162 | 64\% |

9.3.7. Table 57 shows the GEH performance by band for the calibration and validation counts in the PM peak. Relaxing the GEH criteria to 8 shows $90 \%$ of counts achieve a GEH below 8 , highlighting a number of counts marginally have a GEH marginally outside WebTAG requirements.

Table 57 - PM Peak - All User Classes - Calibration and Validation results

| GEH Range | Calibration |  | Validation |  | Combined |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| GEH $<2$ | 773 | $63 \%$ | 8 | $3 \%$ | 781 | $53 \%$ |
| GEH $<4$ | 949 | $78 \%$ | 142 | $56 \%$ | 1091 | $74 \%$ |
| GEH $<6$ | 1051 | $86 \%$ | 187 | $74 \%$ | 1238 | $84 \%$ |
| GEH $<8$ | 1109 | $91 \%$ | 216 | $85 \%$ | 1325 | $90 \%$ |
| GEH $<10$ | 1152 | $94 \%$ | 225 | $89 \%$ | 1377 | $93 \%$ |
| GEH $<5$ | 1007 | $82 \%$ | 162 | $64 \%$ | 1169 | $79 \%$ |

9.3.8. In summary the calibration results for individual counts show that in terms of flow criteria, the targets stipulated in WebTAG is achieved in all time periods. In terms of GEH values below 5 this falls marginally below the target of $85 \%$ stipulated in WebTAG, given the scale of the count data used with the county model it is considered this performance shows the county model still generally matches observed traffic flows well.
9.3.9. LMVRs regarding the local validation of Lowestoft and Suffolk Energy Gateway show that for these locations, the model achieves the required level of calibration stipulated within WebTAG. It is considered that once the validation of Ipswich is updated to inform the Development Consent Order modelling for The Upper Orwell Crossings, the overall county model validation will improve and reach the required level required in WebTAG.

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9.3.10. Appendix I contains details of the performance for each individual link count used in validation or calibration in terms of GEH and flow by peak hour modelled.
9.3.11. Appendix $J$ contains plots of the modelled flow in pcus for the following localities relevant to the scheme appraisals in Ipswich, Lowestoft and East Suffolk.

## JOURNEY TIME PERFORMANCE

9.3.12. Table 58 provides a summary of the number of journey time routes which have been modelled within $15 \%$ of the observed journey time across all three peaks. This summary shows the SCTM does not achieve the requirement of $85 \%$ stipulated within WebTAG in the AM and PM peak. In the inter peak, 89\% of routes are within $15 \%$ suggesting the model validates well to more free-flow conditions.

Table 58 - Journey Time Route Performance - Modelled Within 15\% of Observed

| Peak <br> Hour | Total Journey Time <br> Route | Modelled Journey Time <br> Routes within 15\% of <br> Observed | Percentage |
| :---: | :---: | :--- | :---: |
| AM | 154 | 123 | $80 \%$ |
| IP | 154 | 138 | $89 \%$ |
| PM | 154 | 125 | $81 \%$ |

9.3.13. Table 59 shows that if the criteria is relaxed to modelled journey times being with $20 \%$ of observed then nearly every route ( $97 \%$ ) in the inter peak shows a close fit between modelled and observed. This change also means $85 \%$ of routes in the AM peak and $88 \%$ of routes in the PM peak show modelled journey times within $20 \%$ of the observed. This analysis shows there are a number of routes which fall marginally outside the criteria stipulated in WebTAG for journey time validation. This gives confidence at a country wide level that the model generally represents strategic journey times well.

Table 59 - Journey Time Route Performance - Modelled Within 20\% of Observed

| Peak <br> Hour | Total Journey Time <br> Route | Modelled Journey Time <br> Routes within 20\% of <br> Observed | Percentage |
| :---: | :---: | :--- | :---: |
| AM | 154 | 131 | $85 \%$ |
| IP | 154 | 149 | $97 \%$ |
| PM | 154 | 135 | $88 \%$ |

9.3.14. Detailed journey time route coverage has been specified within the area of detailed modelling for Lowestoft and Suffolk Energy Gateway. This is detailed in the respective local LMVRs relevant to each of these locations. The journey time validation within these reports shows journey time validation of $85 \%$ or greater in terms of modelled travel time compared to observed journey time validation is achieved in all modelled time periods. For the forthcoming local validation of Ipswich a detailed series of additional journey time routes will be added.
9.3.15. Appendix D-2 provides a summary of the performance of each individual journey time route for each peak hour, comparing the overall observed and modelled journey time. Appendix D-3 also contains graphs which detail provide a comparison of the modelled journey time and observed journey time for each individual journey time route and direction by peak hour

## 10 SUMMARY OF MODEL DEVELOPMENT, STANDARDS ACHIEVED AND FITNESS FOR PURPOSE

10.1.1. This D3 Local Model Validation Report (LMVR) details the fitness for purpose of the SATURN model which forms the Suffolk County Transport Model (SCTM). This highway model will be used in conjunction with a public transport and demand model which have been developed in VISUM.
10.1.2. The SCTM highway model represents a base year of 2016, and has incorporated the networks previously developed in the Ipswich Transport Model (ITM) and Lowestoft Transport Model (LTM) which were used in 2015 for the Outline Business Cases for the Upper Orwell Crossing in Ipswich and Lake Lothing Third Crossing in Lowestoft.
10.1.3. It is proposed the SCTM highway model can be used as a stand-alone highway model from which to build forecast highway assessments, but also in conjunction with the public transport model and demand model to test the impacts of multi-modal changes and interventions. The version of the SCTM detailed within this report will be used to support the Lake Lothing Third Crossing scheme through the DCO process, as well as the Outline Business Case for the Suffolk Energy Gateway scheme. The county wide model validation will be updated and improved within Ipswich to inform the DCO process for The Upper Orwell Crossing. It will also be used to test various other proposed schemes, local authority growth strategies and developer testing.
10.1.4. The validation and calibration has referenced the latest guidelines stipulated in WebTAG as the basis for determining the fitness for purpose of the SCTM highway model.
10.1.5. An extensive data collection exercise was carried out predominantly in April 2016 in order to collect appropriate data to develop the SCTM highway model. Analysis of this data is reported in the D2 SCTM Highway Data Collection Report (December 2017). This commissioned data has been supplemented by ATC data from permanent count sites provided by Suffolk County Council, Highways England's TRADS database and the 2015 data collection carried out in Ipswich and Lowestoft for the Outline Business Cases. Traffic surveys undertaken in 2017 to inform local validation for the Suffolk Energy Gateway and Ipswich have also been utilised. The range of traffic data collected is considered appropriate and sufficient to create a strategic highway assignment. Further data collection will have to be considered going forward for any local testing of schemes and developments which need to be carried out.
10.1.6. The trip matrices used for both the highway model and public transport model have been derived predominantly from Mobile Network Data (MND) supplied by Telefonica, supplemented with a synthetic matrix derived from 2011 Census Journey to Work data. Extensive verification of the MND has been carried out, detailed in Appendix F, comparing the data against established data sources such as NTEM, 2011 Census and 2015 National Travel Survey to ensure the MND is fit for purpose to form the basis for model matrices. The methodology which has been employed to build the matrices may need to be refined following review by third parties, and related to guidance within WebTAG, with regard to matrix construction and verification of MND.
10.1.7. As part of the validation process, route choice validation has been carried out, detailed in Appendix $G$, for the stipulated number of routes defined by a formula in WebTAG. The results of the route choice validation show the SCTM highway model shows realistic minimum cost routes between the selected O-D pairs.
10.1.8. An extensive range of screenlines how been presented in this report, covering key strategic movements across Suffolk. Assignment of the prior matrix showed the requirement for matrix estimation to improve model calibration and performance against WebTAG guidelines. The final post matrix estimate assignment is shown to match well across the array of screenlines, achieving a flow difference within $5 \%$ in the majority of cases.
10.1.9. The impacts of the matrix estimation process have been discussed in Section 8 in terms of trip totals, trip length distribution and regression analysis. These results provide confidence the matrix estimate process has not fundamentally altered the prior matrix assignment.
10.1.10. The model is shown to converge satisfactorily across all three peaks. In terms of combined calibration and validation counts, the model is shown to achieve close to $85 \%$ of counts with a GEH of below 5 across all three peaks. In terms of flow criteria, the model achieves the required criteria outlined in WebTAG for calibration counts. The analysis of the breakdown of GEH values suggests there are significant number of counts with a GEH marginally outside a value of 5 .

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10.1.11. A comprehensive coverage of journey time routes have been included in the SCTM highway model. Taking into account directionality there are 154 routes which have been used to analyse journey time performance across the key strategic routes within the county. In terms of the WebTAG requirement of observed modelled journey times being within $15 \%$ of the modelled journey times, this has been achieved for $80 \%$ of routes in the AM peak, $89 \%$ in the inter peak and $81 \%$ in the PM peak. For the AM peak and PM peak this is below the $85 \%$ threshold stipulated in WebTAG, however the threshold is achieved in the inter peak period. Relaxing this criteria to $20 \%$ in terms of the difference between modelled and observed journey times shows the $85 \%$ threshold is achieved. It is considered the model generally matches travel time and peak specific congestion across key routes within the county
10.1.12. It is considered the SCTM highway model has been shown to provide a reasonable match to observed traffic count and journey time data. Local validation undertaken within Lowestoft and the area of detailed modelling for the Suffolk Energy Gateway scheme shows the required flow, GEH and journey time performance is achieved. This is reported in separate local LMVRs relevant to the scheme appraisals being undertaken for these localities. The SCTM highway model provides a robust basis from which to create forecast assignments for future scheme and development testing. It is advised for local interventions however that further refinement and validation of the SCTM highway model in the local area may be required.
10.1.13. The validation and calibration performance of the model will be improved as part of the updated modelling within Ipswich to inform the DCO process for The Upper Orwell Crossings. Generally, further refinement of the model will be required to improve its performance in relation to WebTAG requirements and any subsequent changes which are made to WebTAG guidance going forward.

## Appendix A

AREA OF DETAIL MODELLING


# Appendix A. 1 

## ZONAL DETAIL








(2)



# Appendix A. 2 

NETWORK DETAIL











Appendix B
QUEUED FLOW AT END OF TIME

PERIOD







Appendix C
A14 COUNT DATA ADJ USTMENTS
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A14 JUNCTION 52, CLAYDON
Table C-1 - A14 Junction 52, Claydon traffic count adjustments - AM peak

| Junction Movement | Direction | Data <br> Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |
| Prior to junction | EB | TRADS | 2899 | 2412 | 176 | 311 |  |  |  |  |
| Off-slip | EB | TRADS | 315 | 270 | 13 | 32 | 315 | 247 | 36 | 32 |
| On-slip | EB | MCTC | 775 | 622 | 101 | 52 |  |  |  |  |
| Prior to junction | WB | TRADS | 2694 | 2116 | 169 | 409 | 2694 | 1783 | 487 | 424 |
| Off-slip | WB | TRADS | 706 | 603 | 38 | 66 | 706 | 540 | 106 | 60 |
| On-slip | WB | MCTC | 243 | 158 | 57 | 28 |  |  |  |  |

Table C-2 - A14 Junction 52, Claydon traffic count adjustments - Inter peak

| Junction Movement | Direction | Data <br> Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |
| Prior to junction | EB | TRADS | 1664 | 1127 | 128 | 409 |  |  |  |  |
| Off-slip | EB | TRADS | 153 | 108 | 11 | 34 | 153 | 86 | 33 | 34 |
| On-slip | EB | MCTC | 540 | 381 | 94 | 65 |  |  |  |  |
| Prior to junction | WB | TRADS | 1913 | 1406 | 131 | 376 | 1913 | 1272 | 317 | 324 |
| Off-slip | WB | TRADS | 507 | 401 | 34 | 71 | 507 | 343 | 88 | 76 |
| On-slip | WB | MCTC | 155 | 92 | 33 | 30 |  |  |  |  |

Table C-3 - A14 Junction 52, Claydon traffic count adjustments - PM peak

| Junction Movement | Direction | Data <br> Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |
| Prior to junction | EB | TRADS | 2638 | 2184 | 178 | 276 |  |  |  |  |
| Off-slip | EB | TRADS | 283 | 258 | 10 | 15 | 283 | 208 | 60 | 15 |
| On-slip | EB | MCTC | 827 | 693 | 118 | 16 |  |  |  |  |
| Prior to junction | WB | TRADS | 2974 | 2606 | 103 | 265 | 2974 | 2409 | 297 | 268 |
| Off-slip | WB | TRADS | 684 | 618 | 32 | 33 | 684 | 566 | 91 | 27 |
| On-slip | WB | MCTC | 286 | 241 | 36 | 9 |  |  |  |  |

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## A14 JUNCTION 53, BURY ROAD

Table C-4 - A14 Junction 53, Bury Road traffic count adjustments - AM peak

| Junction <br> Movement | Direction | Data <br> Source | Original Survey Data |  |  |  |  | Adjusted Survey Data |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :---: |
|  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |  |  |
| Off-slip | EB | TRADS | 1023 | 932 | 49 | 42 | 1023 | 871 | 110 | 42 |  |
| Within <br> junction | EB | TRADS | 2427 | 1928 | 186 | 313 | 2336 | 1855 | 192 | 289 |  |
| On-slip | EB | MCTC | 457 | 317 | 118 | 22 |  |  |  |  |  |
| Off-slip | WB | TRADS | 514 | 463 | 33 | 19 | 454 | 391 | 56 | 7 |  |
| Within <br> junction | WB | TRADS | 1901 | 1388 | 132 | 381 | 1901 | 1148 | 353 | 400 |  |
| On-slip | WB | MCTC | 754 | 604 | 127 | 23 | 793 | 635 | 134 | 24 |  |

Table C-5 - A14 Junction 53, Bury Road traffic count adjustments - Inter peak

| Junction Movement | Direction | Data Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |
| Off-slip | EB | TRADS | 659 | 593 | 36 | 31 | 659 | 516 | 112 | 31 |
| Within junction | EB | TRADS | 1414 | 873 | 132 | 410 | 1392 | 830 | 153 | 409 |
| On-slip | EB | MCTC | 358 | 263 | 68 | 27 |  |  |  |  |
| Off-slip | WB | TRADS | 289 | 243 | 23 | 23 | 257 | 188 | 56 | 13 |
| Within junction | WB | TRADS | 1321 | 860 | 101 | 360 | 1321 | 795 | 214 | 313 |
| On-slip | WB | MCTC | 637 | 514 | 111 | 12 | 592 | 478 | 103 | 11 |

Table C-6 - A14 Junction 53, Bury Road traffic count adjustments - PM peak

| Junction <br> Movement | Direction | Data |  |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  | Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
|  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |  |
| Off-slip | EB | TRADS | 843 | 798 | 33 | 12 | 843 | 693 | 138 | 12 |
| Within <br> junction | EB | TRADS | 2166 | 1750 | 170 | 246 | 2339 | 1871 | 203 | 265 |
| On-slip | EB | MCTC | 582 | 513 | 62 | 7 |  |  |  |  |
| Off-slip | WB | TRADS | 414 | 381 | 20 | 13 | 366 | 293 | 67 | 7 |
| Within <br> junction | WB | TRADS | 2095 | 1747 | 87 | 261 | 2095 | 1621 | 217 | 256 |
| On-slip | WB | MCTC | 959 | 859 | 87 | 13 | 880 | 788 | 80 | 12 |

## A14 JUNCTION 54, SPROUGHTON ROAD

Table C-7 - A14 Junction 54, Sproughton Road traffic count adjustments - AM peak

| Junction Movement | Direction | Data Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |
| Prior to Junction | EB | TRADS | 2900 | 2319 | 233 | 348 | 2793 | 2172 | 310 | 311 |
| Off-slip | EB | TRADS | 427 | 373 | 26 | 28 | 427 | 312 | 88 | 28 |
| On-slip | EB | MCTC | 502 | 396 | 85 | 21 | 645 | 509 | 109 | 27 |
| Off-slip | WB | TRADS | 277 | 218 | 37 | 22 | 277 | 198 | 59 | 20 |
| On-slip | WB | MCTC | 291 | 205 | 67 | 19 |  |  |  |  |

Table C-8 - A14 Junction 54, Sproughton Road traffic count adjustments - Inter peak

| Junction Movement | Direction | Data <br> Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |
| Prior to Junction | EB | TRADS | 1738 | 1167 | 147 | 423 | 1750 | 1093 | 221 | 436 |
| Off-slip | EB | TRADS | 230 | 192 | 18 | 20 | 230 | 145 | 65 | 20 |
| On-slip | EB | MCTC | 210 | 143 | 51 | 16 | 280 | 191 | 68 | 21 |
| Off-slip | WB | TRADS | 202 | 148 | 29 | 25 | 202 | 127 | 56 | 20 |
| On-slip | WB | MCTC | 194 | 131 | 45 | 18 |  |  |  |  |

Table C-9 - A14 Junction 54, Sproughton Road traffic count adjustments - PM peak

| Junction Movement | Direction | Data Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |
| Prior to Junction | EB | TRADS | 2734 | 2308 | 175 | 251 | 2921 | 2384 | 265 | 272 |
| Off-slip | EB | TRADS | 377 | 353 | 15 | 10 | 377 | 308 | 59 | 10 |
| On-slip | EB | MCTC | 388 | 336 | 48 | 4 | 391 | 338 | 48 | 4 |
| Off-slip | WB | TRADS | 446 | 390 | 37 | 19 | 446 | 371 | 65 | 10 |
| On-slip | WB | MCTC | 355 | 294 | 50 | 11 |  |  |  |  |

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## A14 JUNCTION 55, COPDOCK INTERCHANGE

Table C-10 - A14 Junction 55, Copdock Interchange traffic count adjustments - AM peak

| Junction <br> Movement | Direction | Data <br> Source | Original Survey Data |  |  |  | Total | Cars | LGVs | HGVs | Total |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Prior to <br> junction | EB | TRADS | 3011 | 2358 | 279 | 374 | 3011 | 2369 | 332 | 310 |  |
| Off-slip | EB | TRADS | 1320 | 1071 | 99 | 150 | 1299 | 950 | 233 | 116 |  |
| Within <br> junction | EB | TRADS | 1792 | 1419 | 99 | 194 |  |  |  | LGVs | HGVs |
| On-slip | EB | MCTC | 1362 | 1051 | 207 | 104 | 1450 | 1119 | 220 | 111 |  |
| Off-slip | WB | TRADS | 1171 | 916 | 75 | 180 | 1171 | 835 | 202 | 134 |  |
| Within <br> junction | WB | TRADS | 1371 | 1042 | 91 | 239 | 1214 | 744 | 217 | 253 |  |
| On-slip | WB | MCTC | 977 | 683 | 159 | 135 | 1127 | 788 | 183 | 156 |  |
| After <br> junction | WB | TRADS | 2341 | 1805 | 152 | 384 | 2341 | 1532 | 401 | 408 |  |

Table C-11 - A14 Junction 55, Copdock Interchange traffic count adjustments - Inter peak

| Junction <br> Movement | Direction | Data <br> Source | Original Survey Data |  |  |  |  | Total | Cars | LGVs | HGVs |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Table C-12 - A14 Junction 55, Copdock Interchange traffic count adjustments - PM peak

| Junction Movement | Direction | Data Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |
| Prior to junction | EB | TRADS | 2934 | 2484 | 196 | 254 | 2934 | 2414 | 254 | 266 |
| Off-slip | EB | TRADS | 1477 | 1334 | 61 | 82 | 1470 | 1169 | 192 | 109 |
| Within junction | EB | TRADS | 1464 | 1245 | 62 | 157 |  |  |  |  |
| On-slip | EB | MCTC | 1340 | 1074 | 168 | 98 | 1243 | 996 | 156 | 91 |
| Off-slip | WB | TRADS | 1315 | 1139 | 51 | 125 | 1315 | 1120 | 118 | 76 |
| Within junction | WB | TRADS | 1675 | 1417 | 65 | 193 | 1480 | 1166 | 129 | 186 |
| On-slip | WB | MCTC | 956 | 736 | 152 | 68 | 1072 | 825 | 170 | 76 |
| After junction | WB | TRADS | 2552 | 2180 | 108 | 264 | 2552 | 1991 | 299 | 262 |

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## A14 JUNCTION 56, A137 WHERSTEAD

Table C-13- A14 Junction 56, A137 Wherstead traffic count adjustments - AM peak

| Junction <br> Movement | Direction | Data <br> Source | Original Survey Data |  |  |  | Total | Cars | LGVs | HGVs |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  | Total | Cars | LGVs | HGVs |  |  |  |  |  |
| Prior to <br> junction | EB | TRADS | 3162 | 2518 | 295 | 349 | 3162 | 2538 | 319 | 305 |
| Off-slip | EB | TRADS | 562 | 484 | 29 | 49 | 542 | 467 | 51 | 24 |
| Within <br> junction | EB | TRADS | 2619 | 2179 | 156 | 284 | 2619 | 2070 | 268 | 281 |
| On-slip | EB | MCTC | 906 | 835 | 54 | 17 | 882 | 813 | 53 | 17 |
| Off-slip | WB | MCTC | 676 | 594 | 68 | 14 |  |  |  |  |
| On-slip | WB | MCTC | 538 | 411 | 80 | 47 | 516 | 394 | 77 | 45 |
| After <br> junction | WB | TRADS | 2385 | 1780 | 196 | 409 | 2385 | 1579 | 419 | 387 |

Table C-14 - A14 Junction 56, A137 Wherstead traffic count adjustments - Inter peak

| Junction <br> Movement | Direction | Data <br> Source | Original Survey Data |  |  |  |  | Total | Cars | LGVs | HGVs |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Prior to <br> junction | EB | TRADS | 1891 | 1232 | 172 | 487 | 1891 | 1237 | 215 | 439 |  |
| Off-slip | EB | TRADS | 320 | 263 | 20 | 37 | 307 | 221 | 53 | 33 |  |
| Within <br> junction | EB | TRADS | 1584 | 1041 | 99 | 444 | 1584 | 1016 | 162 | 406 |  |
| On-slip | EB | MCTC | 386 | 301 | 61 | 24 | 359 | 280 | 57 | 22 |  |
| Off-slip | WB | MCTC | 338 | 260 | 57 | 21 |  |  |  | Cars |  |
| On-slip | WB | MCTC | 348 | 245 | 56 | 47 | 298 | 210 | 48 | 40 |  |
| After <br> junction | WB | TRADS | 1720 | 1202 | 145 | 373 | 1720 | 1130 | 273 | 317 |  |

Table C-15- A14 Junction 56, A137 Wherstead traffic count adjustments - PM peak

| Junction <br> Movement | Direction | Data <br> Source | Original Survey Data |  |  |  |  | Adjusted Survey Data |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :---: |
|  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |  |  |
| Prior to <br> junction | EB | TRADS | 2707 | 2210 | 214 | 284 | 2707 | 2241 | 218 | 248 |  |
| Off-slip | EB | TRADS | 509 | 465 | 25 | 19 | 332 | 263 | 63 | 6 |  |
| Within <br> junction | EB | TRADS | 2376 | 1936 | 116 | 323 | 2376 | 1978 | 155 | 242 |  |
| On-slip | EB | MCTC | 785 | 708 | 66 | 11 | 634 | 572 | 53 | 9 |  |
| Off-slip | WB | MCTC | 815 | 707 | 94 | 14 |  |  |  |  |  |


| On-slip | WB | MCTC | 619 | 544 | 64 | 11 | 444 | 390 | 46 | 8 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| After <br> junction | WB | TRADS | 2795 | 2371 | 138 | 286 | 2795 | 2286 | 247 | 262 |

A14 JUNCTION 57, A1189 NACTON ROAD
Table C-16 - A14 Junction 57, A1 189 Nacton Road traffic count adjustments - AM peak

| Junction <br> Movement | Direction | Data <br> Source | Original Survey Data |  |  |  | Total | Cars | LGVs | HGVs | Total |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Prior to <br> junction | EB | TRADS | 3501 | 2951 | 228 | 322 | 3501 | 2883 | 321 | 297 |  |
| Off-slip | EB | TRADS | 1549 | 1323 | 167 | 59 |  | LGVs | HGVs |  |  |
| Within <br> junction | EB | TRADS | 1981 | 1602 | 133 | 247 | 1952 | 1560 | 154 | 238 |  |
| On-slip | EB | MCTC | 350 | 208 | 82 | 59 | 164 | 98 | 39 | 28 |  |
| Off-slip | WB | TRADS | 512 | 469 | 11 | 32 | 512 | 450 | 31 | 30 |  |
| Within <br> junction | WB | TRADS | 1964 | 1557 | 97 | 310 | 1806 | 1273 | 235 | 298 |  |
| On-slip | WB | MCTC | 791 | 541 | 188 | 62 | 740 | 506 | 176 | 58 |  |

Table C-17- A14 Junction 57, A1189 Nacton Road traffic count adjustments - Inter peak

| Junction Movement | Direction | Data Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |
| Prior to junction | EB | TRADS | 1943 | 1345 | 135 | 462 | 1943 | 1295 | 219 | 429 |
| Off-slip | EB | TRADS | 749 | 536 | 133 | 80 |  |  |  |  |
| Within junction | EB | TRADS | 1194 | 752 | 70 | 371 | 1194 | 760 | 86 | 348 |
| On-slip | EB | MCTC | 350 | 208 | 82 | 59 | 314 | 187 | 74 | 53 |
| Off-slip | WB | TRADS | 278 | 230 | 15 | 33 | 278 | 187 | 51 | 40 |
| Within junction | WB | TRADS | 1317 | 937 | 88 | 292 | 1281 | 818 | 199 | 263 |
| On-slip | WB | MCTC | 745 | 562 | 129 | 54 | 479 | 362 | 83 | 35 |

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Table C-18 - A14 Junction 57, A1189 Nacton Road traffic count adjustments - PM peak

| Junction Movement | Direction | Data <br> Source | Original Survey Data |  |  |  | Adjusted Survey Data |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Total | Cars | LGVs | HGVs | Total | Cars | LGVs | HGVs |
| Prior to junction | EB | TRADS | 3010 | 2578 | 144 | 288 | 3010 | 2550 | 209 | 251 |
| Off-slip | EB | TRADS | 897 | 710 | 143 | 44 |  |  |  |  |
| Within junction | EB | TRADS | 2092 | 1740 | 90 | 261 | 2113 | 1840 | 66 | 206 |
| On-slip | EB | MCTC | 350 | 208 | 82 | 59 | 553 | 329 | 130 | 94 |
| Off-slip | WB | TRADS | 197 | 166 | 7 | 24 | 197 | 124 | 14 | 59 |
| Within junction | WB | TRADS | 2071 | 1736 | 84 | 250 | 1841 | 1141 | 187 | 243 |
| On-slip | WB | MCTC | 1523 | 1369 | 124 | 29 | 1325 | 1192 | 108 | 25 |

## A14 JUNCTION 58, A12 / A1156 FELIXSTOWE ROAD

Table C-19 - A14 Junction 58, A12 / Felixstowe Road traffic count adjustments - AM peak

| Junction <br> Movement | Direction | Data <br> Source | Original Survey Data |  |  |  | Total | Cars | LGVs | HGVs | Total |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  | Cars | LGVs | HGVs |  |  |  |  |  |  |  |
| Prior to <br> junction | EB | TRADS | 2146 | 1673 | 195 | 278 | 2116 | 1658 | 193 | 266 |  |
| Off-slip | EB | TRADS | 1364 | 1154 | 88 | 122 |  |  |  |  |  |
| On-slip | EB | MCTC | 588 | 512 | 64 | 12 |  |  |  |  |  |
| Prior to <br> junction | WB | TRADS | 1661 | 1219 | 142 | 300 |  |  |  |  |  |
| Off-slip | WB | MCTC | 566 | 484 | 60 | 22 | 533 | 456 | 56 | 21 |  |
| Within <br> junction | WB | TRADS | 1047 | 764 | 35 | 247 | 1128 | 763 | 86 | 279 |  |
| On-slip | WB | MCTC | 1189 | 979 | 160 | 50 |  |  |  |  |  |
| After <br> junction | WB | TRADS | 2491 | 1949 | 185 | 357 | 2317 | 1742 | 246 | 329 |  |

Table C-20 - A14 Junction 58, A12 / Felixstowe Road traffic count adjustments - Inter peak

| Junction <br> Movement | Direction | Data <br> Source | Original Survey Data |  |  |  |  | Total | Cars | LGVs | HGVs |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  | Total | Cars | LGVs | HGVs |  |  |  |  |  |  |
| Prior to <br> junction | EB | TRADS | 1507 | 984 | 114 | 409 | 1507 | 946 | 159 | 402 |  |
| Off-slip | EB | TRADS | 795 | 645 | 56 | 93 |  |  |  |  |  |
| On-slip | EB | MCTC | 381 | 319 | 47 | 15 |  |  |  |  |  |
| Prior to <br> junction | WB | TRADS | 1098 | 752 | 112 | 234 |  |  |  |  |  |
| Off-slip | WB | MCTC | 449 | 376 | 58 | 15 | 434 | 363 | 56 | 14 |  |
| Within <br> junction | WB | TRADS | 654 | 390 | 42 | 222 | 664 | 389 | 56 | 220 |  |
| On-slip | WB | MCTC | 894 | 659 | 158 | 77 |  |  |  |  |  |
| After <br> junction | WB | TRADS | 1547 | 1049 | 156 | 342 | 1558 | 1048 | 214 | 297 |  |

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Table C-21 - A14 Junction 58, A12 / Felixstowe Road traffic count adjustments - PM peak

| Junction <br> Movement | Direction | Data <br> Source | Original Survey Data |  |  |  |  | Total | Cars | LGVs | HGVs |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  | Total | Cars | LGVs | HGVs |  |  |  |  |  |  |
| Prior to <br> junction | EB | TRADS | 2645 | 2271 | 126 | 247 | 2666 | 2169 | 196 | 300 |  |
| Off-slip | EB | TRADS | 1546 | 1452 | 55 | 39 |  |  |  |  |  |
| On-slip | EB | MCTC | 571 | 516 | 53 | 2 |  |  |  |  |  |
| Prior to <br> junction | WB | TRADS | 1631 | 1272 | 131 | 228 |  |  |  |  |  |
| Off-slip | WB | MCTC | 631 | 571 | 57 | 3 | 666 | 602 | 60 | 3 |  |
| Within <br> junction | WB | TRADS | 985 | 718 | 36 | 231 | 966 | 670 | 71 | 225 |  |
| On-slip | WB | MCTC | 1073 | 908 | 123 | 42 |  |  |  |  |  |
| After <br> junction | WB | TRADS | 2344 | 1891 | 162 | 291 | 2039 | 1578 | 194 | 267 |  |

# Appendix D 

## J OURNEY TIME ROUTES

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# Appendix D. 1 

J OURNEY TIME ROUTE

DESCRIPTIONS

Table D-1 Journey Time Routes

| Route | Length (km) | Road | Description |
| :---: | :---: | :---: | :---: |
| 1 | 11.4 | A14 | J36-J38 through Newmarket |
| 2 | 12.6 | A14 | J38-J41 |
| 3 | 14.4 | A14 | J41-J46 through Bury St Edmunds |
| 4 | 11.0 | A14 | J46-J49 |
| 5 | 9.8 | A14 | J49-J51 through Stowmarket |
| 6 | 9.6 | A14 | J51-J54 to lpswich |
| 7 | 10.9 | A14 | J54-J57 through Ipswich |
| 8 | 13.6 | A14 | J57- J62 through Felixstowe |
| 9 | 13.8 | A12 | Dedham to Ipswich (J55) |
| 10 | 11.9 | A12 | J58 to Woodbridge |
| 11 | 18.3 | A12 | Woodbridge to Farnham |
| 12 | 9.4 | A12 | Farnham to Darsham |
| 13 | 10.5 | A12 | Darsham to Wangford |
| 14 | 14.1 | A12 | Wangford to Pakefield |
| 15 | 12.0 | A12 | Pakefield to Blundeston (through Lowestoft) |
| 16 | 10.6 | A134 | Middleton to Long Melford |
| 17 | 9.3 | A134 | Long Melford to Stanningfield |
| 18 | 8.2 | A134 | Stanningfield to Bury St Edmunds |
| 19 | 10.0 | A134 | Bury St Edmunds to Airfield |
| 20 | 6.2 | A134 | Airfield to Thetford |
| 21 | 10.7 | A143 | Bury St Edmunds to Ixworth |
| 22 | 11.8 | A143 | Ixworth to Rickinghall |
| 23 | 13.6 | A143 | Rickinghall to Scole |
| 24 | 10.4 | A11 | Kennett to Mildenhall |
| 26 | 13.9 | A1065 | Mildenhall to Lakenheath |
| 27 | 14.2 | A140 | Coddenham to Wickham Skeith |
| 28 | 11.5 | A140 | Wickham Skeith to Scole |
| 29 | 12.6 | A1101 | Bury St Edmunds to Icklingham |
| 30 | 12.0 | A1101 | Icklingham to Beck Row |
| 31 | 10.4 | A1101 | Beck Row to Burnt Fen |
| 32 | 12.3 | A143 | Haverhill to Stradishall |
| 33 | 8.8 | A143 | Stradishall to Chevington |
| 34 | 7.6 | A143 | Chevington to Bury St Edmunds |
| 35 | 13.2 | A134 | Great Conard to Nayland |
| 36 | 11.1 | A1071 | Newton to Hadleigh |

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Project No.: 70016133 | Our Ref No.: 70016133 Suffolk County Council

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| Route | Length (km) | Road | Description |
| :--- | :--- | :--- | :--- |
| 37 | 11.5 | A1071 | Hadleigh to Ipswich |
| 38 | 14.1 | A1141 | Hadleigh to Lavenham |
| 39 | 8.1 | A1141 | Lavenham to Stanningfield |
| 40 | 12.1 | A137 | Brantham to Ipswich |
| 41 | 13.0 | A145 | Weston to Blythburgh |
| 42 | 13.4 | A144 | Darsham to St Lawrence |
| 43 | 9.3 | A144 | St Lawrence to Ditchingham |
| 44 | 10.2 | A1094 | Farnham to Aldeburgh |
| 45 | 12.9 | A1120 | Stowupland to Peats Corner |
| 46 | 11.7 | A1120 | Peats Corner to Dennington |
| 47 | 12.9 | A1120 | Dennington to Yoxford |
| 48 | 15.1 | A1088 | Thretford to Ixworth |
| 49 | 8.7 | A1088 | Ixworth to Woolpit |
| 50 | 9.0 | A1303 | Kennett to Newmarket Heath |
| 51 | 10.1 | A1092 | Wixoe to Cavendish |
| 52 | 6.8 | A1092 | Cavendish to Stanstead |
| 53 | 5.5 | A1307 | through Haverhill |
| 54 | 5.6 | A1017 | through Haverhill |
| 55 | 5.7 | A134 | through Sudbury |
| 56 | 5.9 | A154 | through Felixstowe |
| 57 | 7.1 | A1308 | through Stowmarket |
| 58 | 5.5 | A1302 | through Bury St Edmunds |
| 59 | 7.4 | A145 | through Beccles |
| 60 | 9.0 | A1145 | through Lowestoft |
| 61 | 13.5 | A1214 | through Ipswich |
| 62 | 11.8 | A1156 | through Ipswich |
| 63 | 7.9 | A1214 | through Ipswich |
| 64 | 7.4 | A145 | Beccles to Lowestoft |
| 101 | 4.5 | B1375 | B1375 Gorleston Road |
| 102 | 6.1 | A12 | A12 Yarmouth Road / Katwijk Way |
| 103 | 3.2 | A1117 / A1144 | A11177 Normanston Drive / A1144 St Peter's |
| Street |  |  |  |
| 104 | 6.8 | A12 / B1532 | A12 London Road / B1532 London Road South |
| 105 | 3.6 | B1074 / A1117 | B1074 / A1117 Millennium Way / Oulton Road |
| 106 | 9.5 | A146 | A146 Beccles Road / A146 Waveney Drive |
| 200 | 7.3 | B1438 | Yarmouth Road / Melton Road (Woodbridge) |
| 201 | 13.5 | B1116 / B1078 / B1438 |  |
|  |  |  |  |


| Route | Length $(\mathrm{km})$ | Road | Description |
| :--- | :--- | :--- | :--- |
| 202 | 9.9 | B1078 | B1078 lpswich Road |
| 203 | 3.7 | B1078 | B1078 Ash Road |
| 204 | 11.5 | A1152 | A1152 |
| 205 | 6.0 | B1069 | B1069 (South of A1094) |
| 206 | 4.3 | B1069 | B1069 Snape Road (North of A1094) |
| 207 | 7.6 | B1119 | B1119 Saxmundham Road |

# Appendix D. 2 

J OURNEY TIME ROUTE

PERFORMANCE SUMMARY

## AM Peak

| ID | Name |
| :---: | :---: |
| 1 | 1-EB |
| 2 | 1-WB |
| 3 | 2-EB |
| 4 | 2-WB |
| 5 | 3-EB |
| 6 | 3-WB |
| 7 | 4-EB |
| 8 | 4-WB |
| 9 | 5-EB |
| 10 | 5-WB |
| 11 | 6-EB |
| 12 | 6-WB |
| 13 | 7-EB |
| 14 | 7-WB |
| 15 | 8-EB |
| 16 | 8-WB |
| 17 | 9-NB |
| 18 | 9-SB |
| 19 | 10-NB |
| 20 | 10-SB |
| 21 | 11-EB |
| 22 | 11-WB |
| 23 | 12-NB |
| 24 | 12-SB |
| 25 | 13-NB |
| 26 | 13-SB |
| 27 | 14-NB |
| 28 | 14-SB |
| 29 | 15-NB |
| 30 | 15-SB |
| 31 | 16-NB |
| 32 | 16-SB |
| 33 | 17-NB |
| 34 | 17-SB |
| 35 | 18-NB |
| 36 | 18-SB |
| 37 | 19-NB |
| 38 | 19-SB |
| 39 | 20-NB |
| 40 | 20-SB |
| 41 | 21-EB |
| 42 | 21-WB |
| 43 | 22-EB |
| 44 | 22-WB |
| 45 | 23-EB |
| 46 | 23-WB |
| 47 | 24-NB |
| 48 | 24-SB |
| 51 | 26-NB |
| 52 | 26-SB |
| 53 | 27-NB |
| 54 | 27-SB |
| 55 | 28-NB |
| 56 | 28-SB |


| Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: |
| 380 | 392 | 12 | 3\% | Yes |
| 513 | 539 | 26 | 5\% | Yes |
| 444 | 462 | 18 | 4\% | Yes |
| 461 | 562 | 101 | 22\% | No |
| 505 | 523 | 18 | 4\% | Yes |
| 606 | 743 | 137 | 23\% | No |
| 390 | 398 | 8 | 2\% | Yes |
| 418 | 492 | 74 | 18\% | No |
| 385 | 376 | -9 | -2\% | Yes |
| 425 | 392 | -33 | -8\% | Yes |
| 380 | 511 | 131 | 35\% | No |
| 376 | 476 | 100 | 27\% | No |
| 481 | 674 | 193 | 40\% | No |
| 477 | 524 | 47 | 10\% | Yes |
| 534 | 504 | -30 | -6\% | Yes |
| 520 | 534 | 14 | 3\% | Yes |
| 963 | 571 | -392 | -41\% | No |
| 549 | 495 | -54 | -10\% | Yes |
| 637 | 624 | -13 | -2\% | Yes |
| 800 | 693 | -107 | -13\% | Yes |
| 841 | 897 | 56 | 7\% | Yes |
| 888 | 975 | 87 | 10\% | Yes |
| 504 | 514 | 10 | 2\% | Yes |
| 500 | 514 | 14 | 3\% | Yes |
| 534 | 494 | -40 | -8\% | Yes |
| 536 | 494 | -42 | -8\% | Yes |
| 766 | 760 | -6 | -1\% | Yes |
| 782 | 751 | -31 | -4\% | Yes |
| 544 | 538 | -6 | -1\% | Yes |
| 479 | 479 | 0 | 0\% | Yes |
| 725 | 659 | -66 | -9\% | Yes |
| 857 | 1134 | 277 | 32\% | No |
| 523 | 497 | -26 | -5\% | Yes |
| 489 | 490 | 1 | 0\% | Yes |
| 825 | 519 | -306 | -37\% | No |
| 501 | 490 | -11 | -2\% | Yes |
| 525 | 543 | 18 | 3\% | Yes |
| 546 | 523 | -23 | -4\% | Yes |
| 353 | 332 | -21 | -6\% | Yes |
| 363 | 364 | 1 | 0\% | Yes |
| 623 | 638 | 15 | 2\% | Yes |
| 743 | 778 | 35 | 5\% | Yes |
| 630 | 603 | -27 | -4\% | Yes |
| 720 | 693 | -27 | -4\% | Yes |
| 686 | 647 | -39 | -6\% | Yes |
| 704 | 643 | -61 | -9\% | Yes |
| 381 | 505 | 124 | 33\% | No |
| 440 | 567 | 127 | 29\% | No |
| 759 | 757 | -2 | 0\% | Yes |
| 787 | 819 | 32 | 4\% | Yes |
| 800 | 736 | -64 | -8\% | Yes |
| 871 | 780 | -91 | -10\% | Yes |
| 637 | 585 | -52 | -8\% | Yes |
| 614.459274 | 622 | 8 | 1\% | Yes |

## AM Peak

| ID | Name |
| :---: | :---: |
| 57 | 29-NB |
| 58 | 29-SB |
| 59 | 30-EB |
| 60 | 30-WB |
| 61 | 31-NB |
| 62 | 31-SB |
| 63 | 32-EB |
| 64 | 32-WB |
| 65 | 33-NB |
| 66 | 33-SB |
| 67 | 34-EB |
| 68 | 34-WB |
| 69 | 35-NB |
| 70 | 35-SB |
| 71 | 36-EB |
| 72 | 36-WB |
| 73 | 37-EB |
| 74 | 37-WB |
| 75 | 38-SB |
| 76 | 38-NB |
| 77 | 39-NB |
| 78 | 39-SB |
| 79 | 40-NB |
| 80 | 40-SB |
| 81 | 41-NB |
| 82 | 41-SB |
| 83 | 42-NB |
| 84 | 42-SB |
| 85 | 43-NB |
| 86 | 43-SB |
| 87 | 44-EB |
| 88 | 44-WB |
| 89 | 45-EB |
| 90 | 45-WB |
| 91 | 46-EB |
| 92 | 46-WB |
| 93 | 47-EB |
| 94 | 47-WB |
| 95 | 48-NB |
| 96 | 48-SB |
| 97 | 49-NB |
| 98 | 49-SB |
| 99 | 50-EB |
| 100 | 50-WB |
| 101 | 51-EB |
| 102 | 51-WB |
| 103 | 52-EB |
| 104 | 52-WB |
| 105 | 53-EB |
| 106 | 53-WB |
| 107 | 54-EB |
| 108 | 54-WB |
| 109 | 55-NB |
| 110 | 55-SB |


| Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: |
| 857 | 772 | -85 | -10\% | Yes |
| 953 | 755 | -198 | -21\% | No |
| 946 | 809 | -137 | -15\% | Yes |
| 933 | 764 | -169 | -18\% | No |
| 508 | 494 | -14 | -3\% | Yes |
| 523 | 498 | -25 | -5\% | Yes |
| 689 | 619 | -70 | -10\% | Yes |
| 684 | 636 | -48 | -7\% | Yes |
| 469 | 398 | -71 | -15\% | Yes |
| 459 | 408 | -51 | -11\% | Yes |
| 655 | 586 | -70 | -11\% | Yes |
| 542 | 537 | -5 | -1\% | Yes |
| 801 | 822 | 21 | 3\% | Yes |
| 775 | 837 | 62 | 8\% | Yes |
| 567 | 638 | 71 | 13\% | Yes |
| 638 | 648 | 10 | 2\% | Yes |
| 1167 | 794 | -373 | -32\% | No |
| 734 | 720 | -14 | -2\% | Yes |
| 875 | 818 | -57 | -6\% | Yes |
| 940 | 821 | -119 | -13\% | Yes |
| 577 | 709 | 132 | 23\% | No |
| 481 | 470 | -11 | -2\% | Yes |
| 1638 | 926 | -712 | -43\% | No |
| 987 | 895 | -92 | -9\% | Yes |
| 689 | 671 | -18 | -3\% | Yes |
| 694 | 690 | -4 | -1\% | Yes |
| 922.025737 | 781 | -141 | -15\% | Yes |
| 902 | 776 | -126 | -14\% | Yes |
| 613 | 533 | -80 | -13\% | Yes |
| 616 | 538 | -78 | -13\% | Yes |
| 559 | 494 | -65 | -12\% | Yes |
| 540 | 502 | -38 | -7\% | Yes |
| 813 | 772 | -41 | -5\% | Yes |
| 820 | 779 | -41 | -5\% | Yes |
| 736 | 558 | -178 | -24\% | No |
| 690 | 557 | -133 | -19\% | No |
| 779 | 797 | 18 | 2\% | Yes |
| 795 | 791 | -4 | 0\% | Yes |
| 779 | 687 | -92 | -12\% | Yes |
| 798 | 702 | -96 | -12\% | Yes |
| 533 | 478 | -55 | -10\% | Yes |
| 521 | 485 | -36 | -7\% | Yes |
| 628 | 543 | -85 | -14\% | Yes |
| 832 | 691 | -141 | -17\% | No |
| 728 | 649 | -79 | -11\% | Yes |
| 732 | 691 | -41 | -6\% | Yes |
| 424 | 462 | 38 | 9\% | Yes |
| 431 | 466 | 35 | 8\% | Yes |
| 524 | 508 | -16 | -3\% | Yes |
| 509 | 506 | -3 | -1\% | Yes |
| 297 | 292 | -5 | -2\% | Yes |
| 311 | 308 | -3 | -1\% | Yes |
| 532 | 574 | 42 | 8\% | Yes |
| 498 | 405 | -93 | -19\% | No |

## AM Peak

| ID | Name |
| :---: | :---: |
| 111 | 56-NB |
| 112 | 56-SB |
| 113 | 57-EB |
| 114 | 57-WB |
| 115 | 58-NB |
| 116 | 58-SB |
| 117 | 59-EB |
| 118 | 59-WB |
| 119 | 60-NB |
| 120 | 60-SB |
| 121 | 61-EB |
| 122 | 61-WB |
| 123 | 62-EB |
| 124 | 62-WB |
| 125 | 63-EB |
| 126 | 63-WB |
| 127 | 64-EB |
| 128 | 64-WB |
| 129 | 101-NB |
| 130 | 101-SB |
| 131 | 102-NB |
| 132 | 102-SB |
| 133 | 103-EB |
| 134 | 103-WB |
| 135 | 104-NB |
| 136 | 104-SB |
| 137 | 105-EB |
| 138 | 105-WB |
| 139 | 106-EB |
| 140 | 106-WB |
| 141 | 200-NB |
| 142 | 200-SB |
| 143 | 201-NB |
| 144 | 201-SB |
| 145 | 202-EB |
| 146 | 202-WB |
| 147 | 203-EB |
| 148 | 203-WB |
| 149 | 204-EB |
| 150 | 204-WB |
| 151 | 205-NB |
| 152 | 205-SB |
| 153 | 206-NB |
| 154 | 206-SB |
| 155 | 207-EB |
| 156 | 207-WB |


| Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: |
| 601 | 559 | -42 | -7\% | Yes |
| 665 | 535 | -130 | -20\% | No |
| 642 | 565 | -77 | -12\% | Yes |
| 714 | 553 | -161 | -23\% | No |
| 694 | 602 | -92 | -13\% | Yes |
| 853 | 594 | -259 | -30\% | No |
| 600 | 593 | -7 | -1\% | Yes |
| 670 | 650 | -20 | -3\% | Yes |
| 1025 | 894 | -131 | -13\% | Yes |
| 956 | 889 | -67 | -7\% | Yes |
| 2250 | 1849 | -401 | -18\% | No |
| 2359 | 1726 | -633 | -27\% | No |
| 2321 | 1411 | -910 | -39\% | No |
| 2081 | 1592 | -489 | -23\% | No |
| 1336 | 762 | -574 | -43\% | No |
| 1502 | 826 | -676 | -45\% | No |
| 316 | 306 | -10 | -3\% | Yes |
| 311 | 317 | 6 | 2\% | Yes |
| 356 | 412 | 56 | 16\% | Yes |
| 412 | 361 | -51 | -12\% | Yes |
| 701 | 662 | -39 | -6\% | Yes |
| 753 | 668 | -85 | -11\% | Yes |
| 383 | 375 | -8 | -2\% | Yes |
| 372 | 428 | 56 | 15\% | Yes |
| 543 | 518 | -25 | -5\% | Yes |
| 390 | 401 | 11 | 3\% | Yes |
| 505 | 462 | -43 | -8\% | Yes |
| 447 | 365 | -82 | -18\% | No |
| 620 | 480 | -140 | -23\% | No |
| 515 | 502 | -13 | -3\% | Yes |
| 702 | 635 | -67 | -10\% | Yes |
| 799 | 682 | -117 | -15\% | Yes |
| 1041 | 890 | -151 | -14\% | Yes |
| 1046 | 902 | -144 | -14\% | Yes |
| 590 | 563 | -27 | -5\% | Yes |
| 588 | 574 | -14 | -2\% | Yes |
| 280 | 274 | -6 | -2\% | Yes |
| 292 | 276 | -16 | -5\% | Yes |
| 753 | 763 | 10 | 1\% | Yes |
| 865 | 908 | 43 | 5\% | Yes |
| 392 | 383 | -8 | -2\% | Yes |
| 364 | 395 | 31 | 8\% | Yes |
| 327 | 374 | 47 | 14\% | Yes |
| 314 | 345 | 31 | 10\% | Yes |
| 617 | 620 | 3 | 1\% | Yes |
| 596 | 596 | 0 | 0\% | Yes |

## Interpeak

| ID | Name |
| :---: | :---: |
| 1 | 1-EB |
| 2 | 1-WB |
| 3 | 2-EB |
| 4 | 2-WB |
| 5 | 3-EB |
| 6 | 3-WB |
| 7 | 4-EB |
| 8 | 4-WB |
| 9 | 5-EB |
| 10 | 5-WB |
| 11 | 6-EB |
| 12 | 6-WB |
| 13 | 7-EB |
| 14 | 7-WB |
| 15 | 8-EB |
| 16 | 8-WB |
| 17 | 9-NB |
| 18 | 9-SB |
| 19 | 10-NB |
| 20 | 10-SB |
| 21 | 11-EB |
| 22 | 11-WB |
| 23 | 12-NB |
| 24 | 12-SB |
| 25 | 13-NB |
| 26 | 13-SB |
| 27 | 14-NB |
| 28 | 14-SB |
| 29 | 15-NB |
| 30 | 15-SB |
| 31 | 16-NB |
| 32 | 16-SB |
| 33 | 17-NB |
| 34 | 17-SB |
| 35 | 18-NB |
| 36 | 18-SB |
| 37 | 19-NB |
| 38 | 19-SB |
| 39 | 20-NB |
| 40 | 20-SB |
| 41 | 21-EB |
| 42 | 21-WB |
| 43 | 22-EB |
| 44 | 22-WB |
| 45 | 23-EB |
| 46 | 23-WB |
| 47 | 24-NB |
| 48 | 24-SB |
| 51 | 26-NB |
| 52 | 26-SB |
| 53 | 27-NB |
| 54 | 27-SB |
| 55 | 28-NB |
| 56 | 28-SB |


| Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: |
| 391 | 400 | 9 | 2\% | Yes |
| 395 | 406 | 11 | 3\% | Yes |
| 442 | 460 | 18 | 4\% | Yes |
| 433 | 456 | 23 | 5\% | Yes |
| 525 | 517 | -8 | -2\% | Yes |
| 516 | 513 | -3 | -1\% | Yes |
| 391 | 391 | 0 | 0\% | Yes |
| 388 | 387 | -1 | 0\% | Yes |
| 378 | 349 | -29 | -8\% | Yes |
| 381 | 327 | -54 | -14\% | Yes |
| 352 | 398 | 46 | 13\% | Yes |
| 351 | 394 | 43 | 12\% | Yes |
| 413 | 452 | 39 | 9\% | Yes |
| 407 | 431 | 24 | 6\% | Yes |
| 527 | 495 | -32 | -6\% | Yes |
| 520 | 495 | -25 | -5\% | Yes |
| 555 | 513 | -42 | -7\% | Yes |
| 509 | 489 | -20 | -4\% | Yes |
| 623 | 589 | -34 | -6\% | Yes |
| 639 | 554 | -85 | -13\% | Yes |
| 845 | 876 | 31 | 4\% | Yes |
| 879 | 886 | 7 | 1\% | Yes |
| 507 | 502 | -5 | -1\% | Yes |
| 515 | 498 | -17 | -3\% | Yes |
| 547 | 485 | -62 | -11\% | Yes |
| 551 | 484 | -67 | -12\% | Yes |
| 814 | 753 | -61 | -7\% | Yes |
| 787 | 746 | -41 | -5\% | Yes |
| 560 | 622 | 62 | 11\% | Yes |
| 514 | 438 | -76 | -15\% | Yes |
| 725 | 639 | -86 | -12\% | Yes |
| 861 | 756 | -105 | -12\% | Yes |
| 523 | 456 | -67 | -13\% | Yes |
| 498 | 468 | -30 | -6\% | Yes |
| 527 | 461 | -66 | -12\% | Yes |
| 513 | 478 | -35 | -7\% | Yes |
| 518 | 500 | -18 | -3\% | Yes |
| 522 | 492 | -30 | -6\% | Yes |
| 348 | 325 | -23 | -7\% | Yes |
| 330 | 339 | 9 | 3\% | Yes |
| 616 | 630 | 14 | 2\% | Yes |
| 630 | 624 | -6 | -1\% | Yes |
| 633 | 599 | -34 | -5\% | Yes |
| 645 | 619 | -26 | -4\% | Yes |
| 681 | 631 | -50 | -7\% | Yes |
| 687 | 631 | -56 | -8\% | Yes |
| 389 | 517 | 128 | 33\% | No |
| 386 | 509 | 123 | 32\% | No |
| 775 | 762 | -13 | -2\% | Yes |
| 767 | 769 | 2 | 0\% | Yes |
| 785 | 698 | -87 | -11\% | Yes |
| 802 | 698 | -104 | -13\% | Yes |
| 603 | 554 | -49 | -8\% | Yes |
| 594 | 560 | -34 | -6\% | Yes |

## Interpeak

| ID | Name |
| :---: | :---: |
| 57 | 29-NB |
| 58 | 29-SB |
| 59 | 30-EB |
| 60 | 30-WB |
| 61 | 31-NB |
| 62 | 31-SB |
| 63 | 32-EB |
| 64 | 32-WB |
| 65 | 33-NB |
| 66 | 33-SB |
| 67 | 34-EB |
| 68 | 34-WB |
| 69 | 35-NB |
| 70 | 35-SB |
| 71 | 36-EB |
| 72 | 36-WB |
| 73 | 37-EB |
| 74 | 37-WB |
| 75 | 38-SB |
| 76 | 38-NB |
| 77 | 39-NB |
| 78 | 39-SB |
| 79 | 40-NB |
| 80 | 40-SB |
| 81 | 41-NB |
| 82 | 41-SB |
| 83 | 42-NB |
| 84 | 42-SB |
| 85 | 43-NB |
| 86 | 43-SB |
| 87 | 44-EB |
| 88 | 44-WB |
| 89 | 45-EB |
| 90 | 45-WB |
| 91 | 46-EB |
| 92 | 46-WB |
| 93 | 47-EB |
| 94 | 47-WB |
| 95 | 48-NB |
| 96 | 48-SB |
| 97 | 49-NB |
| 98 | 49-SB |
| 99 | 50-EB |
| 100 | 50-WB |
| 101 | 51-EB |
| 102 | 51-WB |
| 103 | 52-EB |
| 104 | 52-WB |
| 105 | 53-EB |
| 106 | 53-WB |
| 107 | 54-EB |
| 108 | 54-WB |
| 109 | 55-NB |
| 110 | 55-SB |


| Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: |
| 838 | 745 | -93 | -11\% | Yes |
| 855 | 743 | -112 | -13\% | Yes |
| 957 | 774 | -183 | -19\% | No |
| 862 | 742 | -120 | -14\% | Yes |
| 569 | 494 | -75 | -13\% | Yes |
| 528 | 494 | -34 | -6\% | Yes |
| 657 | 605 | -52 | -8\% | Yes |
| 660 | 604 | -56 | -8\% | Yes |
| 473 | 383 | -90 | -19\% | No |
| 468 | 384 | -84 | -18\% | No |
| 565 | 546 | -19 | -3\% | Yes |
| 539 | 522 | -17 | -3\% | Yes |
| 791 | 750 | -41 | -5\% | Yes |
| 778 | 743 | -35 | -5\% | Yes |
| 573 | 632 | 59 | 10\% | Yes |
| 597 | 634 | 37 | 6\% | Yes |
| 699 | 745 | 46 | 7\% | Yes |
| 735 | 702 | -33 | -5\% | Yes |
| 946 | 816 | -130 | -14\% | Yes |
| 939 | 805 | -134 | -14\% | Yes |
| 550 | 479 | -71 | -13\% | Yes |
| 536 | 470 | -66 | -12\% | Yes |
| 943 | 860 | -83 | -9\% | Yes |
| 911 | 843 | -68 | -7\% | Yes |
| 710 | 675 | -35 | -5\% | Yes |
| 718 | 683 | -35 | -5\% | Yes |
| 880 | 768 | -112 | -13\% | Yes |
| 898 | 772 | -126 | -14\% | Yes |
| 624 | 522 | -102 | -16\% | No |
| 634 | 527 | -107 | -17\% | No |
| 582 | 499 | -83 | -14\% | Yes |
| 587 | 498 | -89 | -15\% | Yes |
| 810 | 748 | -62 | -8\% | Yes |
| 825 | 748 | -77 | -9\% | Yes |
| 721 | 551 | -170 | -24\% | No |
| 710 | 552 | -158 | -22\% | No |
| 814 | 794 | -20 | -2\% | Yes |
| 795 | 791 | -4 | 0\% | Yes |
| 769 | 683 | -86 | -11\% | Yes |
| 783 | 691 | -92 | -12\% | Yes |
| 510.69732 | 460 | -51 | -10\% | Yes |
| 520 | 458 | -62 | -12\% | Yes |
| 649 | 549 | -100 | -15\% | Yes |
| 631.845153 | 621 | -10 | -2\% | Yes |
| 703 | 648 | -55 | -8\% | Yes |
| 714 | 656 | -58 | -8\% | Yes |
| 444 | 461 | 17 | 4\% | Yes |
| 453 | 454 | 1 | 0\% | Yes |
| 538 | 505 | -33 | -6\% | Yes |
| 542 | 495 | -47 | -9\% | Yes |
| 292 | 290 | -2 | -1\% | Yes |
| 303 | 290 | -13 | -4\% | Yes |
| 537 | 482 | -55 | -10\% | Yes |
| 535 | 393 | -142 | -27\% | No |

## Interpeak

| ID | Name |
| :---: | :---: |
| 111 | 56-NB |
| 112 | 56-SB |
| 113 | 57-EB |
| 114 | 57-WB |
| 115 | 58-NB |
| 116 | 58-SB |
| 117 | 59-EB |
| 118 | 59-WB |
| 119 | 60-NB |
| 120 | 60-SB |
| 121 | 61-EB |
| 122 | 61-WB |
| 123 | 62-EB |
| 124 | 62-WB |
| 125 | 63-EB |
| 126 | 63-WB |
| 127 | 64-EB |
| 128 | 64-WB |
| 129 | 101-NB |
| 130 | 101-SB |
| 131 | 102-NB |
| 132 | 102-SB |
| 133 | 103-EB |
| 134 | 103-WB |
| 135 | 104-NB |
| 136 | 104-SB |
| 137 | 105-EB |
| 138 | 105-WB |
| 139 | 106-EB |
| 140 | 106-WB |
| 141 | 200-NB |
| 142 | 200-SB |
| 143 | 201-NB |
| 144 | 201-SB |
| 145 | 202-EB |
| 146 | 202-WB |
| 147 | 203-EB |
| 148 | 203-WB |
| 149 | 204-EB |
| 150 | 204-WB |
| 151 | 205-NB |
| 152 | 205-SB |
| 153 | 206-NB |
| 154 | 206-SB |
| 155 | 207-EB |
| 156 | 207-WB |


| Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: |
| 577 | 529 | -48 | -8\% | Yes |
| 619 | 541 | -78 | -13\% | Yes |
| 604 | 522 | -82 | -14\% | Yes |
| 630 | 526 | -104 | -17\% | No |
| 556 | 544 | -12 | -2\% | Yes |
| 648 | 566 | -82 | -13\% | Yes |
| 622 | 579 | -43 | -7\% | Yes |
| 654 | 613 | -41 | -6\% | Yes |
| 873 | 958 | 85 | 10\% | Yes |
| 887 | 954 | 67 | 8\% | Yes |
| 1652 | 1673 | 21 | 1\% | Yes |
| 1746 | 1662 | -84 | -5\% | Yes |
| 1709 | 1365 | -344 | -20\% | No |
| 1645 | 1500 | -145 | -9\% | Yes |
| 901 | 729 | -172 | -19\% | No |
| 872 | 761 | -111 | -13\% | Yes |
| 311 | 299 | -12 | -4\% | Yes |
| 311 | 302 | -9 | -3\% | Yes |
| 344 | 368 | 24 | 7\% | Yes |
| 387 | 358 | -29 | -7\% | Yes |
| 723 | 632 | -91 | -13\% | Yes |
| 868 | 690 | -178 | -20\% | No |
| 359 | 388 | 29 | 8\% | Yes |
| 401 | 474 | 73 | 18\% | No |
| 641 | 516 | -125 | -20\% | No |
| 401 | 395 | -6 | -1\% | Yes |
| 464 | 446 | -18 | -4\% | Yes |
| 423 | 426 | 3 | 1\% | Yes |
| 532 | 481 | -51 | -10\% | Yes |
| 483 | 515 | 32 | 7\% | Yes |
| 709.97515 | 626 | -84 | -12\% | Yes |
| 703.380395 | 631 | -72 | -10\% | Yes |
| 1005.43087 | 880 | -126 | -13\% | Yes |
| 1028.48444 | 878 | -150 | -15\% | Yes |
| 589.922961 | 559 | -31 | -5\% | Yes |
| 594.21187 | 560 | -34 | -6\% | Yes |
| 296 | 271 | -25 | -8\% | Yes |
| 295 | 275 | -20 | -7\% | Yes |
| 752.363064 | 758 | 6 | 1\% | Yes |
| 781.897221 | 805 | 23 | 3\% | Yes |
| 411.813582 | 389 | -23 | -5\% | Yes |
| 396.237111 | 386 | -11 | -3\% | Yes |
| 329 | 362 | 33 | 10\% | Yes |
| 309.479604 | 341 | 32 | 10\% | Yes |
| 673.877994 | 616 | -57 | -9\% | Yes |
| 624.44435 | 598 | -26 | -4\% | Yes |

## PM Peak

| ID | Name |
| :---: | :---: |
| 1 | $1-$ EB |
| 2 | $1-W B$ |

PM Peak

| ID | Name | Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 57 | 29-NB | 884 | 927 | 43 | 5\% | Yes |
| 58 | 29-SB | 902 | 767 | -136 | -15\% | Yes |
| 59 | 30-EB | 998 | 799 | -199 | -20\% | No |
| 60 | 30-WB | 848 | 789 | -59 | -7\% | Yes |
| 61 | 31-NB | 487 | 501 | 14 | 3\% | Yes |
| 62 | 31-SB | 502 | 494 | -8 | -2\% | Yes |
| 63 | 32-EB | 640 | 628 | -12 | -2\% | Yes |
| 64 | 32-WB | 650 | 620 | -30 | -5\% | Yes |
| 65 | 33-NB | 459 | 393 | -66 | -14\% | Yes |
| 66 | 33-SB | 460 | 399 | -61 | -13\% | Yes |
| 67 | 34-EB | 568 | 542 | -26 | -5\% | Yes |
| 68 | 34-WB | 551 | 531 | -20 | -4\% | Yes |
| 69 | 35-NB | 794 | 835 | 41 | 5\% | Yes |
| 70 | 35-SB | 760 | 823 | 63 | 8\% | Yes |
| 71 | 36-EB | 585 | 640 | 55 | 9\% | Yes |
| 72 | 36-WB | 640 | 655 | 15 | 2\% | Yes |
| 73 | 37-EB | 719 | 728 | 9 | 1\% | Yes |
| 74 | 37-WB | 704 | 734 | 30 | 4\% | Yes |
| 75 | 38-SB | 909 | 822 | -87 | -10\% | Yes |
| 76 | 38-NB | 862 | 817 | -45 | -5\% | Yes |
| 77 | 39-NB | 525 | 486 | -39 | -7\% | Yes |
| 78 | 39-SB | 502 | 478 | -24 | -5\% | Yes |
| 79 | 40-NB | 1384 | 907 | -477 | -34\% | No |
| 80 | 40-SB | 1180 | 954 | -226 | -19\% | No |
| 81 | 41-NB | 680 | 694 | 14 | 2\% | Yes |
| 82 | 41-SB | 712 | 684 | -28 | -4\% | Yes |
| 83 | 42-NB | 843.312359 | 774 | -70 | -8\% | Yes |
| 84 | 42-SB | 851 | 778 | -73 | -9\% | Yes |
| 85 | 43-NB | 583 | 531 | -52 | -9\% | Yes |
| 86 | 43-SB | 630 | 537 | -93 | -15\% | Yes |
| 87 | 44-EB | 549 | 501 | -48 | -9\% | Yes |
| 88 | 44-WB | 544 | 498 | -46 | -9\% | Yes |
| 89 | 45-EB | 773 | 776 | 3 | 0\% | Yes |
| 90 | 45-WB | 800 | 769 | -31 | -4\% | Yes |
| 91 | 46-EB | 676 | 559 | -117 | -17\% | No |
| 92 | 46-WB | 689 | 555 | -134 | -19\% | No |
| 93 | 47-EB | 819 | 795 | -24 | -3\% | Yes |
| 94 | 47-WB | 760 | 792 | 32 | 4\% | Yes |
| 95 | 48-NB | 801 | 690 | -111 | -14\% | Yes |
| 96 | 48-SB | 775 | 708 | -67 | -9\% | Yes |
| 97 | 49-NB | 508.012052 | 489 | -19 | -4\% | Yes |
| 98 | 49-SB | 490 | 474 | -16 | -3\% | Yes |
| 99 | 50-EB | 639 | 637 | -2 | 0\% | Yes |
| 100 | 50-WB | 608 | 582 | -26 | -4\% | Yes |
| 101 | 51-EB | 705 | 667 | -38 | -5\% | Yes |
| 102 | 51-WB | 696 | 658 | -38 | -5\% | Yes |
| 103 | 52-EB | 412 | 467 | 55 | 13\% | Yes |
| 104 | 52-WB | 440 | 458 | 18 | 4\% | Yes |
| 105 | 53-EB | 600 | 536 | -64 | -11\% | Yes |
| 106 | 53-WB | 565 | 501 | -64 | -11\% | Yes |
| 107 | 54-EB | 290 | 302 | 12 | 4\% | Yes |
| 108 | 54-WB | 301 | 294 | -7 | -2\% | Yes |
| 109 | 55-NB | 579 | 518 | -61 | -10\% | Yes |
| 110 | 55-SB | 606 | 405 | -201 | -33\% | No |

PM Peak

| ID | Name |
| :---: | :---: |
| 111 | 56-NB |
| 112 | 56-SB |
| 113 | 57-EB |
| 114 | 57-WB |
| 115 | 58-NB |
| 116 | 58-SB |
| 117 | 59-EB |
| 118 | 59-WB |
| 119 | 60-NB |
| 120 | 60-SB |
| 121 | 61-EB |
| 122 | 61-WB |
| 123 | 62-EB |
| 124 | 62-WB |
| 125 | 63-EB |
| 126 | 63-WB |
| 127 | 64-EB |
| 128 | 64-WB |
| 129 | 101-NB |
| 130 | 101-SB |
| 131 | 102-NB |
| 132 | 102-SB |
| 133 | 103-EB |
| 134 | 103-WB |
| 135 | 104-NB |
| 136 | 104-SB |
| 137 | 105-EB |
| 138 | 105-WB |
| 139 | 106-EB |
| 140 | 106-WB |
| 141 | 200-NB |
| 142 | 200-SB |
| 143 | 201-NB |
| 144 | 201-SB |
| 145 | 202-EB |
| 146 | 202-WB |
| 147 | 203-EB |
| 148 | 203-WB |
| 149 | 204-EB |
| 150 | 204-WB |
| 151 | 205-NB |
| 152 | 205-SB |
| 153 | 206-NB |
| 154 | 206-SB |
| 155 | 207-EB |
| 156 | 207-WB |


| Observed (s) | Modelled (s) | Diff | \% | Pass? |
| :---: | :---: | :---: | :---: | :---: |
| 595 | 541 | -54 | -9\% | Yes |
| 615 | 572 | -43 | -7\% | Yes |
| 662 | 553 | -109 | -16\% | No |
| 699 | 546 | -153 | -22\% | No |
| 869 | 625 | -244 | -28\% | No |
| 972 | 653 | -319 | -33\% | No |
| 603 | 629 | 26 | 4\% | Yes |
| 622 | 626 | 4 | 1\% | Yes |
| 958 | 958 | 0 | 0\% | Yes |
| 941 | 1005 | 64 | 7\% | Yes |
| 2491 | 1683 | -808 | -32\% | No |
| 2228 | 1738 | -490 | -22\% | No |
| 2458 | 1330 | -1128 | -46\% | No |
| 2808 | 1807 | -1001 | -36\% | No |
| 1303 | 748 | -555 | -43\% | No |
| 1131 | 845 | -286 | -25\% | No |
| 323 | 344 | 21 | 7\% | Yes |
| 312 | 314 | 2 | 1\% | Yes |
| 341 | 376 | 35 | 10\% | Yes |
| 467 | 432 | -35 | -8\% | Yes |
| 683 | 630 | -53 | -8\% | Yes |
| 760 | 708 | -52 | -7\% | Yes |
| 352 | 369 | 17 | 5\% | Yes |
| 468 | 488 | 20 | 4\% | Yes |
| 655 | 546 | -109 | -17\% | No |
| 359 | 399 | 40 | 11\% | Yes |
| 462 | 444 | -18 | -4\% | Yes |
| 412 | 440 | 28 | 7\% | Yes |
| 677 | 483 | -194 | -29\% | No |
| 570 | 548 | -22 | -4\% | Yes |
| 673.08282 | 634 | -39 | -6\% | Yes |
| 737.282279 | 640 | -97 | -13\% | Yes |
| 983.804215 | 884 | -100 | -10\% | Yes |
| 1001.79803 | 888 | -114 | -11\% | Yes |
| 554.51515 | 566 | 12 | 2\% | Yes |
| 554.963935 | 568 | 13 | 2\% | Yes |
| 269 | 273 | 4 | 1\% | Yes |
| 321 | 276 | -45 | -14\% | Yes |
| 778.469468 | 826 | 48 | 6\% | Yes |
| 771.010842 | 807 | 36 | 5\% | Yes |
| 393.816157 | 396 | 2 | 0\% | Yes |
| 377.123742 | 382 | 5 | 1\% | Yes |
| 313 | 374 | 61 | 19\% | No |
| 322.831513 | 347 | 24 | 7\% | Yes |
| 592.584831 | 624 | 31 | 5\% | Yes |
| 594.142846 | 597 | 3 | 0\% | Yes |

# Appendix D. 3 

J OURNEY TIME ROUTE GRAPHS AM



| 1-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |
| 600 ( ${ }^{\text {a }}$ |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| $1_{2027}$ |  |  |  |  |  |  |  |
| -100 9 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  |  |  |  |  |  |  |  |
|  |  | $\rightarrow$ | $\rightarrow$ | - - |  |  |  |


| 2-B |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 600 |  |  |  |  |  |  |  |
| 500 年 |  |  |  |  |  |  |  |
| $\widetilde{5}^{400}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |
| ${ }_{200}^{1526 \ldots \ldots}$ |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  |  |  |  |  |  |  |  |
|  |  | $\square$ | $\bigcirc$ | - - |  |  |  |


| 2-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 600 2527 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |
| ${ }_{2180}{ }_{2185}$ |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 |  | 8000 | 10000 | 12000 | 14000 |
|  |  |  |  |  |  |  |  |
|  |  | $\square$ | $\square$ | - - |  |  |  |



EME





600 5-MB
88
500

(s) $\partial \boldsymbol{\square}!\perp$




EM-L

| 7-WM |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 600 |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |
| $\overbrace{}^{400}$ |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |
| $\begin{aligned} & 100 \\ & 30075 \end{aligned}$ |  |  |  |  |  |  |
| $02000 \quad 4000 \quad 6000 \quad 10000$ |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |
|  |  | Model | bserved | RBRang |  |  |



$$
\begin{aligned}
& \text { 8-WB }
\end{aligned}
$$


9.sB

700
700
600






| 12-NB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |  |
| 600 (3217 |  |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |  |
| 1003195 |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 | 10000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |  |  |  |
| $\multimap$ Modelled $\multimap$ Observed - - DMRBRange |  |  |  |  |  |  |  |  |  |  |


| 12-SB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \bar{\pi} 400 \\ & \stackrel{\rightharpoonup}{\omega} \\ & \underline{E}=300 \end{aligned}$ |  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |  |
| $100_{3217}$ |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 |  |  | 5000 | 6000 | 7000 | 8000 | 9000 | 10000 |
|  |  |  |  |  | stance ( $\mathbf{m}$ ) |  |  |  |  |  |
|  |  |  | $\bigcirc$ | d | Observed | - D |  |  |  |  |



| 13-5B |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |
| 玉 $400 \sim \ldots-3225$ |  |  |  |  |  |  |
| $\text { 兰 } 300 \quad 3328-3224$ |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |
| $100_{3233}$ |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |
|  |  | Mode | served | B Range |  |  |



14-SB


1000
1000
900
800
700
600
500
400
300
200
10030
0
(s) $\boldsymbol{\partial}!\perp$


15-SB



| 16-5B |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 ( 279 |  |  |  |  |  |
| 1000 - |  |  |  |  |  |
| $800$ |  |  |  |  |  |
|  |  |  |  |  |  |
|  |  |  |  |  |  |
| $200$ |  |  |  |  |  |
| 0 2000 4000 6000 8000 10000 |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |
| $\longrightarrow$ Modelled Observed - - DMRB Range |  |  |  |  |  |


| 17-NB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |  |
| 600 边 |  |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |  |
| 1002793 |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 | 10000 |
|  |  |  |  |  | tance (m) |  |  |  |  |  |
|  |  |  | $\square$ | d | bserved | - - D |  |  |  |  |


| 17-SB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{100}{ }_{3316}$ |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 | 10000 |
|  |  |  |  |  | stance (m) |  |  |  |  |  |
|  |  |  | $\square$ | - | Observed | - D |  |  |  |  |



| 18-SB |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |  |
| 500 边 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 200 - $2133 \cdots$ |  |  |  |  |  |  |  |  |  |
| $100_{2111}$ |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  | Model | $\square \bigcirc$ | - | BRange |  |  |  |




| 20－NB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 450 |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |
| 350 |  |  |  |  |  |  |  |
| （ $\begin{array}{r}300 \\ \text { ¢ } 250\end{array}$ |  |  |  |  |  |  |  |
| $\begin{aligned} & \frac{\pi}{\pi} 250 \\ & 0 \\ & \stackrel{\sigma}{F} 200 \end{aligned}$ |  |  |  |  |  |  |  |
| ${ }^{150}$ |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |
| $55_{3108}$ |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 |
|  | Distance（m） |  |  |  |  |  |  |
|  |  | $\square$ | $\rightarrow$ | －－ |  |  |  |


| 20－SB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 450 |  |  |  |  |  |  |  |
| 400 边 |  |  |  |  |  |  |  |
| 350 |  |  |  |  |  |  |  |
| ［ $\begin{array}{r}300 \\ \text {（9）250 }\end{array}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 亭 200 边 |  |  |  |  |  |  |  |
| 150 |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |
| 503270 ニニ－ |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 |
|  |  |  |  |  |  |  |  |
|  |  | $\square$ | $\longrightarrow$ | －－ |  |  |  |



21-WB


| 22-B3 |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |
| 700 边 |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |
| 1003264 |  |  |  |  |  |  |  |
| 0 |  |  |  |  |  |  |  |
|  | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  | Distance (m) |  |  |  |  |  |  |
| $\ldots$ Modelled $\longrightarrow$-Observed - - DMRB Range |  |  |  |  |  |  |  |


| 22-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |  |
| 800 边 |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 300200 |  |  |  |  |  |  |  |
| 1003943 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |
|  |  | $\square$ | $\bigcirc$ | - - |  |  |  |


| 23-B |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |  |
| 800 ( 700 - 5604 |  |  |  |  |  |  |  |
| 700 年 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| (en 500 |  |  |  |  |  |  |  |
| 300 |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |
| 1003943 |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  | Distance (m) |  |  |  |  |  |  |
|  |  | $\ldots$ | $\bigcirc$ | - - |  |  |  |


| 23-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |  |
| 800700 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 700600 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 300 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Distance ( $m$ ) |  |  |  |  |  |  |  |
|  |  | $\square$ | $\square$ | - - |  |  |  |








| 29-SB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |
| 1000 ( |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 玉 ${ }^{800}$ ( ${ }^{\text {a }}$ |  |  |  |  |  |  |  |
| 㝥 600 |  |  |  |  |  |  |  |
| - 400 |  |  |  |  |  |  |  |
| ${ }^{200} 3278$ |  |  |  |  |  |  |  |
| 0 |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |
| $\ldots$ Modelled $\multimap$ Observed - - - DMRB Range |  |  |  |  |  |  |  |


| 30-B |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 200 3799 $200 \ldots \ldots$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  | Distance ( m ) |  |  |  |  |  |  |
| $\ldots$ Modelled $\longrightarrow$ Oobserved - - DMRBRange |  |  |  |  |  |  |  |



| 31-NB |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| $100{ }^{100}$ |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Distance ( $m$ ) |  |  |  |  |  |  |
| $\ldots$ Modelled $\longrightarrow$ Oobserved - - DMRB Range |  |  |  |  |  |  |


| 31-SB |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |
| 600 边 |  |  |  |  |  |  |
| 500 边 |  |  |  |  |  |  |
| $\begin{aligned} & \bar{\pi} 400 \\ & \stackrel{\rightharpoonup}{0} \\ & \text { In } 300 \end{aligned}$ |  |  |  |  |  |  |
| $100{ }^{200}$ |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Distance ( $m$ ) |  |  |  |  |  |  |
| $\multimap$ Modelled $\multimap$ Observed - - DMRBRange |  |  |  |  |  |  |



| 32－WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |  |
| 800 边 |  |  |  |  |  |  |  |
| 700 边 |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |
| $\begin{aligned} & \bar{\pi} 500 \\ & \stackrel{⿹}{0} 00 \\ & E=400 \end{aligned}$ |  |  |  |  |  |  |  |
| 300 |  |  |  |  |  |  |  |
| ${ }_{100}^{200} 309 \ldots \ldots$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 0 － 0 － |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
| Distance（ $m$ ） |  |  |  |  |  |  |  |
|  |  | $\square$ | $\ldots$ | －－ |  |  |  |



33-5B


gMtE
3
(



700
700
600 (s) aulı


| 38-SB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |  |
| © ${ }^{800}$ ( |  |  |  |  |  |  |  |  |
| 年 |  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |  |
| 2003135 |  |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 | 16000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |  |
| $\multimap$ Modelled $\multimap$ Observed - - DMRBRange |  |  |  |  |  |  |  |  |




39-SB

| 39-sB |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 600 |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |  |  |
|  | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  | Model | $\bigcirc 0$ | - | Brange |  |  |  |





| 42-NB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| [ ${ }^{800}$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |  |
| ${ }_{3217}$ |  |  |  |  |  |  |  |  |
| 0 |  |  |  |  |  |  |  |  |
| 0 | Distance (m) |  |  |  |  |  |  |  |
| $\multimap$ Modelled $\multimap$ Observed - - DMRBRRange |  |  |  |  |  |  |  |  |


| 42-SB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| [5 800 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |
| ${ }_{0}^{3983}$ |  |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 | 16000 |
|  |  |  |  | stance (m) |  |  |  |  |
|  |  |  | led | oserved | - DMRB |  |  |  |






45-WB

(s) $\boldsymbol{\partial} \boldsymbol{\square}!$




| 47-WB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1000 |  |  |  |  |  |  |  |  |
| 800 |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| $\begin{gathered} 200 \\ 321 \end{gathered}$ |  |  |  |  |  |  |  |  |
| -2000 | -200 | 2000 | 4000 |  |  | 10000 | 12000 | 14000 |
|  | -200 Distance (m) |  |  |  |  |  |  |  |
| $\multimap$ Modelled $\longrightarrow$ Observed - - DMRBRange |  |  |  |  |  |  |  |  |



| 49-NB |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |  |  |
| 玉 400 |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{100} 3327889$ |  |  |  |  |  |  |  |  |  |  |  |
| -1000 | $-100{ }^{\circ}$ | 1000 | 2000 | 3000 |  |  |  | 7000 | 8000 | 9000 | 10000 |
|  |  |  |  |  |  |  |  |  |  |  |  |
| $\multimap$ Modelled $\multimap$ Observed - - DMRBRange |  |  |  |  |  |  |  |  |  |  |  |


| 49-SB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |  |
| 600 边 |  |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 200 - 3914 |  |  |  |  |  |  |  |  |  |  |
| $1^{100} 3325$ |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 | 10000 |
|  |  |  |  |  | tance (m) |  |  |  |  |  |
|  |  |  | $\bigcirc$ | - | Observed | - D |  |  |  |  |



50-WB

| 50-WB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 |  | $5000$ istance (m) |  |  | 8000 | 9000 | 10000 |
|  |  |  | - M | - | observed | - D |  |  |  |  |





600
500



53-WB

| 53-VB |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |
| 600 ( 6012 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |
| $100_{2540}$ |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 |
|  | Distance ( $m$ ) |  |  |  |  |  |
|  |  | Mode | oserved | R Range |  |  |





55-SB









| 61-B |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3000 |  |  |  |  |  |  |  |  |
| 2500 |  |  |  |  |  |  |  |  |
| 2000 ( $\ldots \ldots \ldots \ldots$ |  |  |  |  |  |  |  |  |
| ( 1000 |  |  |  |  |  |  |  |  |
| ${ }_{30048730} 30155$ |  |  |  |  |  |  |  |  |
| $-500{ }^{0}$ | 2000 | 4000 | 6000 |  |  | 12000 | 14000 | 16000 |
|  | Distance (m) |  |  |  |  |  |  |  |
| $\multimap$ Modelled $\multimap$ Observed - - - DMRBRange |  |  |  |  |  |  |  |  |




3000
2500

[^1]





| 103-B |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 500 |  |  |  |  |  |  |  |
| 450400 |  |  |  |  |  |  |  |
| 350¢900 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| - 250 |  |  |  |  |  |  |  |
| 150 - 704 |  |  |  |  |  |  |  |
| 100 - 10 |  |  |  |  |  |  |  |
| 0 500 1000 1500 2000 2500 3000 3500 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Distance ( $m$ ) |  |  |  |  |  |  |  |
| $\ldots$ Modelled $\multimap$ Observed - - DMRBRange |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 103-WB |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |
| 450 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| $\begin{aligned} & 400 \\ & 350 \end{aligned}$ |  |  |  |  |  |  |  |
| © 300 |  |  |  |  |  |  |  |
| - 250 |  |  |  |  |  |  |  |
| 150 |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |
| 506070 <br> 0 |  |  |  |  |  |  |  |
| 0 | 500 | 1000 | 1500 | 2000 | 2500 | 3000 | 3500 |
|  |  |  |  |  |  |  |  |
|  |  | $\square$ | $\square$ | - - |  |  |  |



104-SB
104-SB

| 104-SB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 500 |  |  |  |  |  |  |  |  |
| 450 ( 4020 |  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |  |
| - $\begin{array}{r}350 \\ 300\end{array}$ ( |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \stackrel{\rightharpoonup}{\otimes} 250 \\ & \stackrel{\rightharpoonup}{E} 200 \end{aligned}$ |  |  |  |  |  |  |  |  |
| 150 |  |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |  |
| 501250 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 0 | 500 | 1000 | 1500 | 2000 | 2500 | 3000 | 3500 | 4000 |
|  |  |  |  | stance ( |  |  |  |  |
|  |  |  | elled | bserved | DMRB |  |  |  |



105-WB
108-WB
10






| 201-NB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1400 |  |  |  |  |  |  |  |  |
| 1200 |  |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |  |
| $\begin{array}{ll} \frac{\pi}{0} & 800 \\ 0 \\ \frac{\pi}{F} & 600 \end{array}$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |  |
| $2003157 ~^{5771}$ |  |  |  |  |  |  |  |  |
|          <br> 0 2000 4000 6000 8000 10000 12000 14000 16000 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |  |
| $\multimap$ Modelled $\longrightarrow$ Oobserved - - DMRBRange |  |  |  |  |  |  |  |  |


| 201-SB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1400 |  |  |  |  |  |  |  |  |
| 1200 |  |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |  |
| $\ldots$ Modelled $\multimap$ Observed - - DMRBRange |  |  |  |  |  |  |  |  |



202-WB
-
202-NB




| 203-WB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 400 |  |  |  |  |  |  |  |  |
| 350 |  |  |  |  |  |  |  |  |
| 300 边 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |  |
| $50_{5945}$ |  |  |  |  |  |  |  |  |
| 0 | 500 | 1000 | 1500 | 2000 | 2500 | 3000 | 3500 | 4000 |
|  |  |  |  | tance (m) |  |  |  |  |
|  |  |  | alled | served | - DMR |  |  |  |


204.WB

| 204-NB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |
| 1000 ( 3158 |  |  |  |  |  |  |  |
| ¢ 800 ( ${ }^{\text {a }}$ |  |  |  |  |  |  |  |
| $\begin{aligned} & \stackrel{\text { ज }}{\stackrel{\pi}{0}} \\ & \stackrel{y}{\mid=} \\ & \hline \end{aligned}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| $2003169$ |  |  |  |  |  |  |  |
| 0 2000 4000 6000 8000 10000 12000 14000 |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |
| $\multimap$ Modelled $\simeq$ - Observed - - - DMRBR Range |  |  |  |  |  |  |  |




206-SB



9M-LOZ

| 207-NB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |  |
| 600 边 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 200 - .5519 - |  |  |  |  |  |  |  |  |
| $100_{3183}$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  | 1000 | 2000 | 3000 | 5000 | 6000 | 7000 | 8000 | 9000 |
|  | Distance (m) |  |  |  |  |  |  |  |
|  |  |  | Mode | - | R Range |  |  |  |

# Appendix D. 4 

## J OURNEY TIME ROUTE GRAPHS

いい|
INTER PEAK


| 1-WB |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 500 |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| -100 |  |  |  |  |  |  |
|  | $\rightarrow$ Modelled | Obsered | DMRB Range | - Series |  |  |



2-WB






| 5-B |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 500 |  |  |  |  |  |  |  |  |  |
| 450 |  |  |  |  |  |  |  |  |  |
| 400350 |  |  |  |  |  |  |  |  |  |
| - 300 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 150 |  |  |  |  |  |  |  |  |  |
| 100 [0. |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 0 | 2000 |  | 4000 |  |  |  | 10000 |  | 12000 |
|  |  |  |  |  |  |  |  |  |  |
|  | $\ldots$ Modelled $\sim$ Observed - - DMRB Range $\ldots$ Series |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 5-WB |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |
| 450 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 150100 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
| $503141 \times \ldots$ |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 6000 | 7000 | 8000 | 9000 | 10000 |
|  |  |  |  |  |  |  |  |  |  |
|  |  | $\rightarrow$ | delled | - Obs | RB Range | $\cdots$ |  |  |  |





| 7-MB |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |
| 450 |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |
| 350300 |  |  |  |  |  |  |
| ธ 300 |  |  |  |  |  |  |
| $\stackrel{\text { E }}{\text { E }} 250$ |  |  |  |  |  |  |
| $150$ |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |
| $\longrightarrow$ Modelled $\longrightarrow$ Observed $\sim$ - DMRB Range $\ldots$ Series5 |  |  |  |  |  |  |



8-WB


9.98


| 10-NB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |
| 700 年 |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |
| $\widetilde{5}^{500}$ |  |  |  |  |  |  |  |
| \% ${ }_{\text {\% }}^{\text {\% }}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| ${ }^{1030095 ~}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  | Distance (m) |  |  |  |  |  |  |
|  |  | Iodelled | Observed | DMRB Range | - Seris |  |  |


| 10-SB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |
| 600 . 60096 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| ( 400 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 200 - 50053--20 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| -100 0 | 2000 | 4000 | 6000 |  | 10000 | 12000 | 14000 |
|  | Distance (m) |  |  |  |  |  |  |
| $\ldots$ Modelled ——Observed - - DMRB Range ఋ. Series |  |  |  |  |  |  |  |



| 17-WB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |  |  |
| 1000 ( 10 |  |  |  |  |  |  |  |  |  |  |
| 800 |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \stackrel{\text { ज }}{\stackrel{\pi}{0}} \\ & \stackrel{\rightharpoonup}{E} \end{aligned} 600$ |  |  |  |  |  |  |  |  |  |  |
| $400$ |  |  |  |  |  |  |  |  |  |  |
| $2003195 \ldots$ |  |  |  |  |  |  |  |  |  |  |
| 0 2000 4000 6000 8000 10000 12000 14000 16000 18000 20000 |  |  |  |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |  |  |  |
| $\longrightarrow$ Modelled $\longrightarrow$ Observed $\ldots$ Series5 |  |  |  |  |  |  |  |  |  |  |



| 12-SB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |  |
| 600 ( |  |  |  |  |  |  |  |  |  |  |
| 500 ( |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \bar{\pi} 400 \\ & \stackrel{0}{\omega}=300 \\ & \underline{E}=3 \end{aligned}$ |  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |  |
| $100_{3217}$ |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 |  |  |  |  | 7000 | 8000 | 9000 | 10000 |
|  |  |  |  |  | tance ( m ) |  |  |  |  |  |
|  |  | $\longrightarrow$ | delled | - Obse | - - - | B Range | $\cdots$ |  |  |  |





14-SB



15-SB



16-SB



17-SB


$\mathbf{9 S - 8 I}$

| 18-SB |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |
| 1002111 |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 |
|  |  |  |  |  |  |  |  |  |  |
|  |  | $\rightarrow$ | - | served | - DMR | - |  |  |  |



19-5B



20-SB

| 20-SB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 450 |  |  |  |  |  |  |  |
| 400 ( 3108 |  |  |  |  |  |  |  |
| 350 边 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 150 |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |
| 503270 |  |  |  |  |  |  |  |
|  | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 |
|  | Distance (m) |  |  |  |  |  |  |
|  |  | Modelled | bserved | DMRB Range | - Seri |  |  |



21-WB




aMEZ

| 23-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |  |
| 800 |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |
| ${ }^{600}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 300 |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |
| $1005604,2-3345$ - |  |  |  |  |  |  |  |
| 0 |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |
|  |  | odelled | Observed | DMRB Range | " Ser |  |  |








| 29.58 |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 |  | 800 |  | 12000 | 14000 |
| Disame (m) |  |  |  |  |  |  |  |
| $\ldots$ Modelled $\rightarrow$ Observed -- DMRBRange $\ldots$ Series |  |  |  |  |  |  |  |




| 31-SB |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |
| 600 边 |  |  |  |  |  |  |
| 500 - |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |
| $1^{100} 3320$ |  |  |  |  |  |  |
|  | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 |
| Distance (m) |  |  |  |  |  |  |
|  |  | d - | - | $\cdots$ |  |  |



## am-ze


(s) $\boldsymbol{\text { mu! }}$


33-SB


$\mathbf{a n t e}$

| 34-VB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \text { 可 } 400 \\ & 0 \\ & \stackrel{1}{\underline{E}} 300 \\ & \underline{1} 300 \end{aligned}$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 200 - 334 |  |  |  |  |  |  |  |  |
| ${ }_{0}^{100}{ }_{0}$ |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |  |
|  |  | - Mod | $\square-$ | - - | Range | Series5 |  |  |








| 38-NB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |
| 1000 年 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| ${ }^{200} 3130$ |  |  |  |  |  |  |  |  |
| $\begin{array}{lllllllll}0 & 2000 & 4000 & 6000 & 8000 & 10000 & 12000 & 14000 & 16000\end{array}$ |  |  |  |  |  |  |  |  |
| Distance ( $m$ ) |  |  |  |  |  |  |  |  |
| $\ldots$ Modelled ——Observed - - DMRB Range ऑ. Series5 |  |  |  |  |  |  |  |  |






| 41-SB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |  |
| 800 |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |
| $\begin{array}{ll} \text { ज } & 500 \\ \stackrel{\rightharpoonup}{\omega} & 400 \\ \underset{\underline{E}}{ } & 300 \end{array}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |
| $1002861$ |  |  |  |  |  |  |  |
| -100 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
| ( Distance (m) |  |  |  |  |  |  |  |
|  |  | odelled | bserved | DMRB Range | - Seri |  |  |





43-5B

$$
\text { (s) } \partial \omega_{!\perp}
$$



| 44.WB |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |
| 700 |  |  |  |  |  |
| 600 |  |  |  |  |  |
| $\begin{aligned} & 500 \\ & \mathbf{5}_{5}^{500} \\ & 0.000 \end{aligned}$ |  |  |  |  |  |
|  |  |  |  |  |  |
| $\begin{array}{r} \boldsymbol{F}_{300} \\ 200 \end{array}$ |  |  |  |  |  |
| 1003189 |  |  |  |  |  |
| 0 | 2000 | 4000 | 8000 | 10000 | 12000 |
|  |  |  |  |  |  |
|  |  | d | - |  |  |



| 45-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1000 |  |  |  |  |  |  |  |
| 900800 |  |  |  |  |  |  |  |
| $\begin{array}{r}700 \\ \hline 600\end{array}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| $\begin{array}{r}100 \\ \hline 300 \\ \\ \hline 200\end{array}$ |  |  |  |  |  |  |  |
| 2001003201 |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 |  | 10000 | 12000 | 14000 |
|  |  |  |  |  |  |  |  |
|  |  | odelled | Dbsered | DMRE | - Sels |  |  |





| 47-WB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1000 |  |  |  |  |  |  |  |  |
| 800 -5910541822 |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| $0215,5534-\cdots-1$ |  |  |  |  |  |  |  |  |
| -2000 | $-200$ |  |  | 6000 | $8000$ | $10000$ | 12000 | 14000 |
|  |  | $\longrightarrow$ | $\longrightarrow$ | - | Range | Series5 |  |  |






50-WB

| 50-WB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |  |  |  |
| 700 边 |  |  |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 200 - 2534 |  |  |  |  |  |  |  |  |  |  |
| 1002539 |  |  |  |  |  |  |  |  |  |  |
| 0 |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 | 10000 |
| Distance (m) |  |  |  |  |  |  |  |  |  |  |
|  |  | $\longrightarrow$ | delled | - Obs | - - | B Range | $\cdots$ |  |  |  |



51-WB


aM-zs



$$
53-\mathrm{WB}
$$





55-SB




57-vB

| 57-WB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |
| $\widehat{5}^{500}$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |
| $1002381$ |  |  |  |  |  |  |  |  |
| 0 1000 2000 3000 4000 5000 6000 7000 8000 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |  |
|  |  | - Mod | $\square$ | - - | Range | Series5 |  |  |










> 6-WB




| 102-NB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |  |  |  |  |
| 800 |  |  |  |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |  |  |  |
| 玉s $\begin{array}{r}600 \\ 500\end{array}$ |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| ( |  |  |  |  |  |  |  |  |  |  |
| 300 600806090 - |  |  |  |  |  |  |  |  |  |  |
| 200 6050 6070 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 | 10000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |  |  |  |
| $\multimap$ Modelled $\multimap$ Observed - - DMRB Range $\ldots$ Series5 |  |  |  |  |  |  |  |  |  |  |


| 102-SB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |  |  |  |
| [5 ${ }^{800}$ ( ${ }^{(1000}$ |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| ${ }_{4}^{200}{ }_{420}$ |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 | 10000 |
|  |  |  |  |  | Distance ( $m$ ) |  |  |  |  |  |
|  |  | $\square$ | elled | Obse | - - - | B Range | * |  |  |  |



| 104-NB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| $100_{1020}$ |  |  |  |  |  |  |  |
| 0 | 500 | 1000 | 1500 | 2000 | 2500 | 3000 | 3500 |
|  | Distance ( $\mathbf{m}$ ) |  |  |  |  |  |  |
| $\ldots$ Modelled $\sim$ Observed - - DMRB Range $\ldots$ Series5 |  |  |  |  |  |  |  |

GStOL


| 105-WB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 600 |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \frac{\pi}{\mathbf{o}} 300 \\ & \text { 压 } \end{aligned}$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| $-100{ }^{\circ}$ |  |  |  |  |  |  |  |  |
|  | $\cdots$ | - | seved | - DMRB | $\cdots$ |  |  |  |







| 201-SB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1400 |  |  |  |  |  |  |  |  |
| 1200 |  |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| $-200{ }^{\circ}$ |  |  | 6000 |  | 10000 | 12000 |  | 16000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |  |
| $\ldots$ Modelled ——observed - - DMRB Range $\ldots$ Series |  |  |  |  |  |  |  |  |




| 203-EB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 400 |  |  |  |  |  |  |  |  |
| 350 |  |  |  |  |  |  |  |  |
| 300 |  |  |  |  |  |  |  |  |
| 250 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |  |
| $50_{3177,}, 3173$ |  |  |  |  |  |  |  |  |
| $-50$ | 500 | 1000 | 1500 | $\begin{gathered} 2000 \\ \text { Distance (m) } \end{gathered}$ | 2500 | 3000 | 3500 | 4000 |
| $\multimap$ Modelled $\multimap$ Observed - - DMRB Range $\simeq$ Series |  |  |  |  |  |  |  |  |



204.WB
$+2$

| 204-NB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1000 |  |  |  |  |  |  |  |
| 900 - 3158 |  |  |  |  |  |  |  |
| 800 |  |  |  |  |  |  |  |
| $\begin{array}{ll} \text { ज } & 600 \\ \stackrel{0}{0} & 500 \\ \underline{E} & 400 \end{array}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| $300$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |
| $\ldots$ Modelled $\sim$ Observed - - DMRB Range $\ldots$ Series5 |  |  |  |  |  |  |  |



85-902
205


| 206-NB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 450 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 350 |  |  |  |  |  |  |  |  |  |  |
| ( $\begin{array}{r}300 \\ \text { ¢ } 250\end{array}$ |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 150 |  |  |  |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |  |  |  |
| $50_{3190}$ |  |  |  |  |  |  |  |  |  |  |
| 0 | 500 | 1000 | 1500 | 2000 | 2500 | 3000 | 3500 | 4000 | 4500 | 5000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |  |  |  |
|  |  | $\cdots$ | delled | - Obse | - - | B Range | $\cdots$ - S |  |  |  |


| 206-SB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ${ }^{400} \mid$ |  |  |  |  |  |  |  |  |  |  |
| 350 |  |  |  |  |  |  |  |  |  |  |
| 300 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |  |  |  |
| ${ }^{503183} \ldots$ |  |  |  |  |  |  |  |  |  |  |
| 0 | 500 | 1000 | 1500 | 2000 | 2500 | 3000 | 3500 | 4000 | 4500 | 5000 |
|  |  |  |  |  | Stamee (m) |  |  |  |  |  |
|  |  | $\cdots$ | deled | - obs | -- - | B Range | * S |  |  |  |


| 207-B |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |  |  |  |
| 800 |  |  |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |  |  |
| ${ }_{5}^{600} \times 1{ }^{600}$ |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 300200 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 200$1003195 \ldots$ |  |  |  |  |  |  |  |  |  |
|  | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |  |  |
|  |  | $\square$ | d | served | - DMRE | $\cdots$ |  |  |  |


| 207-WB |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |  |  |
| 700 边 |  |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 200 . 5549 |  |  |  |  |  |  |  |  |  |
| $100_{3183} \ldots \ldots$ |  |  |  |  |  |  |  |  |  |
| 0 0 |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 |
|  | Distance ( m ) |  |  |  |  |  |  |  |  |
|  |  | $\ldots$ | - | served | - DMR | $\cdots$ |  |  |  |

# Appendix D. 5 

J OURNEY TIME ROUTE GRAPHS PM —— いゝ|)


| 1-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 500450 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |
| 350300 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| $\begin{array}{r} 150 \\ 100 \end{array}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  | Iodeled | Observed |  | - Ser |  |  |



2-WB
2



3-WB
3-1




700
600
(s) $\partial W_{\Perp}$



| 5-B |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 500 |  |  |  |  |  |  |  |  |  |
| 450400 |  |  |  |  |  |  |  |  |  |
| © $\begin{array}{r}350 \\ 300\end{array}$ |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 150 |  |  |  |  |  |  |  |  |  |
| 1002367 , - - |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
|  | 2000 |  | 4000 |  |  |  | 10000 |  | 12000 |
|  |  |  |  |  |  |  |  |  |  |
| $\ldots$ Modeled $\sim$ Observed - - DMRB Range $\ldots$ Series5 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 5-WB |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |
| 450 ( 4 , |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 350 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 150 |  |  |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |  |  |
| 503141 |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 6000 | 7000 | 8000 | 9000 | 10000 |
|  |  |  |  |  |  |  |  |  |  |
|  |  | $\rightarrow$ | deled | - obse | B Range | $\cdots$ |  |  |  |



6-WB
$\square$



600
500

(s) $\boldsymbol{\text { əu! }}$


| 7-MB |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |
| 500 30052 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |
| $1_{30075}$ |  |  |  |  |  |  |
| 0 2000 4000 6000 8000 10000 12000 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |
| $\longrightarrow$ Modelled $\longrightarrow$ Observed $\ldots$ - - DMRB Range $\ldots$ Series5 |  |  |  |  |  |  |



8-WB

9.98




| 11-WB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |  |  |
| 1000 ( 800 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{\circ} 400$ |  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |  |
| 0 - 0 - - - - - |  |  |  |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 | 16000 | 18000 | 20000 |
| Distance (m) |  |  |  |  |  |  |  |  |  |  |
|  |  | $\ldots$ | elled | - obse | - - - | RB Range | - S |  |  |  |



| 12-SB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |  |
| $\frac{\text { ज }}{0} 400$ |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |  |
| $100_{3217}$ |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |  |  |  |
|  |  | - | delled | - Obs | - - | B Range | $\cdots$ |  |  |  |



| 13-5B |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |
| © 400 |  |  |  |  |  |  |
| IE 300 |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |
| $100_{3233}$ |  |  |  |  |  |  |
| 0 2000 4000 6000 8000 10000 12000 |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |
|  |  | - | - - | - |  |  |


| 14-NB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| [ ${ }^{800}$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 0 | 0 |  |  |  |  |  |  | 16000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |  |
| $\ldots$ Modelled $\sim$-Observed - - DMRB Range $\ldots$ Series |  |  |  |  |  |  |  |  |


| 14-SB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |  |  |
| 800 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 700 \\ & 600 \end{aligned}$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 300 |  |  |  |  |  |  |  |  |
| 200 1020 1019-2. |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| Distance ( $m$ ) |  |  |  |  |  |  |  |  |
| $\ldots$ Modelled ——Observed - - DMRBRange ऑ. Series5 |  |  |  |  |  |  |  |  |



15-SB



| 17－NB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \text { ⿹丁口 } 400 \\ & \stackrel{0}{0} \\ & \text { IE } 300 \end{aligned}$ |  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |  |
| 1002793 |  |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 | 10000 |
|  | Distance（m） |  |  |  |  |  |  |  |  |  |
|  |  | $\rightarrow$ | delled | －Obs | －－ | B Range | $\cdots$ |  |  |  |




18-SB

| 18-SB |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |
| 1002111 |  |  |  |  |  |  |  |  |  |
| 0 | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 800 | 9000 |
|  |  |  |  |  |  |  |  |  |  |
|  |  | $\rightarrow$ | - | served | - DMR | - |  |  |  |




8S-0Z

| 20-SB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 450 |  |  |  |  |  |  |  |
| 400 - 3108 |  |  |  |  |  |  |  |
| 350300 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |
| 503270 |  |  |  |  |  |  |  |
|  | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 |
|  | Distance (m) |  |  |  |  |  |  |
|  |  | Modelled | bserved | DMRB Range | - Ser |  |  |



| 21-WB |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| -100 Distance (m) |  |  |  |  |  |  |
| —Modelled ——Observed - - DMRB Range —. Series5 |  |  |  |  |  |  |



22-wB


aMEz

| 23-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |  |
| 800 |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 300 |  |  |  |  |  |  |  |
| 200 退 |  |  |  |  |  |  |  |
| 1005604, -3245 |  |  |  |  |  |  |  |
| 0 |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |
|  |  | odelled | Observed | DMRB Range | " Ser |  |  |





| 26-SB |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |  |  |
|  | 800 |  |  |  |  |  | -- |  |  |
| T $\quad 600$ |  |  |  |  |  |  |  |  |  |
| 兴 400 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| -2000 | $-200{ }^{\circ}$ | 2000 | 4000 | 6000 |  | 10000 | 12000 | 14000 | 16000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |  |  |
| $\ldots$ Modelled $\multimap$-Observed - - DMRB Range $\ldots$ Series |  |  |  |  |  |  |  |  |  |


| 27-NB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1000 |  |  |  |  |  |  |  |  |
| 800 边 |  |  |  |  |  |  |  |  |
| [ ${ }^{600}$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| $20057573145 \ldots \ldots$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| $\begin{array}{lllllllll}0 & 2000 & 4000 & 6000 & 8000 & 10000 & 12000 & 14000 & 16000\end{array}$ |  |  |  |  |  |  |  |  |
| -200 Distance (1) |  |  |  |  |  |  |  |  |
| $\ldots$ Modelled $\multimap$ Observed - - DMRB Range $\ldots$ Series |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 27-SB |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 800 解 |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \frac{\pi}{\sigma} \\ & \stackrel{\pi}{E} \\ & \hline 100 \end{aligned}$ |  |  |  |  |  |  |  |  |
| ${ }_{1302}^{200} \text {, } 3905--$ |  |  |  |  |  |  |  |  |
|  | 2000 | 4000 | 6000 |  | 10000 | 12000 | 14000 | 16000 |
| -200 |  |  |  | cance (1) |  |  |  |  |
|  |  | - Mod | $\longrightarrow 0$ | - - | Range | Series5 |  |  |


| 28-NB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |
| 700 ( 324332453246 |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 200 - 3937 |  |  |  |  |  |  |  |
| ${ }^{100} 0_{1302}$ _- - - - |  |  |  |  |  |  |  |
|  | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |
| ——Modelled ——Observed - - - DMRB Range ఒ. Series5 |  |  |  |  |  |  |  |


| 28-SB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |
| [ $\begin{array}{r}500 \\ \text { (T) } \\ \hline 00\end{array}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| $\qquad$ |  |  |  |  |  |  |  |
| $-1000$ | 2000 | 4000 |  | 8000 | 10000 | 12000 | 14000 |
|  | Distance (m) |  |  |  |  |  |  |
| ——Modelled ——Observed - - - DMRB Range —\%Series5 |  |  |  |  |  |  |  |




gM-OE

| 30-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |
| 1000 ( 80 |  |  |  |  |  |  |  |
| 800 年 |  |  |  |  |  |  |  |
| $\stackrel{\pi}{\pi} \underset{E}{\infty} 600$ |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |
| $200$ |  |  |  |  |  |  |  |
| $\begin{array}{llllllll} & 2000 & 4000 & 6000 & 8000 & 10000 & 12000 & 14000\end{array}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |
| $\ldots$ Modelled ——Observed _ - - DMRBRRange $\ldots$ Series5 |  |  |  |  |  |  |  |




| 32-B |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |
| 700 边 |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |
| $\begin{gathered} 100_{258} 28 \\ 0 \end{gathered}$ |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| $-100{ }^{0}$ | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |
| $\ldots$ Modeled $\sim$ Observed - - DMRBRange ऑ. Series5 |  |  |  |  |  |  |  |


| 32-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |
| 700 边 |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |
| 1003309 |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |
|  |  | Modelled | Observed | DMRB Range | * Seri |  |  |



33-SB


ante




700
700
600 웅 우 웅
(s) $\boldsymbol{\text { 訁u! }}$



| 38-SB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |
| 1000 边 |  |  |  |  |  |  |  |  |
| 玉 ${ }^{800} 000$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |  |
| ${ }^{200} 3135$ |  |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 | 16000 |
|  | Distance ( $m$ ) |  |  |  |  |  |  |  |
| $\ldots$ Modeled $\longrightarrow$ OObserved - - DMRB Range $\ldots$ Series |  |  |  |  |  |  |  |  |


| 38-NB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1200 |  |  |  |  |  |  |  |  |
| 1000800 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| ${ }_{300} 3130 \ldots \ldots{ }^{3132}$ |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 | 14000 | 16000 |
|  |  |  |  | Distance ( $m$ ) |  |  |  |  |
|  |  | - Mod | $\square 0$ | --- | Range | Series5 |  |  |



39-5B

| 39-SB |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \text { 픙 } 400 \\ & \stackrel{\rightharpoonup}{I} 300 \end{aligned}$ |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |
| $1^{100} 3316$ |  |  |  |  |  |  |  |  |  |
|  | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 9000 |
|  | Distance (m) |  |  |  |  |  |  |  |  |
|  |  | $\longrightarrow$ | - | served | - DMRB | $\cdots$ |  |  |  |








| 43-NB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 800 |  |  |  |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |  |  |
| 1003983 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| $\ldots$ Modeled $\ldots$ Observed - - DMRBRange ¢ Series5 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 43-SB |  |  |  |  |  |  |  |  |  |  |
| 800 |  |  |  |  |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |  |  |  |  |
| 600 退 |  |  |  |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| 200 370 3235 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| $-100{ }^{0}$ | 1000 | 2000 |  | $4000$ | $\begin{aligned} & 5000 \\ & \text { stance (m) } \end{aligned}$ |  |  | 8000 | 9000 | 10000 |
|  |  | $\square$ | delled | - obse | - - - | B Range | $\cdots$ |  |  |  |


| 44-B |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |
| 600 ( |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |
| $100_{3179}$ |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 |
|  | Distance ( $m$ ) |  |  |  |  |  |
|  |  | d | - - | $\cdots$ |  |  |




GM-St

1000
900
800
700
600
500
400
300
200
10032
0
(s) $\boldsymbol{\partial w}!\perp$





| 49-NB |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 700 |  |  |  |  |  |  |  |  |  |  |  |
| 600 lon 3325 |  |  |  |  |  |  |  |  |  |  |  |
| 500 边 |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |
| 103338889 - 0 |  |  |  |  |  |  |  |  |  |  |  |
| -1000 | $-100{ }^{\circ}$ | 1000 | 2000 | 3000 |  |  |  | 7000 | 8000 | 9000 | 10000 |
|  | Distance (m) |  |  |  |  |  |  |  |  |  |  |
| ——Modelled ——Observed - - DMRBRange ——Series5 |  |  |  |  |  |  |  |  |  |  |  |




$$
\begin{aligned}
& \text { 50-WB }
\end{aligned}
$$

| 51-B |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 900 |  |  |  |  |  |  |
| 800 边 |  |  |  |  |  |  |
| 700 |  |  |  |  |  |  |
| 600 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| 300200 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  2000 4000 6000 8000 10000 12000 |  |  |  |  |  |  |
| Distance ( $\mathbf{m}$ ) |  |  |  |  |  |  |
| $\ldots$ Modelled ——Observed - - - DMRB Range ——Series |  |  |  |  |  |  |




| 52-WB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 600 |  |  |  |  |  |  |  |  |
| 500 ( |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |  |  |
|          <br> 0 1000 2000 3000 4000 5000 6000 7000 8000 |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |  |
| $\ldots$ Modeled $\ldots$ Observed - - DMRBRange -. Series5 |  |  |  |  |  |  |  |  |


an-es





55-SB






58-SB



59-WB









| 6-MB |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1400 |  |  |  |  |  |  |  |  |  |
| 1200 |  |  |  |  |  |  |  |  |  |
| 1000 |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 40030275 |  |  |  |  |  |  |  |  |  |
| $20030253$ |  |  |  |  |  |  |  |  |  |
| 0 1000 2000 3000 4000 5000 6000 7000 8000 9000 |  |  |  |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |  |  |  |
| $\longrightarrow$ Modelled $\longrightarrow$ Observed $\longrightarrow$ - - DMRB Range $\simeq$ Series5 |  |  |  |  |  |  |  |  |  |



64-WB

| 64-VB |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 400 |  |  |  |  |  |  |
| 350 |  |  |  |  |  |  |
| 300 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| 100 |  |  |  |  |  |  |
| 502010 |  |  |  |  |  |  |
| 0 1000 2000 3000 4000 5000 6000 |  |  |  |  |  |  |
| Distance (m) |  |  |  |  |  |  |
|  |  | d | - | $\cdots$ |  |  |


| 101-NB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 450 |  |  |  |  |  |  |  |  |  |  |
| 400350 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
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| 150 |  |  |  |  |  |  |  |  |  |  |
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|  |  | $\longrightarrow$ | delled | - Obs | - - | B Range | $\cdots$ - S |  |  |  |




102-SB
102-SB







| 103-WB |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 600 |  |  |  |  |  |  |  |
| 500 |  |  |  |  |  |  |  |
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| ${ }^{100}{ }_{6970}$ |  |  |  |  |  |  |  |
| 0 | 500 | 1000 | 1500 | 2000 | 2500 | 3000 | 3500 |
| Distance ( $m$ ) |  |  |  |  |  |  |  |
|  |  | Modelled | Observed | DMRB R | - Ser |  |  |





| 105-wB |  |  |  |  |  |  |  |  |
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|  | $\cdots$ | - | served | - DMRB | - |  |  |  |






| 201-SB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1400 |  |  |  |  |  |  |  |  |
| 1200 |  |  |  |  |  |  |  |  |
| 1000 ( |  |  |  |  |  |  |  |  |
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|  |  |  |  |  |  |  |  |  |
| 1200 9 2000 4000 6000 8000 10000 12000 14000 <br>   16000       |  |  |  |  |  |  |  |  |
| Distance ( $m$ ) |  |  |  |  |  |  |  |  |
| $\ldots$ Modelled $\multimap$-Observed - - DMRB Range $\ldots$ Series |  |  |  |  |  |  |  |  |



202-WB

| 202-WB |  |  |  |  |  |  |
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| 700 |  |  |  |  |  |  |
| 600 边 |  |  |  |  |  |  |
| 500 - - - - - - |  |  |  |  |  |  |
| $\begin{aligned} & \text { ज } 400 \\ & \stackrel{0}{E} \quad 300 \end{aligned}$ |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| 200 |  |  |  |  |  |  |
| $100_{3154}$ |  |  |  |  |  |  |
| 0 | 2000 | 4000 | 6000 | 8000 | 10000 | 12000 |
| Distance ( m ) |  |  |  |  |  |  |
| $\ldots$ Modelled —— Observed _ - - DMRB Range —. Series5 |  |  |  |  |  |  |



| 203-WB |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 450 |  |  |  |  |  |  |  |  |
| 400 |  |  |  |  |  |  |  |  |
| 350300 |  |  |  |  |  |  |  |  |
| $\sqrt{5} 250$ |  |  |  |  |  |  |  |  |
| \% |  |  |  |  |  |  |  |  |
| ${ }^{150}$ |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 100 \\ & 55_{5945} \end{aligned}$ |  |  |  |  |  |  |  |  |
| 0 | 500 | 1000 | 1500 | 2000 | 250 | 3000 | 3500 | 4000 |
|  |  |  |  | Distane ( $m$ ) |  |  |  |  |
|  |  | - Mod | $\rightarrow$ | --- | kange | Seres5 |  |  |




9S-902
205-SB
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206-SB

| 206-SB |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 450 |  |  |  |  |  |  |  |  |  |  |
| 400 边 3190 |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 350 \\ & 300 \end{aligned}$ |  |  |  |  |  |  |  |  |  |  |
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| 150100 |  |  |  |  |  |  |  |  |  |  |
| ${ }_{50}^{50} 38$ |  |  |  |  |  |  |  |  |  |  |
| 0 | 500 | 1000 | 1500 | 2000 | 2500 |  | 3500 | 4000 | 4500 | 5000 |
|  |  | $\cdots$ | delled | - obse | - - - | Branoe | - |  |  |  |



9M-LOZ


# Appendix E 

SIGNALISED J UNCTIONS
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## Appendix E

| Signal Details |  |  |  |
| :---: | :---: | :---: | :---: |
| Node Nr | Junction Name | Type | Controller Reference |
| 3855 | A134/Station St/The Street | Signalised Junction | BAJ1 |
| 2823 | Blyburgate/Peddars Lane | Signalised Junction | BEJ1 |
| 2804 | Ellough Rd/Ingate/Lowestoft Rd | Signalised Junction | BEJ3 |
| 2824 | A145 London Rd/St Mary Rd | Signalised Junction | BEJ4 |
| 2837 | Newgate/Smallgate/Market St/Station Rd | Signalised Junction | BEJ5 |
| 2836 | Newgate/Smallgate/Market St/Station Rd | Signalised Junction | BEJ5 |
| 50025 | B113 Bramford Road | Signalised Junction | BLJ1 |
| 5801 | London Rd/High St | Signalised Junction | BDJ2 |
| 2267 | Barton Rd/Ortwell Rd | Signalised Junction | BJ1 |
| 2311 | Angel Hill/Northgate St/Mustow St | Signalised Junction | BJ4 |
| 2221 | Tollgate Lane/Fordham Rd | Signalised Junction | BJ8 |
| 1308 | A1302 Newmarket Rd | Signalised Junction | BJ10 |
| 2866 | Beach Station Rd/Langer Rd | Signalised Junction | FJ1 |
| 2910 | High Rd West/Garrison | Signalised Junction | FJ3 |
| 2922 | Crescent Rd/Hamilton Rd/Cobbold Rd | Signalised Junction | FJ4 |
| 3577 | High Rd West/Railway Approach | Signalised Junction | FJ5 |
| 30449 | Bramford Rd/Riverside Rd | Signalised Junction | IJ1 |
| 30819 | Bramford Rd/Towerhill Rd | Signalised Junction | IJ2 |
| 20025 | Bramford Rd/Chevallier St | Signalised Junction | IJ3 |
| 30142 | Bramford Rd/Sproughton Rd | Signalised Junction | IJ4 |
| 10030 | Crown St/High St | Signalised Junction | IJ5 |
| 30758 | Duke St/Pownall Rd | Signalised Junction | IJ6 |
| 10037 | Queen St/Falcon Rd | Signalised Junction | IJ7 |
| 70151 | Felixstowe Rd/Murray Rd | Signalised Junction | IJ8 |
| 30246 | Felixstowe Rd/Derby Rd | Signalised Junction | IJ9 |
| 30250 | Felixstowe Rd/Kingsway Rd | Signalised Junction | IJ10 |
| 20077 | Foxhall Rd/Derby Rd | Signalised Junction | IJ12 |
| 10006 | Grey Friars Rd/Wolsey Rd | Signalised Junction | IJ13 |
| 20069 | Spring Rd/Grove Lane | Signalised Junction | IJ14 |
| 30235 | Landseer Rd/Clapgate Lane | Signalised Junction | IJ17 |
| 20014 | London Rd/Handford Rd | Signalised Junction | IJ18 |
| 30148 | London Road / Ranelagh Road / Hadleigh Road | Signalised Junction | IJ19 |
| 20015 | London Road / Handford Road (Lidl's) | Signalised Junction | IJ20 |
| 30158 | London Road / Robin Drive | Signalised Junction | IJ21 |
| 30381 | London Rd/Dickens Rd | Signalised Junction | IJ22 |
| 10048 | St Helens St/Upper Orwell St | Signalised Junction | IJ23 |
| 30124 | Norwich Rd/Old Norwich Rd | Signalised Junction | IJ27 |
| 20023 | Norwich Rd/Bramford Rd | Signalised Junction | IJ28 |
| 30136 | Norwich Rd/Ashcroft Rd | Signalised Junction | IJ29 |
| 30126 | Norwich Rd/Meredith Rd | Signalised Junction | IJ30 |
| 20005 | Princess St/Commercial Rd | Signalised Junction | IJ32 |
| 20004 | Princess St/Grafton Way | Signalised Junction | IJ33 |
| 30168 | Princess St/Burrell Rd/Ranliegh Rd/Railway Station | Signalised Junction | IJ34 |
| 30167 | Ranelagh Rd/Ancaster | Signalised Junction | IJ36 |
| 20065 | Spring Rd/Cauldwell Hall Rd | Signalised Junction | IJ37 |
| 10020 | St Helens St/Argyle St | Signalised Junction | IJ38 |
| 10056 | Star Lane/Slade St | Signalised Junction | IJ39 |
| 10018 | Star Lane/Grimwade St | Signalised Junction | IJ40 |
| 30213 | Stoke St/Burrel Rd | Signalised Junction | IJ41 |
| 30214 | Stoke St/Vernon St/Stoke Bridge | Signalised Junction | IJ41 |
| 10090 | Carr St/Upper brook St/Northgate St | Pedestrian Crossing | IJ42 |
| 10045 | Tacket St/Dogs Head St | Signalised Junction | IJ43 |
| 20044 | Valley Rd/Henly Rd | Signalised Junction | IJ44 |
| 70043 | Wherstead Rd/Station St | Signalised Junction | IJ45 |
| 20057 | Woodbridge Rd/Sidegate Lane | Signalised Junction | IJ46 |
| 30196 | Stoke Park Drive/ASDA | Signalised Junction | IJ48 |
| 20072 | Duke St/Fore Hamlet | Signalised Junction | IJ51 |
| 20016 | Handford Rd/Portman Rd | Signalised Junction | IJ52 |
| 10004 | Civic Drive/Princess St | Signalised Junction | IJ53 |
| 10049 | St Helens St/Bond St | Signalised Junction | IJ55 |
| 30799 | Copdock A14 / A12 roundabout | Signalised Junction |  |
| 30798 | A14/A12/A1214 roundabout | Signalised Junction |  |
| 30797 | A14/A12/A1214 roundabout | Signalised Junction |  |
| 30796 | A14/A12/A1214 roundabout | Signalised Junction |  |


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## Signal Details

| Node Nr | Junction Name | Type | Controller Reference |
| :---: | :---: | :---: | :---: |
| 30419 | Ropes Drive West/A1214 Woodbridge Rd | Signalised Junction | Kj3 |
| 30420 | Ropes Drive West/A1214 Woodbridge Rd | Signalised Junction | Kj3 |
| 30114 | A1214/ Martlesham Park n Ride Entrance/Exit Roundabout | Signalised Junction | MAJ1 |
| 30406 | A1214/ Martlesham Park $n$ Ride Entrance/Exit Roundabout | Signalised Junction | MAJ1 |
| 30407 | A1214/ Martlesham Park n Ride Entrance/Exit Roundabout | Signalised Junction | MAJ1 |
| 30408 | A1214/ Martlesham Park n Ride Entrance/Exit Roundabout | Signalised Junction | MAJ1 |
| 30822 | Hadleigh Road / Sainsburys / Allenby Road Ipswich | Signalised Junction | IJ15 |
| 30240 | Nacton Road / Maryon Road | Signalised Junction | IJ26 |
| 10031 | St Matthew St / Crown Street / St Georges St | Pedestrian Crossing |  |
| 20252 | Colchester Road near Sidegate Lane | Pedestrian Crossing |  |
| 70306 | Crown Street / Crown Pools | Pedestrian Crossing |  |
| 70307 | Crown Street / Fonnereau Rd | Pedestrian Crossing |  |
| 70309 | Bixley Road / St Augustine Gardens | Pedestrian Crossing |  |
| 70310 | Bixley Road near Foxhall Road | Pedestrian Crossing |  |
| 70311 | Heath Road near Foxhall Road | Pedestrian Crossing |  |
| 70312 | Heath Road near Ipswich Hospital | Pedestrian Crossing |  |
| 70313 | St Margarets Street / Crown St | Pedestrian Crossing |  |
| 70316 | Norwich Road / Old Norwich Road | Pedestrian Crossing |  |
| 70317 | Woodbridge Road gyratory | Pedestrian Crossing |  |
| 70318 | Woodbridge Road gyratory | Pedestrian Crossing |  |
| 70319 | Grafton Way near Bridge Street | Pedestrian Crossing |  |
| 70320 | Valley Road near Noriwch Road | Pedestrian Crossing |  |
| 70321 | Woodbridge Road near Arthur Terrace | Pedestrian Crossing |  |
| 70322 | A12 Belvedere Road | Pedestrian Crossing |  |
| 70323 | A12 Belvedere Road | Pedestrian Crossing |  |
| 70324 | Bridge Road Road | Pedestrian Crossing |  |
| 4040 | A146 Victoria Road - railway crossing | Level Crossing |  |
| 1260 | Eastern A12 Bascule Bridge - centre | Level Crossing |  |
| 7030 | A1117 Bridge Road level crossing | Level Crossing |  |
| 10191 | Harbour Road level crossing | Level Crossing |  |
| 5650 | A146 level crossing | Level Crossing |  |
| 5651 | Ingate level crossing | Level Crossing |  |
| 5652 | London Rd level crossing | Level Crossing |  |
| 5653 | Grove Rd level crossing | Level Crossing |  |
| 5861 | A1065 level crossing | Level Crossing |  |
| 5859 | Bridge Rd level crossing | Level Crossing |  |
| 5851 | Station Road level crossing | Level Crossing |  |
| 5859 | A154 level crossing | Level Crossing |  |
| 5858 | Bramfield Rd level crossing | Level Crossing |  |
| 5862 | B1112 level crossing | Level Crossing |  |
| 5860 | Saxmundham level crossing | Level Crossing |  |
| 70269 | The Street level crossing | Level Crossing |  |
| 5853 | A137 level crossing | Level Crossing |  |
| 5854 | Wilford Bridge Rd level crossing | Level Crossing |  |
| 5850 | Dullingham Rd level crossing | Level Crossing |  |
| 5860 | B1102 level crossing | Level Crossing |  |
| 5863 | A1101 level crossing | Level Crossing |  |
| 5851 | Station Rd level crossing | Level Crossing |  |
| 5852 | Crown Street level crossing | Level Crossing |  |
| 5856 | Yoxford Rd level crossing | Level Crossing |  |
| 5857 | A12 level crossing | Level Crossing |  |
| 30333 | Westerfield Rd level crossing | Level Crossing |  |


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